

**FUNCTIONAL SERVICING & STORMWATER  
MANAGEMENT REPORT**

**GLENELG PHASE 3  
DUNDALK VILLAGE TWO INC.**

**TOWNSHIP OF SOUTHGATE**

**PREPARED BY:**

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## 1.0 INTRODUCTION

Crozier Consulting Engineers ("Crozier") has been retained by Dundalk Village Two Inc. ("the Developer") to prepare a Functional Servicing and Stormwater Management Report in support of a Draft Plan of Subdivision Application for Glenelg Phase 3 ("Subject Development") located in the north end of the Community of Dundalk, Township of Southgate, County of Grey. The proposed development is herein referred to as the Subject Development. Please refer to **Figure 1** for the Site Location.

The Developer's overall property is approximately 132 ha. The property is legally described as Lots 227, 226, 225, 224, 223, Concession 1, southwest of the Toronto and Sydenham Road as well as Lots 226, 225 Concession 2, southwest of the Toronto and Sydenham Road Geographic Township of Proton, Township of Southgate, County of Grey.

The Subject Development is approximately 33.3 ha and is bounded by CP Rail Trail to the west, wetlands to the east, farmland to the north and residential properties to the south. The Subject Development is located northeast of Glenelg Residential Development – Phase 2.

The Subject Development will consist of 369 single-detached units, 18 semi-detached units, 72 townhouse units, a park block, walkways/ trails, a stormwater management block, an environmental protection area, and approximately 7.45ha of urban Right-of-Way (ROW). The Draft Plan prepared by MHBC Planning (August 18, 2022) has been included in **Figure 2**.

The Developer has assembled a multi-disciplinary consulting team to assist with the technical studies in support of this development. The consulting team includes:

- SLR Consulting Ltd. (SLR) (environmental and hydrogeological)
- Soil Engineers Ltd. (SEL) (geotechnical)
- MHBC (planning)
- CF Crozier & Associates Inc. (civil and transportation engineering)

This report should be read in conjunction with the studies, plans and reports prepared by other members of the development team.

This report has been prepared to provide information concerning the servicing (water, sewer, utilities, and roads) and stormwater management strategy for the development.

## 2.0 DEVELOPMENT BACKGROUND

The Subject Development is currently designated as Neighborhood Area, within Schedule 'A' Map 2 of the Township of Southgate Official Plan (2022) and associated amendments. The hazard lands are taken into consideration in the Concept Plan as that area will be designated as Environmental Protection.

Nearby Draft Plan Approved developments currently undergoing the detailed design process include the 155-unit Glenelg Residential Development – Phase 2 bordering the west limits of the Subject Development and the 88-unit White Rose Park – Phase 3 Development located to the south.

### 3.0 SITE DESCRIPTION

The overall 33.3 ha Subject Development consists largely of agricultural fields, and a natural heritage area exists in the east portion of the site. The limits of the Natural Heritage area have been staked and an appropriate development setback has been applied from dripline through consultation with the Grand River and Saugeen Valley Conservation Authorities. The natural heritage constraints on site have been investigated in detail by SLR. Refer to the Environmental Impact Study (SLR, November 2021) which has been provided under separate cover for more information.

The property topography consists of rolling hills with a central depression with a low point at the northern property line. On site elevations range from 515.5 to 525.2 m. The site lies within the regulatory boundary of the Grand River and Saugeen Valley Conservation Authorities. Site drainage is further discussed in **Section 7.2**.

Preliminary desktop review of the soil by the Soil Survey Map of Grey County (1962) indicates that the site is underlain by Listowel Silty Loam soils. At this time SEL is undergoing a soil investigation which will be provided in support of future submissions.

### 4.0 ROAD STANDARD

Access to the Development is proposed to be provided through two entrances from Glenelg Residential Development – Phase 2, as well as an entrance on the southern boundary from Bradley Street. Roadways and entrances will be constructed in conformance with the Township of Southgate Engineering Standards.

A Traffic Impact Study has been prepared by our office under separate cover, which details transportation engineering considerations and mitigative measures related to the Development. Roadway slopes will range between 0.5% and 8% in conformance with Township of Southgate Engineering Standards. The general grading concept for the Development is presented in **Figure 3**.

Design criteria for the entrances will meet municipal guidelines as well as the applicable sections from the Ontario Building Code (i.e., fire routes).

The internal roadways of the Development will be assumed by the Municipality upon registration of the subdivision.

### 5.0 SANITARY SEWAGE SYSTEM

#### 5.1 Existing Sanitary Sewer Infrastructure

##### 5.1.1 Wastewater Treatment Plant Capacity

The existing wastewater treatment plant (WWTP) is located on Eco Parkway at the south end of Dundalk. The plant treats sewage and discharges the treated effluent to the Foley Drain/ Grand River. Per the Township of Southgate 2022 Reserve Capacity Study (Triton Engineering, 2022), the plant currently operates on average at 1,165 m<sup>3</sup>/day. The uncommitted reserve capacity for the sewage treatment facility is 182 new development ERU's (Equivalent Residential Units). Since this reserve capacity will not be sufficient to service various potential developments that have been granted Draft Plan Approval the municipality is currently in the process of upgrading the wastewater treatment plant to increase capacity in Dundalk from 1,832 m<sup>3</sup>/day to 3,025 m<sup>3</sup>/day to support growth. Refer to **Appendix A** for relevant wastewater treatment plant capacity calculations.

The estimated completion dates of the wastewater treatment plant upgrades are early 2023, which coincides with the timing of building permit submission for the Subject Development. **Table 1** details the Township of Southgate '2022 Reserve Sewage Capacity' and project the wastewater treatment capacities after the 2023 upgrades have been completed.

**Table 1: Township of Southgate Dundalk Sewage Treatment Facility Capacity Projections**

| <b>Dundalk Sewage Treatment Facility Summary</b>                    |             |             |
|---|-------------|-------------|
| <b>Description</b>  | <b>2021</b> | <b>2023</b> |
| Wastewater Treatment Facility Design Capacity (m <sup>3</sup> /day) | 1,832       | 3,025       |
| Current Daily Average Flow (m <sup>3</sup> /day)                    | 1,165       |             |
| Available Capacity (m <sup>3</sup> /day)                            | 667         | 1,860       |
| Average New Development Per Capita Flow <sup>(1)</sup>              | 0.350       | 0.350       |
| Additional Population that can be Served                            | 1905        | 5,314       |
| Person Per ERU <sup>(1)</sup>                                       | 2.66        | 2.66        |
| Additional ERU Capacity   | 716         | 1997        |
| Committed ERUs  | 534         | 534         |
| Available Uncommitted ERUs  | 182         | 1463        |

<sup>(1)</sup> Per 2017 DC Background Study

### 5.1.2 Existing Infrastructure

The existing sewage infrastructure within the vicinity of the Subject Development includes the following:

- Two (2) 250 mm diameter sanitary sewer stubs located at Aitchison Avenue and Corbett Street (Glenelg Residential Development – Phase 1 Lands).
- One (1) 200 mm diameter sanitary sewer that currently ends at a Maintenance Hole at the end of Bradley Street

### 5.1.3 Future Infrastructure

- Two (2) 250 mm diameter sanitary sewer stubs located at Corbett Street (Glenelg Residential Development – Phase 2 Lands).
- One (1) 200 mm diameter sanitary sewer that the Bradley Street Extension (White Rose Development – Phase 3)

## 5.2 Proposed Sanitary Sewer Infrastructure

Sanitary servicing for the development will be supplied by way of connection to the existing Dundalk sanitary sewer collection network. Flows from the Subject Development will ultimately be conveyed to the existing Dundalk Wastewater Treatment Plant.

The Subject Development will be serviced via a gravity sanitary sewer system that follows the alignment of the internal roadway network, with individual service connections to each lot. A substantial amount of fill will be required near portions of the site to ensure adequate cover over the

sanitary sewer. Options to reduce fill quantities will be explored. Upon subdivision registration, sewers and associated roadways will be assumed by the Municipality.

The proposed sanitary sewer for the development will connect to a 200 mm diameter sanitary sewer stub on Bradley Street as well as two 250 mm diameter connections from Glenelg Residential Development Phase 2. The estimated sanitary flow to Glenelg Phase 2 was found to be 12.32 L/s. The estimated flow to the Bradley Street connection was found to be 10.33 L/s. The conveyance capacity of downstream sanitary sewer network, through the Glenelg Residential Development – Phase 2, has been determined to be adequate. A preliminary assessment of the sewers downstream of the Bradley Street connection was completed, a section of 200 mm diameter pipe along Artemesia Street between Toronto Street and Owen Sound Street will be approaching 100% capacity under the proposed development conditions. This leg of sewer and roadway is proposed to be reconstructed to meet the service requirements of the subject development. The conveyance capacity of all downstream infrastructure will be subject to confirmation by the Township's Engineering Consultant. Sanitary infrastructure for the proposed development is illustrated in **Figure 4**. Refer to **Appendix A** for the sanitary flow calculations.

Sanitary flow estimates for the development were estimated in conjunction with the Township of Southgate Engineering Design Standards and the "New Development Unit Flow Rates" as described within the 2022 Reserve Capacity, prepared by the Township's Engineering Consultant.

## **6.0 POTABLE WATER SUPPLY**

### **6.1 Existing Potable Water Supply Infrastructure**

Potable water for the development will be supplied by the Township's municipal water distribution system.

The existing water treatment plant system in Dundalk includes three existing production wells. Per the 2022 Reserve Capacity Study, the well system operates at a maximum daily flow of 941 m<sup>3</sup>/day. This value represents approximately 33% of the system's allowable withdrawal capacity of 2,817 m<sup>3</sup>/day, as specified in the Township's Permit to Take Water. Based on this, the existing system has ample capacity to support Glenelg Phase 3 Development. Refer to **Appendix B** for relevant water capacity calculations.

The existing water distribution infrastructure within the vicinity of the Subject Development includes the following:

- Two (2) 150mm diameter watermain connection stubs located at Aitchison Avenue and Corbett Street (Glenelg Residential Development – Phase 1 Lands).
- One (1) 150mm diameter watermain that dead ends at Bradley Street.

### **6.2 Future Infrastructure**

- Two (2) 150mm diameter watermain connection stubs located at Corbett Street (Glenelg Residential Development – Phase 2 Lands).
- One (1) 150mm diameter watermain that dead ends at Bradley Street Extension (White Rose Development – Phase 3).

### 6.3 Proposed Servicing Strategy

The Subject Development will be serviced via three future watermain connections. There will be two watermain connections at the future ROWs within the Glenelg Residential Development – Phase 2. Additionally, there will be a connection to the watermain at the future White Rose Park – Phase 3.

Watermain with individual service connections for each unit will follow the alignment of the internal roadways according to Township Standards. Connections to the Glenelg Residential Development – Phase 2 and White Rose – Phase 3. This will facilitate a looped distribution network and satisfy the Township and Ministry of Environment, Conservation and Parks' requirements for a looped water distribution system. The Draft Plan does not include any watermain dead-ends and therefore there should be no issue with respect to providing adequate water circulation and preventing the potential for stagnant potable water.

Fire hydrants will be spaced as required to provide the necessary fire protection per municipal standards. Required domestic water flows have been calculated in conformance with the Township of Southgate's Engineering Design Standards and the "New Development Unit Flow Rates" specified within the 2022 Reserve Capacity Study. The maximum day and peak hour water demands have been estimated to be 10.70 L/s and 16.07 L/s, respectively. Additional water supply considerations including fire suppression requirements will be determined during the detailed design phase. Internal watermain sizing will be subject to detailed design and confirmation by the Township's Engineering Consultant.

Refer to **Appendix B** for relevant water demand calculations. The proposed watermain layout is illustrated on **Figure 4**.

## 7.0 PROPOSED STORMWATER MANAGEMENT, SITE GRADING AND DRAINAGE

### 7.1 Stormwater Management (SWM) Criteria

The management of stormwater and site drainage for the proposed development must comply with the policies and standards of the various agencies including the Township of Southgate, Grand River Conservation Authority (GRCA), Saugeen Valley Conservation Authority (SVCA) and the Ministry of Environment, Conservation and Parks (MECP).

The stormwater management criteria for the Subject Development includes:

- Water Quantity Control
  - Control of post-development peak flows to pre-development levels for all storms up to and including the 100-year event.
- Water Quality Control
  - 80% removal efficiency of total suspended solids per MECP "enhanced protection" requirements.
- Erosion Control
  - Minimum 24-hour detention of the 25mm event.
- Development Standards
  - Urban cross section for public roadway with 5-year storm sewer system.
  - Lot grading at 2% optimum.

- Minor and major drainage system to convey frequent and infrequent rainfall/runoff events, respectively.

In meeting the applicable policies and standards of the aforementioned agencies, the development will also be required to meet the following criteria:

- Manage the internal stormwater by safely conveying peak flows to suitable outlets and provide the necessary water quality controls.
- Manage any external drainage entering the site by providing safe conveyance across the Subject Development.
- Ensuring the development lands are not susceptible to flood inundation during all storm events.

## 7.2 Existing Drainage Conditions

Topographic survey indicates that the terrain has rolling hills with a depression in the middle of the site and a low point at the north property line. There is a high point near the east corner of the site where runoff drains towards the natural heritage area and the east property line. The rest of the site drains towards a point on the southeast property line, towards the CP Rail Trail and to the residential lands to the south.

To the west of the development lies the CP Rail Trail. This trail represents an elevated linear structure bounding the property line and has been constructed with side ditches preventing external flows from entering the development site.

To facilitate the pre-development stormwater analysis, the following five (5) catchments have been delineated based on the existing drainage conditions.

- **Catchment PRE-1:** This catchment area is approximately 4.32 ha and is located along the west border of the site. It consists of active agricultural fields. Stormwater from this catchment drains to the west towards the CP trail (outlet #1). Flows entering the CP trail drain in the north direction to a tile drain located on a neighboring property to the north. The flows in the tile drain ultimately are discharged to the northeast wetland.
- **Catchment PRE-2:** This catchment area is approximately 13.33 ha and is in the middle of the site. It consists of active agricultural fields. Stormwater from this catchment drains to the north tile drain (outlet #2). The flows in the tile drain are discharged to the northeast wetland, the same wetland that PRE-1 ultimately discharges to.
- **Catchment PRE-3:** This catchment area is approximately 3.05 ha and is located along the eastern corner of the subject site. It consists of active agricultural fields. Stormwater from this catchment drains to the east tile drain (outlet #3). Flows entering the east tile drain are conveyed to a wetland located within the SVCA regulation area.
- **Catchment PRE-4:** This catchment area is approximately 2.29 ha and is located along the southeastern corner of the subject site. It consists of active agricultural fields. Stormwater from this catchment drains to the southeast tile drain (outlet #4). Flows entering the southeast tile drain are conveyed to a wetland located within the GRCA regulation area.
- **Catchment PRE-5:** This catchment area is approximately 3.00 ha and is located along the south boundary of the subject site. It consists of active agricultural fields. Stormwater from this catchment drains to the residential subdivision to the south.

The existing drainage patterns of the site and conservation authority regulation areas have been reflected in the Pre-Development Drainage Plan (**Figure 5**). Additional information on the pre-development drainage and tile drain locations is provided in **Figure 6**. For the pre-development hydro parameter sheets, please refer to **Appendix C**.

### 7.3 Proposed Drainage Conditions

The Subject Development will be constructed to a fully urbanized system complete with curb and gutter and storm sewers. A dual drainage approach will consist of minor and major stormwater flow routes to ensure adequate conveyance for runoff. The minor drainage system will consist of storm sewers and catchbasins sized to convey the 5-year design storm event. The major drainage system will provide overland stormwater flow routes within the road allowance. Refer to **Figure 4** for proposed storm sewer layout.

To facilitate the post-development stormwater analysis, the following seven (7) catchments have been delineated based on the proposed drainage conditions.

- **Catchment POST-1:** This catchment area is approximately 0.50 ha and consists of a portion of the proposed developed area. The catchment consists of the backlots of the single-detached units. Runoff from this catchment will drain uncontrolled to the Grey County CP Trail (outlet #1). Flows entering the CP trail drain in the north direction to a tile drain located on a neighboring property to the north. The flows in the tile drain ultimately are discharged to the northeast wetland.
- **Catchment POST-2:** This catchment area is approximately 18.83 ha and consists of a large portion of the proposed developed area. The catchment consists of single-family residential, semi-detached residential, townhouses, walkways, and roads. Runoff from this catchment will drain to the SWM facility that discharges to outlet #2. The flows then enter the northeast wetland.
- **Catchment POST-3:** This catchment area is approximately 2.84 ha and consists of a portion of the proposed developed area. The catchment consists of single-family residential, parklands, walkways, and roads. Runoff from this catchment will drain ultimately to outlet #3 (east tile drain). Flows are then conveyed to the east wetland.
- **Catchment POST-4:** This catchment area is approximately 1.09 ha and consists of a portion of the proposed developed area. The catchment consists of the backlots of single-detached units. Runoff from this catchment will drain controlled and uncontrolled to outlet #4 (southeast tile drain).
- **SWMF:** This catchment represents the proposed 1.43 ha SWM Facility block. Runoff from this catchment will drain to the SWM Facility. The SWM Facility will then discharge flows to outlet #2 (north tile drain). Flows are then discharged to the northeast wetland.
- **Catchment POST-5:** This catchment area is approximately 1.28 ha and consists of front lots of single-detached residential units and road area. The minor system will be captured and conveyed to the SWMF. The major system will drain overland to the SWM Facility servicing the Glenelg Phase 2 Lands.
- **Catchment TR-1:** This catchment area is approximately 0.78 ha and consists of the backlots of the single-detached units in Glenelg Phase 2. Runoff from this external catchment will drain uncontrolled to the Grey County CP Trail (outlet #1). Flows entering the CP trail drain in the north direction to a tile drain located on a neighboring property to the north. The flows in the tile drain ultimately are discharged to the northeast wetland. This drainage was taken into consideration from the Glenelg Phase 2 Lands.

The post-development drainage conditions and catchment areas have been presented in **Figure 8**.

Under post-development conditions, the majority of the stormwater from the Subject Development is currently proposed to drain to the SWM facility and outlet to the north tile drain (outlet #2). A portion of the site will drain to each outlet to mimic pre-development conditions. The exception to this is the pre-development catchment draining into the southern residential subdivision (PRE-5). In post-development conditions, flows will not drain to the residential subdivision to the south, as the subdivision was not designed to accommodate the drainage. Additionally, it should be noted that a



portion of the subject site will drain overland to the Glenelg Phase 2 subdivision SWMF to alleviate grading issues associated with matching into the proposed right-of-way connections. The Glenelg Phase 2 SWM Facility is sufficiently sized to accept, treat, and control the major system drainage from this area. Excerpts from the Phase 2 SWM Facility modeling are included in **Appendix G**.

#### **7.4 Proposed SWM Strategy**

The stormwater management strategy varies for each of the seven (7) post-development catchment areas.

For catchment POST-1, backlots of the single-detached units will be conveyed via sheet flow into the CP Trail ditch (Outlet #1). The clean water from the backlots of the single-detached units is proposed to flow uncontrolled and will not require quality treatment. Flows entering outlet #1 (CP Trail) will be conveyed to the north in the CP trail ditch to a neighboring tile drain that ultimately discharges runoff to the northeast wetland.

For catchment POST-2, the drainage area will be routed through the SWMF (SWM pond). Flows from the front lots of the residential units and roadway area will be treated via catchbasin shields and tree pits prior to entering the SWMF. The SWMF is adequately sized to provide quantity, quality, and erosion control for the contributing drainage area. The conceptual proposed stormwater management facility has incorporated a permanent pool and a sediment forebay to provide appropriate water quality treatment. The SWMF will also incorporate extended detention of the 25mm storm event to provide erosion protection. An outlet from the facility will be provided to release flows through to the northeast wetland via outlet #2. Quantity control will be achieved to ensure pre-development peak flows are not exceeded to outlet #2 (north tile drain) and the northeast wetland.

For catchment POST-3, front lots of the single-detached units and road area will be directed to catchbasin shields and tree pits. Overflow from the front lots/road area and park runoff will be directed to the permeable pavement areas (walkways) and bioretention facilities within the park prior to being discharged to Outlet #3 (east tile drain). This treatment train approach will achieve quality control targets (enhanced 80% TSS removal). Clean water from the backlots of the single-detached units will drain controlled via a superpipe and uncontrolled via sheet flow to outlet #3. Quantity control will be achieved to ensure pre-development peak flows are not exceeded to outlet #3 (east tile drain) and the east wetland.

For catchment POST-4, clean water from the back lots of the residential units will drain to outlet #4. Runoff from the backlots will drain controlled via a superpipe and uncontrolled via sheet flow to outlet #4 and the southeast wetland. Quantity control will be achieved to ensure pre-development peak flows are not exceeded to outlet #4 (southeast tile drain) and the southeast wetland.

Catchment POST-5 has a separate outlet for the major and minor storm events. For the minor storm events (up to the 5-year design storm) the front lots of the single-detached units and road area will be directed to catchbasin shields and tree pits prior to entering the storm sewer network and being discharged to SWMF and ultimately Outlet #2. The SWMF has been designed to match pre-development peak flow rates at the outlet taking the POST-5 flows into consideration. For the major storm events (flow exceeding the 5-year design storm), flows will be conveyed through the two road connections to Glenelg Phase 2. Stormwater entering Glenelg Phase 2 will be conveyed through the road network into the stormwater management pond. The Glenelg Phase 2 pond has sufficient capacity to treat flows from POST-5. Quality treatment for the stormwater has been provided in the Glenelg Phase 3 SWMF.

For catchment SWMF, clean water will drain to the SWM Pond. The SWMF is adequately sized to provide quantity, quality, and erosion control for the contributing drainage area. The conceptual proposed stormwater management facility has incorporated a permanent pool and a sediment forebay to provide appropriate water quality treatment. The SWMF will also incorporate extended detention for erosion protection. An outlet from the facility will be provided to release flows through to outlet #2, the northeast wetland. Quantity control will be achieved to ensure pre-development peak flows are not exceeded to outlet #2.

For catchment TR-1, clean water from the residential backlots of the Glenelg Phase 2 Lands will drain uncontrolled via sheet flow to the Grey County CP Trail (outlet #1). These flows will not require quality control treatment. Quantity control will be achieved to ensure pre-development peak flows are not exceeded to outlet #1.

## 7.5 Hydrologic Analysis

A hydrologic model was prepared for the pre-development and post-development scenarios using the stormwater management hydrologic computer program Visual OTTHYMO 6.1 (VO6). The purpose of the modeling was to ensure that quantity control requirements are met (i.e., post-development peak flow rates do not exceed the pre-development flows to the respective drainage area).

To accurately assess the peak flows from the individual catchments, the NasHyd command in VO6 was used to model the pre-development drainage areas. Design storms were generated for the 2-year, 5-year, 10-year, 25-year, 50-year and 100-year events utilizing both 3-hour Chicago and 24-hour SCS Type II rainfall distributions. The Township of Southgate Engineering Standards requires only the modeling of the 3-hour Chicago distribution storms for quantity control facilities. Nevertheless, the 24-hour SCS distribution has been included in the design to ensure the provision of adequate and conservative quantity control. The 25mm Chicago quality event was also modeled within VO6. Intensity-Duration-Frequency (IDF) values were derived from the Ministry of Transportation IDF tool for the Community of Dundalk.

### 7.5.1 Pre-Development Model Setup

To establish the pre-development peak flows, the contributing drainage areas were modeled within VO6. Please refer to **Figure 5** for the Pre-Development Drainage Plan and **Appendix C** for the pre-development hydrological parameter sheets. **Table 2** summarizes the pre-development peak flows rates obtained from the VO6 model.

**Table 2: Summary of Pre-Development Peak Flow Rates**

| Return Period         | Pre-Development Peak Flow Rates (m <sup>3</sup> /s) |   |   |  |  |
|-----------------------|---|---|---|--|--|
|                       | PRE-1<br>Drainage to<br>CP Trail<br>(4.32ha)        | PRE-2<br>Drainage to<br>North Tile Drain<br>(13.33ha) | PRE-3<br>Drainage to<br>East Tile Drain<br>(3.05ha) | PRE-4<br>Drainage to<br>South<br>Residential<br>(2.29ha) | PRE-5<br>Drainage to<br>Southeast Tile<br>Drain<br>(3.0ha) |
| 3-Hour 10 min Chicago |   |   |   |  |  |
| 2-Year                | 0.044   | 0.100   | 0.041   | 0.022  | 0.047  |
| 5-Year                | 0.086   | 0.190   | 0.081   | 0.042  | 0.093  |
| 10-Year               | 0.119   | 0.261   | 0.115   | 0.059  | 0.131  |
| 25-Year               | 0.166   | 0.361   | 0.162   | 0.082  | 0.186  |

|                            |       |       |       |       |       |
|----------------------------|-------|-------|-------|-------|-------|
| 50-Year                    | 0.204 | 0.442 | 0.200 | 0.101 | 0.230 |
| 100-Year                   | 0.244 | 0.528 | 0.241 | 0.121 | 0.278 |
| 24-Hour 15 min SCS Type II |       |       |       |       |       |
| 2-Year                     | 0.143 | 0.296 | 0.144 | 0.070 | 0.162 |
| 5-Year                     | 0.239 | 0.495 | 0.241 | 0.117 | 0.269 |
| 10-Year                    | 0.310 | 0.643 | 0.313 | 0.152 | 0.349 |
| 25-Year                    | 0.404 | 0.839 | 0.407 | 0.199 | 0.455 |
| 50-Year                    | 0.476 | 0.990 | 0.479 | 0.234 | 0.537 |
| 100-Year                   | 0.551 | 1.146 | 0.554 | 0.271 | 0.621 |

### 7.5.2 Post-Development Model Setup

The post-development model was prepared by adding the post-development drainage catchments as referenced in **Section 7.3**. Refer to **Figure 8** for the Post-Development Drainage Plan and **Appendix C** for the post-development area breakdown calculations.

To model the post-development condition, the respective drainage areas were modelled via a combination of StandHyds, NasHyds, RouteReservoirs, DuHyds, and LID commands. Using these tools, the post-development peak flows were analyzed to ensure that quantity control targets were met in the post-development condition. The DuHyd commands were applied to separate the minor and major system flows entering the LIDs. The LIDs were sized based on a water balance analysis that is discussed further in **Section 7.6**. For additional information regarding the layout of the VO6 model, please refer to the VO schematic presented in **Appendix D**.

### 7.5.3 Quantity Control

'Post-to-Pre' peak flow control is proposed for the four outlets for all storm events up to and including the 100-year storm event.

#### Outlet #1

**Table 3** presents the pre-development and post-development peak flows to outlet #1. As shown, the post-development flows are less than the pre-development flows for all storms.

**Table 3: Summary of Pre- and Post-Development Peak Flows (Outlet #1)**

| Return Period              | Pre-Development<br>(m <sup>3</sup> /s) [4.32 ha] | Post-Development<br>(m <sup>3</sup> /s) [1.28 ha] <sup>(1)</sup> |
|----------------------------|--|--|
| 3-Hour 10 min Chicago      |  |  |
| 2-Year                     | 0.044  | 0.040  |
| 5-Year                     | 0.086  | 0.073  |
| 10-Year                    | 0.119  | 0.099  |
| 25-Year                    | 0.166  | 0.133  |
| 50-Year                    | 0.204  | 0.160  |
| 100-Year                   | 0.244  | 0.188  |
| 24-Hour 15 min SCS Type II |  |  |
| 2-Year                     | 0.143  | 0.098  |
| 5-Year                     | 0.239  | 0.155  |
| 10-Year                    | 0.310  | 0.195  |

|          |       |       |
|----------|-------|-------|
| 25-Year  | 0.404 | 0.248 |
| 50-Year  | 0.476 | 0.288 |
| 100-Year | 0.551 | 0.328 |

(1) Catchment area from TR-1 (0.78ha) and POST-1 (0.50ha)

### Outlet #2

The proposed SWM Facility will be a stormwater management wet pond located on the north side of the development that will provide the required stormwater quantity, quality, and erosion controls. The SWM pond will discharge stormwater to outlet #2 into the northeast wetland.

A preliminary outlet structure has been designed as a multi-stage outlet to address both quality and quantity control requirements. The outlet structure will consist of a 200 mm diameter extended detention orifice set at the permanent pool elevation. A secondary orifice has been included above the extended detention storage elevation to control effluent from the pond for storms exceeding the 25 mm event.

Using the ROUTE RESERVOIR command in VO6, the volume of detention storage required in the SWMF to attenuate the post-development peak flows to pre-development was determined based on a storage – discharge relationship. The ROUTE RESERVOIR command was used to model the wet pond storage. To meet quantity control requirements, it was concluded that approximately 12,720 m<sup>3</sup> of active detention storage is required in the SWM Facility. **Table 4** presents the required storage volume for the SWMF. Please refer to **Appendix E** for detailed SWMF calculations. The preliminary design of the SWM Facility has been presented in **Figure 9**.

**Table 4: Summary of SWM Facility – Required Storage Volumes**

| Return Period | Storage Volume (m <sup>3</sup> ) |                            |
|---------------|----------------------------------|----------------------------|
|               | 3-Hour 10 min Chicago            | 24-Hour 15 min SCS Type II |
| 2-Year        | 3914                             | 5581                       |
| 5-Year        | 5171                             | 7520                       |
| 10-Year       | 5883                             | 8725                       |
| 25-Year       | 6861                             | 10340                      |
| 50-Year       | 7548                             | 11524                      |
| 100-Year      | 8244                             | 12720                      |

**Table 5** presents the pre-development and post-development peak flows to outlet #2. As shown, the post-development flows are less than the pre-development flows for all storms.

**Table 5: Summary of Pre- and Post-Development Peak Flows (Outlet #2)**

| Return Period (Years)      | Pre-Development (m <sup>3</sup> /s) [13.33 ha] | Post-Development (m <sup>3</sup> /s) [21.55 ha] |
|----------------------------|--|---|
| 3-Hour 10 min Chicago      |  |   |
| 2-Year                     | 0.100  | 0.092   |
| 5-Year                     | 0.190  | 0.186   |
| 10-Year                    | 0.261  | 0.239   |
| 25-Year                    | 0.361  | 0.327   |
| 50-Year                    | 0.442  | 0.404   |
| 100-Year                   | 0.528  | 0.481   |
| 24-Hour 15 min SCS Type II |  |   |

|          |       |       |
|----------|-------|-------|
| 2-Year   | 0.296 | 0.216 |
| 5-Year   | 0.495 | 0.401 |
| 10-Year  | 0.643 | 0.535 |
| 25-Year  | 0.839 | 0.721 |
| 50-Year  | 0.990 | 0.868 |
| 100-Year | 1.146 | 1.019 |

**Outlet #3**

**Table 6** presents the pre-development and post-development peak flows to outlet #3. As shown, the post-development flows are less than the pre-development flows for all storms.

**Table 6: Summary of Pre- and Post-Development Peak Flows (Outlet #3)**

| Return Period (Years)      | Pre-Development<br>(m <sup>3</sup> /s) [3.05 ha] | Post-Development<br>(m <sup>3</sup> /s) [2.84 ha] |
|----------------------------|--|---|
| 3-Hour 10 min Chicago      |  |   |
| 2-Year                     | 0.041  | 0.026   |
| 5-Year                     | 0.081  | 0.044   |
| 10-Year                    | 0.115  | 0.065   |
| 25-Year                    | 0.162  | 0.112   |
| 50-Year                    | 0.200  | 0.149   |
| 100-Year                   | 0.241  | 0.186   |
| 24-Hour 15 min SCS Type II |  |   |
| 2-Year                     | 0.144  | 0.116   |
| 5-Year                     | 0.241  | 0.203   |
| 10-Year                    | 0.313  | 0.251   |
| 25-Year                    | 0.407  | 0.351   |
| 50-Year                    | 0.479  | 0.446   |
| 100-Year                   | 0.554  | 0.525   |

**Outlet #4**

**Table 7** presents the pre-development and post-development peak flows to outlet #4. As shown, the post-development flows are less than the pre-development flows for all storms.

**Table 7: Summary of Pre- and Post-Development Peak Flows (Outlet #4)**

| Return Period (Years)      | Pre-Development<br>(m <sup>3</sup> /s) [2.29 ha] | Post-Development<br>(m <sup>3</sup> /s) [1.09 ha] |
|----------------------------|--|---|
| 3-Hour 10 min Chicago      |  |   |
| 2-Year                     | 0.047  | 0.046   |
| 5-Year                     | 0.093  | 0.087   |
| 10-Year                    | 0.131  | 0.117   |
| 25-Year                    | 0.186  | 0.157   |
| 50-Year                    | 0.230  | 0.189   |
| 100-Year                   | 0.278  | 0.222   |
| 24-Hour 15 min SCS Type II |  |   |
| 2-Year                     | 0.162  | 0.107   |
| 5-Year                     | 0.269  | 0.164   |

|          |       |       |
|----------|-------|-------|
| 10-Year  | 0.349 | 0.206 |
| 25-Year  | 0.455 | 0.261 |
| 50-Year  | 0.537 | 0.303 |
| 100-Year | 0.621 | 0.345 |

#### 7.5.4 Stormwater Quality

As mentioned in **Section 7.4**, the SWM strategy varies based on the post-development drainage area. For catchment POST-3, a treatment train approach is proposed to provide the enhanced 80% TSS removal required. This treatment train approach includes catchbasin shields, tree pits, bioretention, and permeable pavement. The contributing area to POST-1, POST-4 and TR-1 requires no quality control due to clean water areas (roofs and backlots of residential units).

For POST-2, POST-5 and SWMF, quality control will be provided by the SWM Facility. The conceptual design of the proposed SWM Facility has incorporated a permanent pool and a sediment forebay to provide "enhanced protection" (*Stormwater Management Planning and Design Manual*, Ministry of the Environment, 2003). A sediment forebay has been provided to facilitate enhanced quality treatment in conformance with MECP forebay design guidelines. See **Appendix E** for forebay sizing calculations.

The Subject Development drainage area for the SWM Facility is 21.55 ha with an associated imperviousness of 67%. As such, the minimum water quality volume for the stormwater wet pond is 219 m<sup>3</sup>/ha (*Stormwater Management Planning and Design Manual*, Ministry of the Environment, 2003). The total water quality volume consists of 179 m<sup>3</sup>/ha for the permanent pool and 40 m<sup>3</sup>/ha for extended detention. Erosion control will be achieved within the SWMF facility via the detention of the 25mm event for a minimum drawdown of 24 hours.

The required and provided extended detention and permanent pool values have been summarized in **Table 8**. Refer to **Appendix E** for the water quality and extended detention calculations.

**Table 8: Stormwater Management Facility Quality and Erosion Control**

|                        | SWM Facility                      |                                   |
|------------------------|-----------------------------------|-----------------------------------|
|                        | Required Volume (m <sup>3</sup> ) | Provided Volume (m <sup>3</sup> ) |
| Permanent Pool         | 3852                              | 3990                              |
| MOE Extended Detention | 862                               | 3368                              |
| Erosion Control        | 3350                              |                                   |

#### 7.5.5 Stormwater Management Facility Operating Conditions

Considering the water quantity and quality storage requirements for the POST-2, POST-5 and SWMF drainage areas, a preliminary design for the SWM Facility has been completed to demonstrate that the SWM block is adequately sized. A preliminary operating profile of the SWM facility is presented in **Table 9**.

**Table 9: SWM Facility Operating Characteristics**

| Component                 | Elevation (m) | Storage Required (m <sup>3</sup> ) | Storage Provided (m <sup>3</sup> ) |
|---------------------------|---------------|------------------------------------|------------------------------------|
| Bottom                    | 516.62        | --                                 | --                                 |
| Permanent Pool            | 518.20        | 3852                               | 3990                               |
| Extended Detention        | 519.03        | 3350                               | 3368                               |
| 100-Year High Water Level | 520.45        | 12720                              | 13041                              |
| Top of Berm               | 520.75        | --                                 | 15858                              |

As evidenced by Error! Reference source not found., the pond presented herein is sufficiently sized to provide the required stormwater quantity and quality controls. Permits and other regulatory instruments such as an Environmental Compliance Approval (MECP) and Conservation Authority approval will be secured at the detailed design stage.

## 7.6 Water Balance

A feature-based water balance was completed for the subject site due to the proximity of nearby wetland features. The pre-development subject site was delineated into five drainage areas as mentioned in **Section 7.2**. The pre-development water balance was then used to determine the annual pre-development runoff volumes to each drainage area.

In post-development conditions, runoff is increased by 147% annually over the entire subject site compared with pre-development conditions. To reduce the annual runoff in post-development conditions, mitigation measures were introduced on-site via the proposed LIDs. These mitigation measures are proposed to reduce the amount of runoff by volume draining to each of the outlets that feed downstream wetlands.

Due to the soil strata underlying the native soils and groundwater monitoring results, there are considerable constraints to implementing an infiltration system(s) across the subject site. Shallow low-impact development (LID) measures were explored for this reason.

To increase infiltration and reduce runoff, the following LID measures are proposed for the Subject Development. Please refer to **Appendix F** for the LID-specific calculations. The proposed LID locations have been presented in **Figure 10**.

- Tree Pits

Tree pits are proposed throughout the Subject Development within the road right of way between the sidewalk and the street. Runoff draining from the right of way and front lots will drain to the tree pits. Based on the proposed layout and contributing drainage area, the tree pits would mitigate 14,508 m<sup>3</sup> of infiltration per year in post-development conditions. A minimum clearance of 1m is provided between the bottom of the LID and groundwater elevation. In the sizing of the tree pits, a minimum infiltration rate of 12mm/hr was assumed with a safety factor of 2 giving a design infiltration rate of 6mm/hr.

- Bioretention Cells

Three bioretention cells are proposed within the park area. Runoff from the park will be directed to the bioretention cells where filtered runoff can infiltrate into the native soils. Based on the proposed sizing and contributing drainage area, the bioretention cells would mitigate 2,295m<sup>3</sup> of infiltration annually in post-development conditions. A minimum clearance of 1m is provided between the

bottom of the LID and groundwater elevation. In the sizing of the bioretention cells, a minimum infiltration rate of 12mm/hr was assumed with a safety factor of 2 giving a design infiltration rate of 6mm/hr.

- Permeable Pavement

Permeable pavement is proposed within the park area walkways and walkways along the north site boundary. For the park permeable pavement, runoff from the park will be directed to the permeable paver walkways where filtered runoff can infiltrate into the native soils. For the north walkway permeable pavers, single-residential backlots will be directed to the permeable pavement walkway where filtered runoff can infiltration into native soils. Based on the proposed sizing and contributing drainage area, the permeable pavement in the park would mitigate 2,489 m<sup>3</sup> of infiltration annually in post-development conditions. Based on the proposed sizing and contributing drainage area, the permeable pavement at the north site boundary would mitigate 2,316 m<sup>3</sup> of infiltration annually in post-development conditions. A minimum clearance of 1m is provided between the bottom of the LIDs and groundwater elevation. In the sizing of the permeable pavement, a minimum infiltration rate of 12mm/hr was assumed with a safety factor of 2 giving a design infiltration rate of 6mm/hr.

- Enhanced Topsoil Depth

The fourth mitigation strategy proposed for the Subject Development is the use of enhanced topsoil within pervious areas across the site to promote infiltration. A topsoil depth of 300 mm will be required across the pervious area thereby providing increased opportunities for infiltration in the topsoil layout. The expected increased topsoil depth (300 mm) is expected to reduce runoff volume by 25% based on HSG 'C' type soil as per the TRCA and CVC LID manual. The use of enhanced topsoil would mitigate 6,409 m<sup>3</sup> of infiltration annually in post-development conditions.

With the mitigation measures, the following annual runoff is expected in post-development conditions:

- Runoff draining to outlet #1 (CP Trail) decreased by 24% (2,634 m<sup>3</sup>/year),
- Runoff draining to outlet #2 (north tile drain) increased by 233% (80,244 m<sup>3</sup>/year),
- Runoff draining to outlet #3 (east tile drain) decreased by 0% (10 m<sup>3</sup>/year), and,
- Runoff draining to outlet #4 (southeast tile drain) increased by 6% (344 m<sup>3</sup>/year).

Due to site constraints, the runoff draining to outlet #2 in post-development conditions increased by 233% (by volume) compared to pre-development conditions. Due to the expected increase of runoff volume in post-development conditions, additional studies by a geomorphologist will be explored to analyze the receiving capacity of the northeast wetland.

## 8.0 UTILITIES

The development will be serviced with natural gas, telephone, cable TV and hydro. All such utilities are available in the area of development. Coordination for extension of and connection to existing services will be undertaken as development approvals advance. Utilities are proposed to follow the alignment of the internal road network, with individual service connections to each lot.

## 9.0 CONCLUSIONS & RECOMMENDATIONS

Based on the foregoing, we conclude that Glenelg Phase 3 Development can be adequately serviced.

- Access to the Subject Development will be provided by two entrances from Glenelg Residential Development – Phase 2 and one entrance through Bradley Street. The internal roadways will meet Township Standards and provide access for emergency vehicles.



- Gravity sanitary services for the Subject Development will be provided via three connections, one to the White Rose Phase 3 Development and two to the Glenelg Phase 2 Development, further refinement of the sanitary design detailed design may be required to reduce overall site fill requirements.
- An internal watermain will be a looped system and will be provided through two connections at Glenelg Residential Development – Phase 2 and one connection at White Rose Phase 3.
- The development will be fully serviced by hydro, natural gas, cable, and telecommunications.
- The proposed LID measures and the Stormwater Management Facility will provide quality control for the subject site. The proposed LID measures and SWM Facility is adequately sized to provide “enhanced protection” level treatment. Quantity control is met for the subject site by controlling post-development flows to pre-development levels for all storms up to the 100-year storm event. The SWM facility will incorporate a minimum 24-hour retention of the 25mm event to provide erosion control.
- Due to site constraints, the runoff draining to outlet #2 in post-development conditions increased by 233% (by volume) compared to pre-development conditions. Due to the expected increase of runoff volume in post-development conditions, additional studies by a geomorphologist will be explored to analyze the receiving capacity of the northeast wetland.
- Natural hazard constraints (floodplain) do not exist within the Subject Development.
- Further study of the impacts to the downstream drainage features including ecological and geomorphological investigations are underway to confirm SWM Facility outfall design.

Based on the above, we recommend approval of the Planning Applications for the Subject Lands from the perspective of engineering servicing requirements.

**C.F. CROZIER & ASSOCIATES INC.**



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## APPENDICES

**Appendix A:** Sanitary Demand & WWTP Capacity Calculations

**Appendix B:** Water Demand & WTP Capacity Calculations

**Appendix C:** Hydrologic Parameter Sheets

**Appendix D:** VO6 Model Input & Output Files

**Appendix E:** SWM Facility Calculations

**Appendix F:** Water Balance Calculations

**Appendix G:** Glenelg Phase 2 Lands Design Excerpts

## LIST OF FIGURES

**Figure 1:** Site Location Plan

**Figure 2:** Draft Plan of Subdivision

**Figure 3:** Preliminary Grading Plan

**Figure 4:** General Site Servicing Plan

**Figure 5:** Pre-Development Drainage Plan

**Figure 6:** Existing Tile Drains and Pre-Development Drainage

**Figure 7:** Conservation Areas and Pre-Development Drainage Plan

**Figure 8:** Post-Development Drainage Plan

**Figure 9:** Preliminary SWMF #1

**Figure 10:** Proposed LID Plan #1

# APPENDIX A

## Sanitary Demand & WWTP Capacity Calculations

### Dundalk Sanitary Capacity Evaluation

| DESCRIPTION   | MARCH 2022          | POST WWTP UPGRADES | UNITS              |
|---|---------------------|--------------------|--------------------|
| Available Capacity  | 1,832               | 3,025              | m3/day             |
| Average Day Flow  | 1,165               | 1,165              | m3/day             |
| Reserve Capacity  | 667                 | 1,860              | m3/day             |
| Serviced Households   | 1,144               | 1,144              | ERUs               |
| Persons Per New <b>Equivalent Residential Unit</b> (2017 DC Background Study)                       | 2.66                | 2.66               | Persons            |
| Average New Development Per Capita Flow   | 0.35                | 0.35               | m3/day             |
| Equivalent Flow Per Residential Unit  | 0.931               | 0.931              | m3/day             |
| <b>Additional ERUs that can be serviced</b>   | <b><u>716</u></b>   | <b><u>1997</u></b> | <b>ERUs</b>        |
| <b>TOTAL EQUIVALENT RESIDENTIAL UNIT (ERU) SUMMARY OF OCCUPIED, COMMITTED AND UNCOMMITTED UNITS</b> |                     |                    |                    |
| DEVELOPMENT   | OCCUPIED UNITS 2021 | COMMITTED UNITS    | UNCOMMITTED UNITS  |
| White Rose (Phase 1 & 2)  | 63                  | 3                  | 0                  |
| Flato North (Phase 2A)  | 72                  | 0                  | 0                  |
| Flato North (Phase 3)   | 42                  | 4                  | 0                  |
| Flato North (Phase 4)   | 22                  | 0                  | 0                  |
| Flato North (Phase 5)   | 10                  | 49                 | 0                  |
| Flato North (Phase 6)   | 68                  | 48                 | 0                  |
| Glenelg (Phase 1)   | 0                   | 183                | 0                  |
| Flato West Block 75   | 0                   | 56                 | 0                  |
| Flato East (Phase 7, 8 & 10)  | 0                   | 188                | 0                  |
| Infill Lots   | 3                   | 3                  | 0                  |
| <b>TOTAL COMMITTED UNITS 2021</b>   |                     | <b><u>534</u></b>  | <b><u>1463</u></b> |
| White Rose (Phase 3)  | 0                   | 0                  | 88                 |
| Flato East (Phase 9)  | 0                   | 0                  | 47                 |
| Flato East (Phase 11)   | 0                   | 0                  | 193                |
| Glenelg (Phase 2)   | 0                   | 0                  | 155                |
| Dundalk Commercial  | 0                   | 0                  | 11                 |
| Dundalk North (Glenelg Expansion)   | 0                   | 0                  | 453                |
| <b>TOTAL UNCOMMITTED UNITS</b>  |                     |                    | <b><u>1396</u></b> |
| <b>Total Number of Available ERUs Upon Completion of WWTP Upgrades</b>                              |                     |                    | <b>1997</b>        |
| <b>Total Projected ERUs of Reserve Capacity Available Upon Occupation of Committed Units</b>        |                     |                    | <b>1463</b>        |
| <b>Projected ERUs of Reserve Capacity Available Upon Occupation of The Above Uncommitted Units</b>  |                     |                    | <b>67</b>          |



File: 1060-6220  
Date: 2022.08.26  
By: AM  
Check By: JL'A

### Glenelg Phase 3 Development (Bradley Street Connection) - Sanitary Design Criteria

|                             |               |                  |
|-----------------------------|---------------|------------------|
| Developed Site Area         | 10.38         | ha               |
| Number of Residential Units |               |                  |
| Single Residential          | 134           | units            |
| Townhouse                   | 72            | units            |
|                             | <b>TOTAL:</b> | <b>206 units</b> |
| Person Per Unit             | 2.66          | persons/unit     |
| Residential Population      | 548           | persons          |

#### Unit Sewage flows

|  |      |         |
|--|------|---------|
| Residential (Per New Development Unit Flow Rates, Triton Engineering (2022)) | 350  | L/C-day |
| Infiltration (typical)   | 0.15 | L/s/ha  |

#### Total Design Sewage Flows

|  |              |              |
|--|--------------|--------------|
| Infiltration/Inflow Residential          | 1.56         | L/sec        |
| Average Daily Residential Flow           | 2.22         | L/sec        |
| Residential Peak Factor (Harmon Formula) | 4.0          |              |
| <b>Total Peak Daily Flow</b>             | <b>10.33</b> | <b>L/sec</b> |



File: 1060-6220  
Date: 2022.08.26  
By: AM  
Check By: JL'A

### Glenelg Phase 3 Development (Glenelg Connection) - Sanitary Design Criteria

|  |                         |
|--|-------------------------|
| Developed Site Area  | 12.76 ha                |
| Number of Residential Units  |                         |
| Single Residential   | 229 units               |
| Semi   | 18                      |
| Townhouse  | - units                 |
|  | <b>TOTAL: 247 units</b> |
| Person Per Unit  | 2.66 persons/unit       |
| Residential Population   | 657 persons             |
| <b><u>Unit Sewage flows</u></b>  |                         |
| Residential (Per New Development Unit Flow Rates, Triton Engineering (2022)) | 350 L/C-day             |
| Infiltration (typical)   | 0.15 L/s/ha             |
| <b><u>Total Design Sewage Flows</u></b>                                      |                         |
| Infiltration/Inflow Residential  | 1.91 L/sec              |
| Average Daily Residential Flow   | 2.66 L/sec              |
| Residential Peak Factor (Harmon Formula)                                     | 3.9                     |
| <b>Total Peak Daily Flow</b>   | <b>12.32 L/sec</b>      |

### GLENELG PHASE 3 RESIDENTIAL DEVELOPMENT - EXTERNAL

| DESIGN: AM<br>CHECK: JL'A<br>SUBMISSION: 1st FSRSWM |           |              | <b>Peak Factor (M) =</b> $1+(14/4+(P/1000)^{0.5})$<br><b>Avg. Daily/Capita Flow =</b> 350 L/cap.d<br><b>Q infiltration =</b> 0.15 L/ha.s |                |                |       |           |            |             | <b>N = 0.013</b><br><b>Population = 2.66</b> p.p.u. |                |              |              |                |              |
|---|-----------|--------------|--|----------------|----------------|-------|-----------|------------|-------------|---|----------------|--------------|--------------|----------------|--------------|
| CATCHMENT AREA                                      | FROM MH   | TO MH        | LENGTH (m)   | INC. AREA (Ha) | CUM. AREA (Ha) | LOTS  | INC. POP. | TOTAL POP. | PEAK FACTOR | AVG. FLOW (L/S)                                     | MAX FLOW (L/S) | INFIL. (L/S) | TOTAL INFIL. | COMBINED (L/S) |              |
| Phase 2   | 1         | SAN7-PH2     | SAN6-PH2   | 13.4           | 0.18           | 0.18  | 3         | 8          | 8           | 4.42  | 0.03           | 0.14         | 0.03         | 0.03           | <b>0.17</b>  |
|   | 2         | SAN6-PH2     | SAN5-PH2   | 67.3           | 0.62           | 0.8   | 13        | 35         | 43          | 4.33  | 0.17           | 0.75         | 0.12         | 0.12           | <b>0.87</b>  |
|   | 3         | SAN5-PH2     | SAN4-PH2   | 16.6           | 0.19           | 0.99  | 0         | 0          | 43          | 4.33  | 0.17           | 0.75         | 0.15         | 0.15           | <b>0.89</b>  |
|   | 4         | SAN4-PH2     | SAN3-PH2   | 96.2           | 0.63           | 1.62  | 9         | 24         | 67          | 4.29  | 0.27           | 1.16         | 0.24         | 0.24           | <b>1.40</b>  |
|   | 5         | SAN7-PH2     | SAN8-PH2   | 37             | 0.19           | 0.19  | 4         | 11         | 11          | 4.41  | 0.04           | 0.19         | 0.03         | 0.03           | <b>0.22</b>  |
|   | 6         | SAN8-PH2     | SAN9-PH2   | 67             | 0.46           | 0.65  | 7         | 19         | 29          | 4.36  | 0.12           | 0.52         | 0.10         | 0.10           | <b>0.61</b>  |
|   | 10        | SAN10-PH2    | SAN9-PH2   | 83.3           | 0.38           | 0.38  | 6         | 16         | 16          | 4.39  | 0.06           | 0.28         | 0.06         | 0.06           | <b>0.34</b>  |
|   | Phase 3   | SANPLUG2-PH3 | SAN9-PH2   |                | 6.65           | 6.65  | 122       | 325        | 325         | 4.06  | 1.31           | 5.34         | 1.00         | 1.00           | <b>6.34</b>  |
|   | 7         | SAN9-PH2     | SAN15-PH2  | 59.7           | 0.4            | 8.08  | 9         | 24         | 394         | 4.03  | 1.59           | 6.42         | 1.21         | 1.21           | <b>7.63</b>  |
|   | 8         | SAN15-PH2    | SAN3-PH2   | 58.9           | 0.42           | 8.5   | 11        | 29         | 423         | 4.01  | 1.71           | 6.87         | 1.28         | 1.28           | <b>8.15</b>  |
|   | 9         | SAN3-PH2     | SAN2-PH2   | 80             | 0.4            | 10.52 | 7         | 19         | 508         | 3.97  | 2.06           | 8.17         | 1.58         | 1.58           | <b>9.75</b>  |
|   | 11        | SAN10-PH2    | SAN14-PH2  | 87.7           | 0.62           | 0.62  | 19        | 51         | 51          | 4.31  | 0.20           | 0.88         | 0.09         | 0.09           | <b>0.98</b>  |
|   | 12        | SAN14-PH2    | SAN2-PH2   | 54.1           | 0.39           | 1.01  | 12        | 32         | 82          | 4.27  | 0.33           | 1.42         | 0.15         | 0.15           | <b>1.58</b>  |
|   | 13        | SAN2-PH2     | SAN1-PH2   | 80             | 0.35           | 11.88 | 6         | 16         | 606         | 3.93  | 2.46           | 9.65         | 1.78         | 1.78           | <b>11.44</b> |
|   | Phase 3   | SANPLUG1-PH3 | SAN11-PH2  |                | 6.1            | 6.1   | 125       | 333        | 333         | 4.06  | 1.35           | 5.47         | 0.92         | 0.92           | <b>6.38</b>  |
|   | 14        | SAN10-PH2    | SAN11-PH2  | 83.3           | 0.39           | 0.39  | 7         | 19         | 19          | 4.38  | 0.08           | 0.33         | 0.06         | 0.06           | <b>0.39</b>  |
|   | 19        | SANMH13      | SAN12-PH2  | 41.6           | 0.11           | 0.11  | 2         | 5          | 5           | 4.44  | 0.02           | 0.10         | 0.02         | 0.02           |              |
|   | 17        | SAN12-PH2    | SAN11-PH2  | 41.6           | 0.24           | 0.24  | 2         | 5          | 5           | 4.44  | 0.02           | 0.10         | 0.04         | 0.04           | <b>0.13</b>  |
| 15  | SAN11-PH2 | SAN13-PH2    | 88.1   | 0.62           | 7.35           | 15    | 40        | 396        | 4.02        | 1.61  | 6.46           | 1.10         | 1.10         | <b>7.56</b>    |              |
| 16  | SAN13-PH2 | SAN1-PH2     | 77.7   | 0.58           | 7.93           | 15    | 40        | 436        | 4.00        | 1.77  | 7.08           | 1.19         | 1.19         | <b>8.27</b>    |              |
| 18  | SAN1-PH2  | SAN PLUG1    | 61   | 0.34           | 20.15          | 6     | 16        | 1059       | 3.78        | 4.29  | 16.23          | 3.02         | 3.02         | <b>19.25</b>   |              |



### GLENELG PHASE 3 RESIDENTIAL DEVELOPMENT - EXTERNAL

DESIGN: AM  
CHECK: JL'A  
SUBMISSION: 1st FSRSWM

**Peak Factor (M) =**  $1+(14/4+(P/1000)^{0.5})$   
**Avg. Daily/Capita Flow =** 350 L/cap.d  
**Q infiltration =** 0.15 L/ha.s

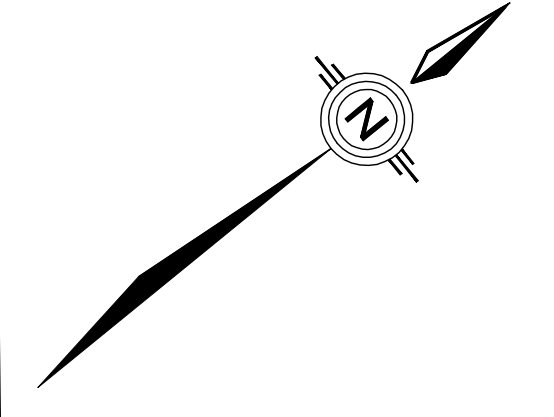
**N = 0.013**  
**Population = 2.66** p.p.u.

| CATCHMENT AREA                  | FROM MH   | TO MH    | LENGTH (m) | INC. AREA (Ha) | CUM. AREA (Ha) | LOTS  | INC. POP. | TOTAL POP. | PEAK FACTOR | AVG. FLOW (L/S) | MAX FLOW (L/S) | INFIL. (L/S) | TOTAL INFIL. | COMBINED (L/S) |              |
|---------------------------------|-----------|----------|------------|----------------|----------------|-------|-----------|------------|-------------|-----------------|----------------|--------------|--------------|----------------|--------------|
|                                 | 14, 15    | SANMH13  | SANMH6     | 83.29          | 1.18           | 1.18  | 11        | 29         | 29          | 4.36            | 0.12           | 0.52         | 0.18         | 0.18           | <b>0.69</b>  |
| 13                              | SANMH13   | SANMH1   | 93.96      | 0.6            | 0.6            | 14    | 37        | 37         | 4.34        | 0.15            | 0.65           | 0.09         | 0.09         | <b>0.74</b>    |              |
| 1                               | SANMH1    | SANMH2   | 95.07      | 0.58           | 1.18           | 14    | 37        | 74         | 4.28        | 0.30            | 1.29           | 0.18         | 0.18         | <b>1.47</b>    |              |
| All of Phase 2 and some Phase 3 | SANPLUG 1 | SANMH2   | 19         | 20.15          | 20.15          | 0     | 0         | 1059       | 3.78        | 4.29            | 16.23          | 3.02         | 3.02         | <b>19.25</b>   |              |
| 2                               | SANMH2    | SANMH3   | 80         | 0.79           | 22.12          | 12    | 32        | 1165       | 3.76        | 4.72            | 17.73          | 3.32         | 3.32         | <b>21.05</b>   |              |
| 16                              | SANMH14   | SANMH6   | 87.81      | 0.58           | 0.58           | 11    | 29        | 29         | 4.36        | 0.12            | 0.52           | 0.09         | 0.09         | <b>0.60</b>    |              |
| 5A                              | SANMH6    | SANMH5A  | 26.09      | 0.1            | 1.86           | 1     | 3         | 61         | 4.30        | 0.25            | 1.06           | 0.28         | 0.28         | <b>1.34</b>    |              |
| 5                               | SANMH5A   | SANMH5   | 57.2       | 0.37           | 2.23           | 8     | 21        | 82         | 4.27        | 0.33            | 1.42           | 0.33         | 0.33         | <b>1.76</b>    |              |
| 4                               | SANMH5    | SANMH4   | 64.46      | 0.38           | 2.61           | 12    | 32        | 114        | 4.23        | 0.46            | 1.96           | 0.39         | 0.39         | <b>2.35</b>    |              |
| 3                               | SANMH4    | SANMH3   | 64.46      | 0.26           | 2.87           | 7     | 19        | 133        | 4.21        | 0.54            | 2.27           | 0.43         | 0.43         | <b>2.70</b>    |              |
| 7                               | SANMH3    | SANMH8   | 80         | 0.56           | 25.55          | 10    | 27        | 1325       | 3.72        | 5.37            | 19.95          | 3.83         | 3.83         | <b>23.78</b>   |              |
| 17                              | SANMH14   | SANMH15  | 87.66      | 0.64           | 0.64           | 12    | 32        | 32         | 4.35        | 0.13            | 0.56           | 0.10         | 0.10         | <b>0.66</b>    |              |
| 18                              | SANMH15   | SANMH16  | 18.59      | 0.11           | 0.75           | 0     | 0         | 32         | 4.35        | 0.13            | 0.56           | 0.11         | 0.11         | <b>0.68</b>    |              |
| 19                              | SANMH16   | SANMH12  | 66.74      | 0.43           | 1.18           | 4     | 11        | 43         | 4.33        | 0.17            | 0.75           | 0.18         | 0.18         | <b>0.92</b>    |              |
| 21                              | SANMH18   | SANMH17  | 25.8       | 0.33           | 0.33           | 6     | 16        | 16         | 4.39        | 0.06            | 0.28           | 0.05         | 0.05         | <b>0.33</b>    |              |
| 20                              | SANMH17   | SANMH12  | 24.74      | 0.16           | 0.49           | 2     | 5         | 21         | 4.38        | 0.09            | 0.38           | 0.07         | 0.07         | <b>0.45</b>    |              |
| 12                              | SANMH12   | SANMH11  | 58.04      | 0.34           | 2.01           | 6     | 16        | 80         | 4.27        | 0.32            | 1.38           | 0.30         | 0.30         | <b>1.68</b>    |              |
| 11                              | SANMH11   | SANMH7   | 58.03      | 0.32           | 2.33           | 5     | 13        | 93         | 4.25        | 0.38            | 1.60           | 0.35         | 0.35         | <b>1.95</b>    |              |
| 6                               | SANMH5    | SANMH7   | 83.29      | 0.48           | 0.48           | 7     | 19        | 19         | 4.38        | 0.08            | 0.33           | 0.07         | 0.07         | <b>0.40</b>    |              |
| 10                              | SANMH7    | SANMH10  | 76.05      | 0.46           | 3.27           | 13    | 35        | 146        | 4.19        | 0.59            | 2.49           | 0.49         | 0.49         | <b>2.98</b>    |              |
| 9                               | SANMH10   | SANMH8   | 76.05      | 0.43           | 3.7            | 12    | 32        | 178        | 4.17        | 0.72            | 3.01           | 0.56         | 0.56         | <b>3.56</b>    |              |
| 8                               | SANMH8    | SANMH9   | 65.5       | 0.43           | 29.68          | 8     | 21        | 1524       | 3.67        | 6.17            | 22.69          | 4.45         | 4.45         | <b>27.14</b>   |              |
| 8A                              | SANMH9    | SANMH106 | 69.95      | 0.45           | 30.13          | 8     | 21        | 1545       | 3.67        | 6.26            | 22.98          | 4.52         | 4.52         | <b>27.50</b>   |              |
| External                        | MH153     | SANMH106 | SANMH105   | 65.85          | 0.00           | 30.13 | 0         | 0          | 1545        | 3.67            | 6.26           | 22.98        | 4.52         | 4.52           | <b>27.50</b> |

NOTE: SANITARY MH 100 SERIES REPRESENT EXTERNAL MANHOLES



# TOWNSHIP OF SOUTHGATE

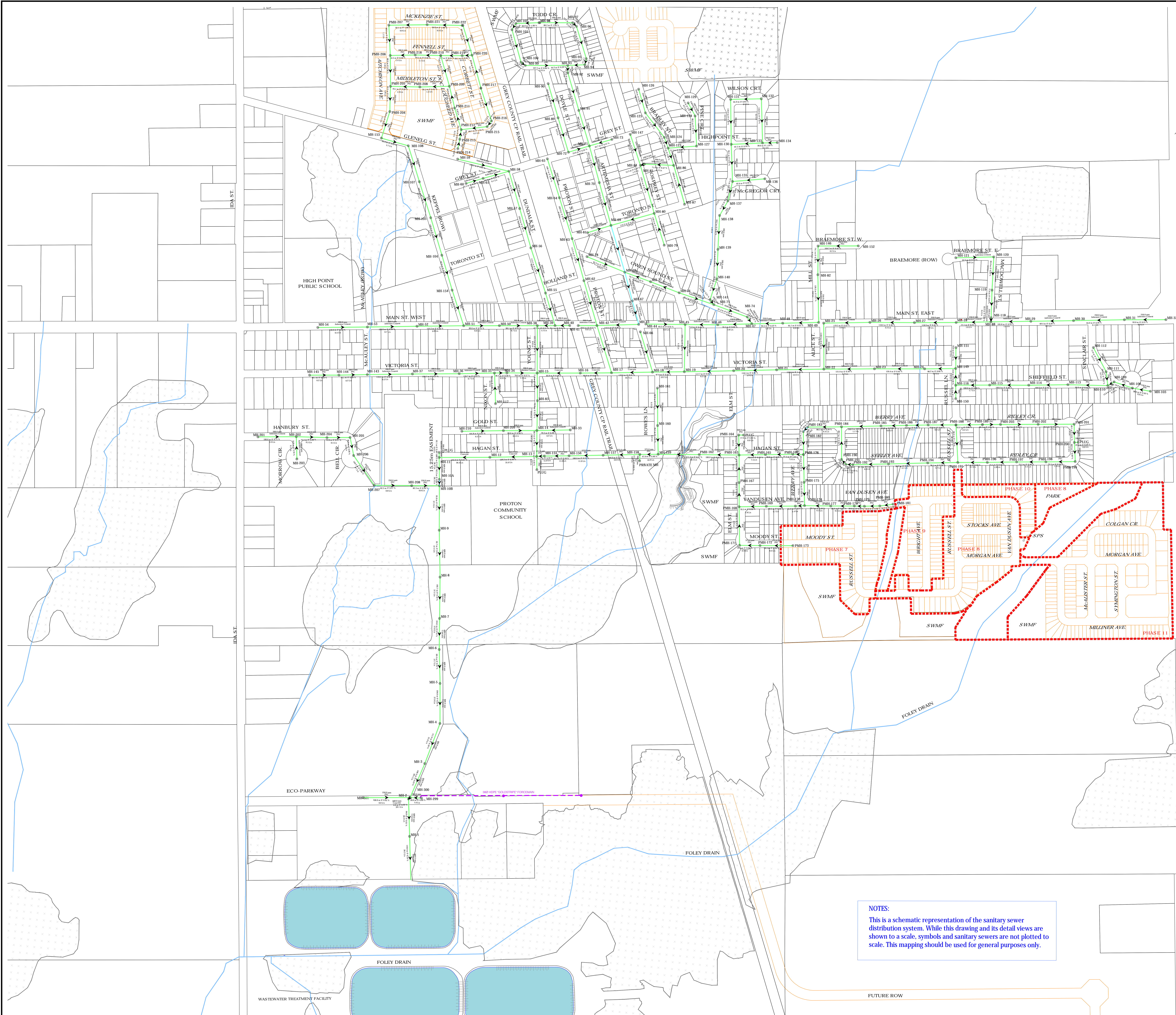


KING'S HIGHWAY No. 10

KING'S HIGHWAY No. 10

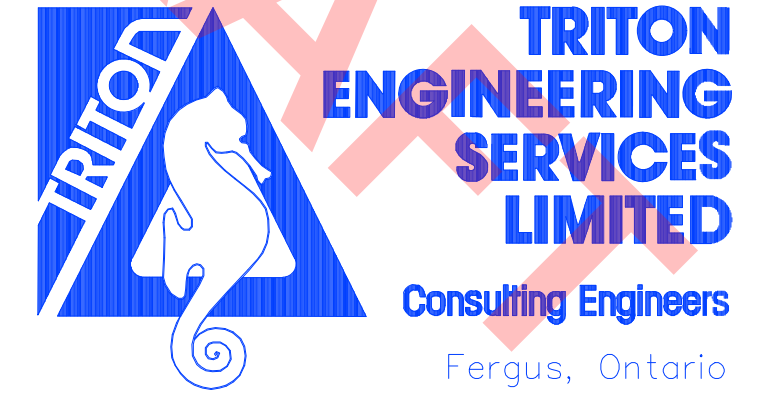
### LEGEND:

- PROPERTY**
  - EXISTING PROPERTY LINE
  - - - DRAFT PLAN APPROVED DEVELOPMENT PARCEL
  - - - DRAFT PLAN APPROVED DEVELOPMENT PHASES
  - WATER COURSES
  - WETLAND (GRCA)
- MANHOLES**
  - ⊙ SANITARY MANHOLE
  - ⊙ MH 159 MUNICIPALLY OWNED MANHOLE NUMBER
  - ⊙ PMH 200 MANHOLE NOT YET ADOPTED BY MUNICIPALITY
- FLOW/CAPACITY (%)**
  - < 20.0
  - < 40.0
  - < 60.0
  - < 80.0
  - < 100.0
- PIPE MATERIAL**
  - AC ASBESTOS CEMENT
  - CONC CONCRETE
  - PVC POLYVINYL CHLORIDE
- DIAMETER MATERIAL**
  - LENGTH @ SLOPE — FLOW/CAPACITY
- FOLEY**
  - IN-LINE CLEAN OUT AND SHUT-OFF VALVE
  - FORCEMAIN



## DUNDALK SANITARY COLLECTION SYSTEM

FEBRUARY 2022  
 SCALE 1:3,000  
 A4179 R02 / A4160 R44/R45







DESIGNED BY: AM  
 REVIEWED BY: JLA  
 DATE: 2022.08.26

**DOWN STREAM SANITARY SEWER CAPACITY ANALYSIS - GLENELG PHASE 3**

| CATCHMENT AREA   | FROM MH  | TO MH        | LENGTH (m)   | COMBINED (L/S) | Combined + additional | DIA. (mm)    | SLOPE (%)  | CAP. (l/s)    | VEL. (m/s)   | Percent Full |
|--|--|--------------|--------------|----------------|-----------------------|--------------|------------|---------------|--------------|--------------|
| <b>Additional flow coming into MH-32<br/>From Hwy 10</b>         | MH-32  | MH-31        | 93           | 0              | 0                     | 250          | 0.25%      | <b>29.73</b>  | 0.61         | 0%           |
|  | MH-31  | MH-30        | 93           | 0.1            | 0.1                   | 250          | 0.26%      | <b>30.32</b>  | 0.62         | 0%           |
|  | MH-30  | MH-29        | 99.4         | 0.5            | 0.5                   | 250          | 0.26%      | <b>30.32</b>  | 0.62         | 2%           |
|  | MH-29  | <b>MH-88</b> | 105.8        | 1.2            | 1.2                   | 250          | 0.29%      | <b>32.02</b>  | 0.65         | 4%           |
| <b>Additional flow coming into MH-120<br/>From Braemore St E</b> | MH-120   | MH-119       | 85           | 0.3            | 0.3                   | 250          | 0.28%      | <b>31.47</b>  | 0.64         | 1%           |
|  | MH-119   | MH-118       | 71.6         | 0.3            | 0.3                   | 250          | 0.29%      | <b>32.02</b>  | 0.65         | 1%           |
|  | MH-118   | <b>MH-88</b> | 10.4         | 0.4            | 0.4                   | 250          | 2.03%      | <b>84.73</b>  | 1.73         | 0%           |
|  | <b>MH-88</b>   | MH-28        | 77.1         | 1.6            | 1.6                   | 250          | 0.23%      | <b>28.52</b>  | 0.58         | 6%           |
|  | MH-28  | MH-27        | 118.9        | 1.9            | 1.9                   | 250          | 0.42%      | <b>38.54</b>  | 0.79         | 5%           |
|  | MH-27  | MH-26        | 118.6        | 2.2            | 2.2                   | 250          | 0.40%      | <b>37.61</b>  | 0.77         | 6%           |
|  | MH-26  | MH-25        | 119.5        | 2.4            | 2.4                   | 250          | 0.43%      | <b>39.00</b>  | 0.79         | 6%           |
|  | MH-25  | MH-22        | 118.9        | 2.7            | 2.7                   | 250          | 0.45%      | <b>39.89</b>  | 0.81         | 7%           |
|  | MH-22  | MH-21        | 118.6        | 5.6            | 5.6                   | 250          | 1.43%      | <b>71.11</b>  | 1.45         | 8%           |
|  | MH-21  | MH-20        | 118.3        | 5.9            | 5.9                   | 300          | 0.26%      | <b>49.31</b>  | 0.70         | 12%          |
|  | MH-20  | <b>MH-19</b> | 117.7        | 6.2            | 6.2                   | 300          | 0.45%      | <b>64.87</b>  | 0.92         | 10%          |
| <b>Additional flow coming into MH-152<br/>From Braemore St W</b> | MH-152   | MH-146       | 104.2        | 0.2            | 0.2                   | 250          | 0.42%      | <b>38.54</b>  | 0.79         | 1%           |
|  | MH-146   | MH-82        | 74.1         | 0.4            | 0.4                   | 250          | 0.46%      | <b>40.33</b>  | 0.82         | 1%           |
|  | MH-82  | MH-49        | 126.2        | 0.6            | 0.6                   | 250          | 0.27%      | <b>30.90</b>  | 0.63         | 2%           |
|  | MH-49  | MH-48        | 95.7         | 0.9            | 0.9                   | 250          | 0.22%      | <b>27.89</b>  | 0.57         | 3%           |
|  | MH-48  | <b>MH-47</b> | 94.5         | 1.2            | 1.2                   | 250          | 0.33%      | <b>34.16</b>  | 0.70         | 4%           |
| <b>Additional flow coming into MH-134<br/>From Highpoint St</b>  | MH-134   | MH-133       | 38           | 0.1            | 0.1                   | 200          | 1.29%      | <b>37.25</b>  | 1.19         | 0%           |
|  | MH-133   | MH-130       | 81.5         | 0.4            | 0.4                   | 200          | 0.59%      | <b>25.19</b>  | 0.80         | 2%           |
|  | MH-130   | MH-135       | 94.1         | 1.2            | 1.2                   | 200          | 0.45%      | <b>22.00</b>  | 0.70         | 5%           |
|  | MH-135   | MH-137       | 57.1         | 1.5            | 1.5                   | 200          | 0.49%      | <b>22.96</b>  | 0.73         | 7%           |
|  | MH-137   | MH-138       | 48.4         | 1.6            | 1.6                   | 200          | 0.37%      | <b>19.95</b>  | 0.64         | 8%           |
|  | MH-138   | MH-139       | 99.2         | 1.6            | 1.6                   | 200          | 0.36%      | <b>19.68</b>  | 0.63         | 8%           |
|  | MH-139   | MH-140       | 61.5         | 1.6            | 1.6                   | 200          | 0.44%      | <b>21.76</b>  | 0.69         | 7%           |
|  | MH-140   | MH-141       | 53.5         | 1.6            | 1.6                   | 200          | 0.41%      | <b>21.00</b>  | 0.67         | 8%           |
|  | MH-141   | MH-75        | 14.4         | 1.6            | 1.6                   | 200          | 0.21%      | <b>15.03</b>  | 0.48         | 11%          |
|  | MH-75  | MH-74        | 91.4         | 1.8            | 1.8                   | 200          | 0.39%      | <b>20.48</b>  | 0.65         | 9%           |
|  | MH-74  | <b>MH-47</b> | 20.7         | 2.4            | 2.4                   | 200          | 0.77%      | <b>28.78</b>  | 0.92         | 8%           |
|  | <b>MH-47</b>   | MH-46        | 79.3         | 3.8            | 3.8                   | 250          | 0.42%      | <b>38.54</b>  | 0.79         | 10%          |
|  | MH-46  | MH-45        | 71.6         | 3.9            | 3.9                   | 250          | 0.40%      | <b>37.61</b>  | 0.77         | 10%          |
|  | MH-45  | <b>MH-19</b> | 124.1        | 4.1            | 4.1                   | 250          | 0.40%      | <b>37.61</b>  | 0.77         | 11%          |
|  | <b>MH-19</b>   | MH-18        | 87.8         | 10.2           | 10.2                  | 375          | 0.42%      | <b>113.63</b> | 1.03         | 9%           |
|  | MH-18  | <b>MH-17</b> | 110.6        | 10.7           | 10.7                  | 375          | 0.41%      | <b>112.27</b> | 1.02         | 10%          |
|  | <b>Additional flow coming from Glenelg Phase 3<br/>From White Rose Phase 3</b> |              |              |                |                       | 10.33        |            |               |              |              |
| <b>From Bradely St</b>   |  |              |              |                | 3.26                  |              |            |               |              |              |
|  | MH-126   | MH-125       | 64           | 3.46           | 13.79                 | 200          | 0.44%      | <b>21.76</b>  | 0.69         | 63%          |
|  | MH-125   | MH-124       | 75           | 3.66           | 13.99                 | 200          | 0.52%      | <b>23.65</b>  | 0.75         | 59%          |
|  | MH-124   | MH-123       | 36           | 3.76           | 14.09                 | 200          | 0.42%      | <b>21.26</b>  | 0.68         | 66%          |
|  | MH-123   | MH-86        | 43.3         | 4.36           | 14.69                 | 200          | 0.46%      | <b>22.24</b>  | 0.71         | 66%          |
|  | MH-86  | MH-85        | 71.9         | 4.56           | 14.89                 | 200          | 0.47%      | <b>22.49</b>  | 0.72         | 66%          |
|  | MH-85  | MH-80        | 129.5        | 4.56           | 14.89                 | 200          | 0.41%      | <b>21.00</b>  | 0.67         | 71%          |
|  | MH-80  | MH-69        | 112.2        | 5.46           | 15.79                 | 200          | 0.43%      | <b>21.51</b>  | 0.68         | 73%          |
|  | <b>Toronto Street to Owen Sound Street Leg</b>                                 | <b>MH-69</b> | <b>MH-68</b> | <b>124.7</b>   | <b>9.16</b>           | <b>19.49</b> | <b>200</b> | <b>0.37%</b>  | <b>19.95</b> | <b>0.64</b>  |
|  | MH-68  | MH-67        | 71           | 9.56           | 19.89                 | 250          | 0.23%      | <b>28.52</b>  | 0.58         | 70%          |
|  | MH-67  | MH-43        | 69.8         | 9.56           | 19.89                 | 250          | 0.26%      | <b>30.32</b>  | 0.62         | 66%          |
|  | MH-43  | MH-42        | 112.5        | 9.66           | 19.99                 | 250          | 0.38%      | <b>36.66</b>  | 0.75         | 55%          |
|  | MH-42  | <b>MH-17</b> | 128.6        | 10.16          | 20.49                 | 250          | 0.43%      | <b>39.00</b>  | 0.79         | 53%          |
|  | <b>MH-17</b>   | MH-16        | 93           | 17.1           | 27.43                 | 450          | 0.40%      | <b>180.32</b> | 1.13         | 15%          |
|  | MH-16  | <b>MH-15</b> | 93.9         | 17.3           | 27.63                 | 450          | 0.39%      | <b>178.05</b> | 1.12         | 16%          |

**DOWN STREAM SANITARY SEWER CAPACITY ANALYSIS - GLENELG PHASE 3**

| CATCHMENT AREA                            | FROM MH                | TO MH        | LENGTH (m) | COMBINED (L/S) | Combined + additional | DIA. (mm) | SLOPE (%)     | CAP. (l/s)    | VEL. (m/s)   | Percent Full |    |
|---|------------------------|--------------|------------|----------------|-----------------------|-----------|---------------|---------------|--------------|--------------|----|
| <b>Additional flow coming into MH-153</b> |                        |              |            |                | <b>27.5</b>           |           |               |               |              |              |    |
| <b>From Glenelg</b>                       | MH-153                 | MH-108       | 65.9       | 0              | 27.5                  | 250       | 0.29%         | <b>32.02</b>  | 0.65         | 86%          |    |
|   | MH-108                 | MH-107       | 100        | 0              | 27.5                  | 250       | 0.30%         | <b>32.57</b>  | 0.66         | 84%          |    |
|   | MH-107                 | MH-105       | 100        | 0.1            | 27.6                  | 250       | 0.30%         | <b>32.57</b>  | 0.66         | 85%          |    |
|   | MH-105                 | MH-104       | 100        | 0.1            | 27.6                  | 250       | 0.30%         | <b>32.57</b>  | 0.66         | 85%          |    |
|   | MH-104                 | MH-154       | 92         | 0.2            | 27.7                  | 250       | 0.30%         | <b>32.57</b>  | 0.66         | 85%          |    |
|   | MH-154                 | MH-51        | 97         | 0.2            | 27.7                  | 250       | 0.30%         | <b>32.57</b>  | 0.66         | 85%          |    |
|   | MH-51                  | MH-50        | 89.6       | 1.5            | 29                    | 300       | 0.15%         | <b>37.45</b>  | 0.53         | 77%          |    |
|   | MH-50                  | MH-38        | 99.1       | 1.6            | 29.1                  | 300       | 0.22%         | <b>45.36</b>  | 0.64         | 64%          |    |
|   | MH-38                  | <b>MH-15</b> | 122.2      | 1.9            | 29.4                  | 300       | 0.36%         | <b>58.02</b>  | 0.82         | 51%          |    |
|   | <b>MH-15</b>           | MH-83        | 71.9       | 25.6           | 63.43                 | 525       | 0.39%         | <b>268.57</b> | 1.24         | 24%          |    |
|   | MH-83                  | MH-14        | 75.9       | 25.8           | 63.63                 | 525       | 0.20%         | <b>192.33</b> | 0.89         | 33%          |    |
|   | MH-14                  | MH-13        | 68         | 26.4           | 64.23                 | 525       | 0.29%         | <b>231.60</b> | 1.07         | 28%          |    |
|   | MH-13                  | MH-12        | 126.2      | 35.9           | 73.73                 | 525       | 0.36%         | <b>258.04</b> | 1.19         | 29%          |    |
|   | MH-12                  | MH-11        | 125.9      | 36.4           | 74.23                 | 525       | 0.37%         | <b>261.60</b> | 1.21         | 28%          |    |
|   | MH-11                  | MH-10A       | 80.2       | 36.4           | 74.23                 | 600       | 0.22%         | <b>288.00</b> | 1.02         | 26%          |    |
|   | MH-10A                 | MH-10B       | 13.3       | 36.5           | 74.33                 | 600       | 0.08%         | <b>173.67</b> | 0.61         | 43%          |    |
|   | <b>From Hanbury St</b> | MH-201       | MH-202     | 100.6          | 0.3                   | 0.3       | 200           | 0.71%         | <b>27.64</b> | 0.88         | 1% |
|   |                        | MH-202       | MH-204     | 72.5           | 0.5                   | 0.5       | 250           | 0.28%         | <b>31.47</b> | 0.64         | 2% |
| MH-204                                    |                        | MH-205       | 72.8       | 0.5            | 0.5                   | 250       | 0.27%         | <b>30.90</b>  | 0.63         | 2%           |    |
| MH-205                                    |                        | MH206        | 46.6       | 0.6            | 0.6                   | 250       | 0.28%         | <b>31.47</b>  | 0.64         | 2%           |    |
| MH-206                                    |                        | MH-207       | 104.2      | 0.7            | 0.7                   | 250       | 0.28%         | <b>31.47</b>  | 0.64         | 2%           |    |
| MH-207                                    |                        | MH-208       | 82.3       | 0.8            | 0.8                   | 250       | 0.27%         | <b>30.90</b>  | 0.63         | 3%           |    |
| MH-208                                    |                        | MH-10B       | 82.3       | 0.8            | 0.8                   | 250       | 0.28%         | <b>31.47</b>  | 0.64         | 3%           |    |
| MH-10B                                    |                        | MH-9         | 95.8       | 37.3           | 149.46                | 600       | 0.22%         | <b>288.00</b> | 1.02         | 52%          |    |
| MH-9                                      |                        | MH-8         | 92.7       | 37.4           | 149.56                | 600       | 0.25%         | <b>307.01</b> | 1.09         | 49%          |    |
| MH-8                                      |                        | MH-7         | 102.7      | 37.5           | 149.66                | 600       | 0.18%         | <b>260.50</b> | 0.92         | 57%          |    |
| MH-7                                      |                        | MH-6         | 104.5      | 37.6           | 149.76                | 600       | 0.14%         | <b>229.74</b> | 0.81         | 65%          |    |
| MH-6                                      |                        | MH-5         | 99.4       | 37.7           | 149.86                | 600       | 0.22%         | <b>288.00</b> | 1.02         | 52%          |    |
| MH-5                                      |                        | MH-4         | 104.6      | 37.8           | 149.96                | 600       | 0.25%         | <b>307.01</b> | 1.09         | 49%          |    |
| MH-4                                      |                        | MH-3         | 111.9      | 37.9           | 150.06                | 600       | 0.18%         | <b>260.50</b> | 0.92         | 58%          |    |
| MH-3                                      | MH-2                   | 106.4        | 38.1       | 150.26         | 600                   | 0.24%     | <b>300.80</b> | 1.06          | 50%          |              |    |
| <b>Wastewater Treatment Facility</b>      | MH-2                   | MH-1         | 94.8       | 39.2           | 151.36                | 600       | 0.25%         | <b>307.01</b> | 1.09         | 49%          |    |
|   | MH-1                   | WWTF         | 110        | 39.3           | 151.46                | 600       | 0.19%         | <b>267.64</b> | 0.95         | 57%          |    |

# APPENDIX B

## Water Demand & WTP Capacity Calculations



File: 1060-6220  
Date: 2022.08.26  
By: AM  
Check By: JL'A

### Glenelg Phase 3 Development - Domestic Water Design Criteria

|  |                    |
|--|--------------------|
| Developed Site Area  | 25.93 ha           |
| Number of Residential Units- Single Detached                                 | 363 units          |
| Number of Residential Units- Semi Detached                                   | 18                 |
| Number of Residential Units- Townhouse                                       | 72 units           |
| Total Number of Units  | 453 units          |
| Persons Per Unit   | 2.66 persons/unit  |
| Residential Population   | 1,205 persons      |
| <b><u>Domestic Water Design Flows</u></b>                                    |                    |
| Residential [Per New Development Unit Flow Rates, Triton Engineering (2022)] | 279 L/C-day        |
| <b><u>Total Domestic Water Design Flows</u></b>                              |                    |
| Average Residential Daily Flow   | 3.89 L/sec         |
| Max Day Peak Factor  | 2.75               |
| <b>Max Day Demand Flow</b>   | <b>10.70</b> L/sec |
| Peak Hour Factor   | 4.13               |
| <b>Peak Hour Flow</b>  | <b>16.07</b> L/sec |



Project: Glenelg Expansion  
 Project No.: 1060-6220  
 Date: 25-Aug-22  
 By: AM  
 Check: JL'A

**Dundalk Water System Capacity Evaluation**

| DESCRIPTION   | 2022               | UNITS       |
|---|--------------------|-------------|
| Available Capacity  | 2,817              | m3/day      |
| Max Day Flow  | 941                | m3/day      |
| Reserve Capacity  | 1,876              | m3/day      |
| Serviced Households   | 1,299              | ERUs        |
| Persons Per Existing Residential Unit (2016 Census Data)                      | 2.6                | Persons     |
| Maximum Day Per Capita Flow   | 0.279              | m3/day      |
| Persons Per New <b>Equivalent Residential Unit</b> (2017 DC Background Study) | 2.66               | Persons     |
| Additional population that can be served                                      | 6724               | Persons     |
| <b>Additional ERUs that can be served</b>                                     | <b><u>2528</u></b> | <b>ERUs</b> |

**TOTAL EQUIVALENT RESIDENTIAL UNIT (ERU) SUMMARY OF OCCUPIED, COMMITTED AND UNCOMMITTED UNITS**

| DEVELOPMENT  | OCCUPIED UNITS 2021 | COMMITTED UNITS   | UNCOMMITTED UNITS  |
|--|---------------------|-------------------|--------------------|
| White Rose (Phase 1 & 2)   | 63                  | 3                 | 0                  |
| Flato North (Phase 2A)   | 72                  | 0                 | 0                  |
| Flato North (Phase 3)  | 42                  | 4                 | 0                  |
| Flato North (Phase 4)  | 22                  | 0                 | 0                  |
| Flato North (Phase 5)  | 10                  | 49                | 0                  |
| Flato North (Phase 6)  | 20                  | 48                | 0                  |
| Glenelg (Phase 1)  | 0                   | 183               | 0                  |
| Flato West Block 75  | 0                   | 56                | 0                  |
| Flato East (Phase 7, 8 & 10)   | 0                   | 188               | 0                  |
| Infill Lots  | 3                   | 3                 | 0                  |
| <b>TOTAL COMMITTED UNITS 2021</b>  |                     | <b><u>534</u></b> | <b>0</b>           |
| White Rose (Phase 3)   | 0                   | 0                 | 88                 |
| Flato East (Phase 9)   | 0                   | 0                 | 47                 |
| Flato East (Phase 11)  | 0                   | 0                 | 193                |
| Glenelg (Phase 2)  | 0                   | 0                 | 155                |
| Dundalk Commercial   | 0                   | 0                 | 11                 |
| Dundalk North (Glenelg Expansion)  | 0                   | 0                 | 453                |
| <b>TOTAL UNCOMMITTED UNITS</b>   |                     |                   | <b><u>1396</u></b> |
| <b>Total Number of Available ERUs</b>  |                     |                   | <b>2528</b>        |
| <b>Total Projected ERUs of Reserve Capacity Available Upon Occupation of Committed Units</b>       |                     |                   | <b>1994</b>        |
| <b>Projected ERUs of Reserve Capacity Available Upon Occupation of The Above Uncommitted Units</b> |                     |                   | <b>598</b>         |

# APPENDIX C

## Hydrologic Parameter Sheets



Project Name: Glenelg Expansion Lands  
 Project Number: 1060-6220  
 Date: 2022-08-11  
 By: KS

**D.A. NAME** PRE-1  
**D.A. AREA (ha)** 4.32

**Hydrologic Parameters: CALIB NASHYD Command  
 Pre Development Drainage Area: Catchment PRE-1  
 Pre-Dev to CP Trail**

**Curve Number Calculation**

| Soil Types Present: |     |            |        |            |
|---------------------|-----|------------|--------|------------|
| Type                | ID  | Hydrologic | % Area | Area       |
| Listowel Silt Loam  | LTW | B          | 100.0% | 4.3        |
|                     |     |            |        | 0          |
|                     |     |            |        | 0          |
|                     |     |            |        | 0          |
| <b>Total Area</b>   |     |            |        | <b>4.3</b> |

**Impervious Landuses Present:**

| Soils                | Roadway  |    | Sidewalk |    | Driveway |    | Building  |    | SWMF     |    | Subtotals |      |
|----------------------|----------|----|----------|----|----------|----|-----------|----|----------|----|-----------|------|
|                      | Area     | CN | Area     | CN | Area     | CN | Area (ha) | CN | Area     | CN | Area      | A*CN |
| LTW                  | 0        | 98 | 0        | 98 | 0        | 98 | 0.000     | 98 | 0        | 98 | 0.00      | 0.00 |
|                      | 0        | 98 | 0        | 98 | 0        | 98 | 0         | 98 |          | 98 | 0         | 0    |
|                      | 0        | 98 |          | 98 |          | 98 |           | 98 |          | 98 | 0         | 0    |
|                      | 0        | 98 |          | 98 |          | 98 |           | 98 |          | 98 | 0         | 0    |
| <b>Subtotal Area</b> | <b>0</b> |    | <b>0</b> |    | <b>0</b> |    | <b>0</b>  |    | <b>0</b> |    |           |      |

**Pervious Landuses Present:**

| Soils                | Woodland    |    | Meadow      |    | Wetland     |    | Lawn        |    | Cultivated |    | Subtotals |        |
|----------------------|-------------|----|-------------|----|-------------|----|-------------|----|------------|----|-----------|--------|
|                      | Area        | CN | Area        | CN | Area        | CN | Area (ha)   | CN | Area       | CN | Area      | A*CN   |
| LTW                  | 0.00        |    | 0.00        |    | 0.00        |    | 0.00        |    | 4.3        | 74 | 4.32      | 319.68 |
|                      | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |    | 0.00      | 0.00   |
|                      | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |    | 0.00      | 0.00   |
|                      | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |    | 0.00      | 0.00   |
| <b>Subtotal Area</b> | <b>0.00</b> |    | <b>0.00</b> |    | <b>0.00</b> |    | <b>0.00</b> |    | <b>4.3</b> |    |           |        |

|                             |  |                         |            |
|-----------------------------|--|-------------------------|------------|
| Composite Area Calculations |  | Total Pervious Area     | 4.3        |
|                             |  | Total Impervious Area   | 0.0        |
|                             |  | % Impervious            | 0.0%       |
|                             |  | Composite Curve Number  | 74.0       |
|                             |  | <b>Total Area Check</b> | <b>4.3</b> |

**Initial Abstraction and Tp Calculations**

| Initial Abstraction |         |             |          | Composite Curve Number              |      |    |      |    |      |    |      |              |
|---------------------|---------|-------------|----------|-------------------------------------|------|----|------|----|------|----|------|--------------|
| Landuse             | IA (mm) | Area (ha)   | A * IA   | Listowel Silt                       |      | 0  |      | 0  |      | 0  |      | A*RC         |
|                     |         |             |          | RC                                  | Area | RC | Area | RC | Area | RC | Area |              |
| Woodland            | 10      | 0.00        | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0            |
| Meadow              | 8       | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0            |
| Wetland             | 16      | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0            |
| Lawn                | 5       | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0.000        |
| Cultivated          | 7       | 4           | 30.24    | 0.35                                | 4    |    | 0    |    | 0    |    | 0    | 1.512        |
| Impervious          | 2       | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0.000        |
| <b>Composite IA</b> |         | <b>4.32</b> | <b>7</b> | <b>Composite Runoff Coefficient</b> |      |    |      |    |      |    |      | <b>0.350</b> |

| Time to Peak Inputs   |            |          |           |                    |                | Uplands |        |               | Bransby Williams |        | Airport |        |
|-----------------------|------------|----------|-----------|--------------------|----------------|---------|--------|---------------|------------------|--------|---------|--------|
| Flow Path Description | Length (m) | Drop (m) | Slope (%) | V/S <sup>0.5</sup> | Velocity (m/s) | Tc (hr) | Tp(hr) | TOTAL Tp (hr) | Tc (hr)          | Tp(hr) | Tc (hr) | Tp(hr) |
| Overland              | 184.5      | 2        | 1.08%     | 2.7                | 0.28           | 0.18    | 0.12   | 0.12          | 0.15             | 0.10   | 0.54    | 0.36   |

Appropriate calculated time to 0.36 Appropriate Method: Airport





Project Name: Glenelg Expansion Lands  
 Project Number: 1060-6220  
 Date: 2022-08-11  
 By: KS

D.A. NAME **PRE-2**  
 D.A. AREA (ha) **13.33**

**Hydrologic Parameters: CALIB NASHYD Command  
 Pre Development Drainage Area: Catchment PRE-2  
 Pre-Dev to North Tile**

**Curve Number Calculation**

| Soil Types Present: |     |            |        |      |
|---------------------|-----|------------|--------|------|
| Type                | ID  | Hydrologic | % Area | Area |
| Listowel Silt Loam  | LTW | B          | 100.0% | 13.3 |
|                     |     |            |        | 0    |
|                     |     |            |        | 0    |
|                     |     |            |        | 0    |
| Total Area          |     |            |        | 13.3 |

| Impervious Landuses Present: |         |    |          |    |          |    |           |    |      |    |           |      |  |
|------------------------------|---------|----|----------|----|----------|----|-----------|----|------|----|-----------|------|--|
| Soils                        | Roadway |    | Sidewalk |    | Driveway |    | Building  |    | SWMF |    | Subtotals |      |  |
|                              | Area    | CN | Area     | CN | Area     | CN | Area (ha) | CN | Area | CN | Area      | A*CN |  |
| LTW                          | 0       | 98 | 0        | 98 | 0        | 98 | 0.000     | 98 | 0    | 98 | 0.00      | 0.00 |  |
|                              | 0       | 98 | 0        | 98 | 0        | 98 | 0         | 98 |      | 98 | 0         | 0    |  |
|                              | 0       | 98 |          | 98 |          | 98 |           | 98 |      | 98 | 0         | 0    |  |
|                              | 0       | 98 |          | 98 |          | 98 |           | 98 |      | 98 | 0         | 0    |  |
| Subtotal Area                | 0       |    | 0        |    | 0        |    | 0         |    | 0    |    | 0         |      |  |

| Pervious Landuses Present: |          |    |        |    |         |    |           |    |            |    |           |        |  |
|----------------------------|----------|----|--------|----|---------|----|-----------|----|------------|----|-----------|--------|--|
| Soils                      | Woodland |    | Meadow |    | Wetland |    | Lawn      |    | Cultivated |    | Subtotals |        |  |
|                            | Area     | CN | Area   | CN | Area    | CN | Area (ha) | CN | Area       | CN | Area      | A*CN   |  |
| LTW                        | 0.00     |    | 0.00   |    | 0.00    |    | 0.00      |    | 13.3       | 74 | 13.33     | 986.42 |  |
|                            | 0        |    | 0.00   |    | 0.00    |    | 0.00      |    | 0.00       |    | 0.00      | 0.00   |  |
|                            | 0        |    | 0.00   |    | 0.00    |    | 0.00      |    | 0.00       |    | 0.00      | 0.00   |  |
|                            | 0        |    | 0.00   |    | 0.00    |    | 0.00      |    | 0.00       |    | 0.00      | 0.00   |  |
| Subtotal Area              | 0.00     |    | 0.00   |    | 0.00    |    | 0.00      |    | 13.3       |    |           |        |  |

|                             |  |                        |      |
|-----------------------------|--|------------------------|------|
| Composite Area Calculations |  | Total Pervious Area    | 13.3 |
|                             |  | Total Impervious Area  | 0.0  |
|                             |  | % Impervious           | 0.0% |
|                             |  | Composite Curve Number | 74.0 |
| Total Area Check            |  |                        | 13.3 |

**Initial Abstraction and Tp Calculations**

| Initial Abstraction |         |           |        | Composite Curve Number       |      |    |      |    |      |   |   |       |
|---------------------|---------|-----------|--------|------------------------------|------|----|------|----|------|---|---|-------|
| Landuse             | IA (mm) | Area (ha) | A * IA | Listowel Silt                |      | 0  |      | 0  |      | 0 |   | A*RC  |
|                     |         |           |        | RC                           | Area | RC | Area | RC | Area |   |   |       |
| Woodland            | 10      | 0.00      | 0      |                              | 0    |    | 0    |    | 0    |   | 0 | 0     |
| Meadow              | 8       | 0         | 0      |                              | 0    |    | 0    |    | 0    |   | 0 | 0     |
| Wetland             | 16      | 0         | 0      |                              | 0    |    | 0    |    | 0    |   | 0 | 0     |
| Lawn                | 5       | 0         | 0      |                              | 0    |    | 0    |    | 0    |   | 0 | 0.000 |
| Cultivated          | 7       | 13        | 93.31  | 0.35                         | 13   |    | 0    |    | 0    |   | 0 | 4.666 |
| Impervious          | 2       | 0         | 0      |                              | 0    |    | 0    |    | 0    |   | 0 | 0.000 |
| Composite IA        |         | 13.33     | 7      | Composite Runoff Coefficient |      |    |      |    |      |   |   | 0.350 |

| Time to Peak Inputs   |            |          |           |                    |                | Uplands |        |               | Bransby Williams |        | Airport |        |
|-----------------------|------------|----------|-----------|--------------------|----------------|---------|--------|---------------|------------------|--------|---------|--------|
| Flow Path Description | Length (m) | Drop (m) | Slope (%) | V/S <sup>0.5</sup> | Velocity (m/s) | Tc (hr) | Tp(hr) | TOTAL Tp (hr) | Tc (hr)          | Tp(hr) | Tc (hr) | Tp(hr) |
| Overland              | 552.64     | 6        | 1.09%     | 2.7                | 0.28           | 0.55    | 0.37   | 0.37          | 0.40             | 0.27   | 0.93    | 0.62   |

Appropriate calculated time to **0.62** Appropriate Method: **Airport**



Project Name: Glenelg Expansion Lands  
 Project Number: 1060-6220  
 Date: 2022-08-11  
 By: KS

**D.A. NAME** PRE-3  
**D.A. AREA (ha)** 3.05

**Hydrologic Parameters: CALIB NASHYD Command**  
**Pre Development Drainage Area: Catchment PRE-3**  
**Pre-Dev to East Tile**

**Curve Number Calculation**

| Soil Types Present: |     |            |        |            |
|---------------------|-----|------------|--------|------------|
| Type                | ID  | Hydrologic | % Area | Area       |
| Listowel Silt Loam  | LTW | B          | 100.0% | 3.1        |
|                     |     |            |        | 0          |
|                     |     |            |        | 0          |
|                     |     |            |        | 0          |
| <b>Total Area</b>   |     |            |        | <b>3.1</b> |

| Impervious Landuses Present: |          |    |          |    |          |    |           |    |          |    |           |      |  |
|------------------------------|----------|----|----------|----|----------|----|-----------|----|----------|----|-----------|------|--|
| Soils                        | Roadway  |    | Sidewalk |    | Driveway |    | Building  |    | SWMF     |    | Subtotals |      |  |
|                              | Area     | CN | Area     | CN | Area     | CN | Area (ha) | CN | Area     | CN | Area      | A*CN |  |
| LTW                          | 0        | 98 | 0        | 98 | 0        | 98 | 0.000     | 98 | 0        | 98 | 0.00      | 0.00 |  |
|                              | 0        | 98 | 0        | 98 | 0        | 98 | 0         | 98 |          | 98 | 0         | 0    |  |
|                              | 0        | 98 |          | 98 |          | 98 |           | 98 |          | 98 | 0         | 0    |  |
|                              | 0        | 98 |          | 98 |          | 98 |           | 98 |          | 98 | 0         | 0    |  |
| <b>Subtotal Area</b>         | <b>0</b> |    | <b>0</b> |    | <b>0</b> |    | <b>0</b>  |    | <b>0</b> |    |           |      |  |

| Pervious Landuses Present:  |             |    |             |    |             |    |             |    |            |                        |           |        |  |
|-----------------------------|-------------|----|-------------|----|-------------|----|-------------|----|------------|------------------------|-----------|--------|--|
| Soils                       | Woodland    |    | Meadow      |    | Wetland     |    | Lawn        |    | Cultivated |                        | Subtotals |        |  |
|                             | Area        | CN | Area        | CN | Area        | CN | Area (ha)   | CN | Area       | CN                     | Area      | A*CN   |  |
| LTW                         | 0.00        |    | 0.00        |    | 0.00        |    | 0.00        |    | 3.1        | 74                     | 3.05      | 225.70 |  |
|                             | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |                        | 0.00      | 0.00   |  |
|                             | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |                        | 0.00      | 0.00   |  |
|                             | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |                        | 0.00      | 0.00   |  |
| <b>Subtotal Area</b>        | <b>0.00</b> |    | <b>0.00</b> |    | <b>0.00</b> |    | <b>0.00</b> |    | <b>3.1</b> |                        |           |        |  |
| Composite Area Calculations |             |    |             |    |             |    |             |    |            | Total Pervious Area    |           | 3.1    |  |
|                             |             |    |             |    |             |    |             |    |            | Total Impervious Area  |           | 0.0    |  |
|                             |             |    |             |    |             |    |             |    |            | % Impervious           |           | 0.0%   |  |
|                             |             |    |             |    |             |    |             |    |            | Composite Curve Number |           | 74.0   |  |
| Total Area Check            |             |    |             |    |             |    |             |    |            |                        |           | 3.1    |  |

**Initial Abstraction and Tp Calculations**

| Initial Abstraction |         |             |          | Composite Curve Number              |      |    |      |    |      |    |      |              |
|---------------------|---------|-------------|----------|-------------------------------------|------|----|------|----|------|----|------|--------------|
| Landuse             | IA (mm) | Area (ha)   | A * IA   | Listowel Silt                       |      | 0  |      | 0  |      | 0  |      | A*RC         |
|                     |         |             |          | RC                                  | Area | RC | Area | RC | Area | RC | Area |              |
| Woodland            | 10      | 0.00        | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0            |
| Meadow              | 8       | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0            |
| Wetland             | 16      | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0            |
| Lawn                | 5       | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0.000        |
| Cultivated          | 7       | 3           | 21.35    | 0.35                                | 3    |    | 0    |    | 0    |    | 0    | 1.068        |
| Impervious          | 2       | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0.000        |
| <b>Composite IA</b> |         | <b>3.05</b> | <b>7</b> | <b>Composite Runoff Coefficient</b> |      |    |      |    |      |    |      | <b>0.350</b> |

| Time to Peak Inputs   |            |          |           |                    |                | Uplands |        |               | Bransby Williams |        | Airport |        |
|-----------------------|------------|----------|-----------|--------------------|----------------|---------|--------|---------------|------------------|--------|---------|--------|
| Flow Path Description | Length (m) | Drop (m) | Slope (%) | V/S <sup>0.5</sup> | Velocity (m/s) | Tc (hr) | Tp(hr) | TOTAL Tp (hr) | Tc (hr)          | Tp(hr) | Tc (hr) | Tp(hr) |
| Overland              | 158.58     | 6.5      | 4.10%     | 2.7                | 0.55           | 0.08    | 0.05   | 0.05          | 0.10             | 0.07   | 0.32    | 0.22   |

Appropriate calculated time to 0.22 Appropriate Method: Airport



Project Name: Glenelg Expansion Lands  
 Project Number: 1060-6220  
 Date: 2022-08-11  
 By: KS

**D.A. NAME** PRE-4  
**D.A. AREA (ha)** 2.29

**Hydrologic Parameters: CALIB NASHYD Command  
 Pre Development Drainage Area: Catchment PRE-4  
 Pre-Dev to South Residential**

**Curve Number Calculation**

| Soil Types Present: |     |            |        |            |
|---------------------|-----|------------|--------|------------|
| Type                | ID  | Hydrologic | % Area | Area       |
| Listowel Silt Loam  | LTW | B          | 100.0% | 2.3        |
|                     |     |            |        | 0          |
|                     |     |            |        | 0          |
|                     |     |            |        | 0          |
| <b>Total Area</b>   |     |            |        | <b>2.3</b> |

| Impervious Landuses Present: |          |    |          |    |          |    |           |    |          |    |           |      |  |
|------------------------------|----------|----|----------|----|----------|----|-----------|----|----------|----|-----------|------|--|
| Soils                        | Roadway  |    | Sidewalk |    | Driveway |    | Building  |    | SWMF     |    | Subtotals |      |  |
|                              | Area     | CN | Area     | CN | Area     | CN | Area (ha) | CN | Area     | CN | Area      | A*CN |  |
| LTW                          | 0        | 98 | 0        | 98 | 0        | 98 | 0.000     | 98 | 0        | 98 | 0.00      | 0.00 |  |
|                              | 0        | 98 | 0        | 98 | 0        | 98 | 0         | 98 |          | 98 | 0         | 0    |  |
|                              | 0        | 98 |          | 98 |          | 98 |           | 98 |          | 98 | 0         | 0    |  |
|                              | 0        | 98 |          | 98 |          | 98 |           | 98 |          | 98 | 0         | 0    |  |
| <b>Subtotal Area</b>         | <b>0</b> |    | <b>0</b> |    | <b>0</b> |    | <b>0</b>  |    | <b>0</b> |    |           |      |  |

| Pervious Landuses Present:  |             |    |             |    |             |    |             |    |            |                        |           |        |  |
|-----------------------------|-------------|----|-------------|----|-------------|----|-------------|----|------------|------------------------|-----------|--------|--|
| Soils                       | Woodland    |    | Meadow      |    | Wetland     |    | Lawn        |    | Cultivated |                        | Subtotals |        |  |
|                             | Area        | CN | Area        | CN | Area        | CN | Area (ha)   | CN | Area       | CN                     | Area      | A*CN   |  |
| LTW                         | 0.00        |    | 0.00        |    | 0.00        |    | 0.00        |    | 2.3        | 74                     | 2.29      | 169.46 |  |
|                             | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |                        | 0.00      | 0.00   |  |
|                             | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |                        | 0.00      | 0.00   |  |
|                             | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |                        | 0.00      | 0.00   |  |
| <b>Subtotal Area</b>        | <b>0.00</b> |    | <b>0.00</b> |    | <b>0.00</b> |    | <b>0.00</b> |    | <b>2.3</b> |                        |           |        |  |
| Composite Area Calculations |             |    |             |    |             |    |             |    |            | Total Pervious Area    |           | 2.3    |  |
|                             |             |    |             |    |             |    |             |    |            | Total Impervious Area  |           | 0.0    |  |
|                             |             |    |             |    |             |    |             |    |            | % Impervious           |           | 0.0%   |  |
|                             |             |    |             |    |             |    |             |    |            | Composite Curve Number |           | 74.0   |  |
| Total Area Check            |             |    |             |    |             |    |             |    |            |                        |           | 2.3    |  |

**Initial Abstraction and Tp Calculations**

| Initial Abstraction |         |             |          | Composite Curve Number              |      |    |      |    |      |    |      |              |
|---------------------|---------|-------------|----------|-------------------------------------|------|----|------|----|------|----|------|--------------|
| Landuse             | IA (mm) | Area (ha)   | A * IA   | Listowel Silt                       |      | 0  |      | 0  |      | 0  |      | A*RC         |
|                     |         |             |          | RC                                  | Area | RC | Area | RC | Area | RC | Area |              |
| Woodland            | 10      | 0.00        | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0            |
| Meadow              | 8       | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0            |
| Wetland             | 16      | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0            |
| Lawn                | 5       | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0.000        |
| Cultivated          | 7       | 2           | 16.03    | 0.35                                | 2    |    | 0    |    | 0    |    | 0    | 0.802        |
| Impervious          | 2       | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0.000        |
| <b>Composite IA</b> |         | <b>2.29</b> | <b>7</b> | <b>Composite Runoff Coefficient</b> |      |    |      |    |      |    |      | <b>0.350</b> |

| Time to Peak Inputs   |            |          |           |                    |                | Uplands |        |               | Bransby Williams |        | Airport |        |
|-----------------------|------------|----------|-----------|--------------------|----------------|---------|--------|---------------|------------------|--------|---------|--------|
| Flow Path Description | Length (m) | Drop (m) | Slope (%) | V/S <sup>0.5</sup> | Velocity (m/s) | Tc (hr) | Tp(hr) | TOTAL Tp (hr) | Tc (hr)          | Tp(hr) | Tc (hr) | Tp(hr) |
| Overland              | 208.86     | 2        | 0.96%     | 2.7                | 0.26           | 0.22    | 0.15   | 0.15          | 0.18             | 0.12   | 0.60    | 0.40   |

Appropriate calculated time to 0.40 Appropriate Method: Airport



Project Name: Glenelg Expansion Lands  
 Project Number: 1060-6220  
 Date: 2022-08-11  
 By: KS

**D.A. NAME** PRE-5  
**D.A. AREA (ha)** 3.00

**Hydrologic Parameters: CALIB NASHYD Command  
 Pre Development Drainage Area: Catchment PRE-5  
 Pre-Dev to Southeast Tile**

**Curve Number Calculation**

| Soil Types Present: |     |            |        |            |
|---------------------|-----|------------|--------|------------|
| Type                | ID  | Hydrologic | % Area | Area       |
| Listowel Silt Loam  | LTW | B          | 100.0% | 3.0        |
|                     |     |            |        | 0          |
|                     |     |            |        | 0          |
|                     |     |            |        | 0          |
| <b>Total Area</b>   |     |            |        | <b>3.0</b> |

| Impervious Landuses Present: |          |    |          |    |          |    |           |    |          |    |           |      |  |
|------------------------------|----------|----|----------|----|----------|----|-----------|----|----------|----|-----------|------|--|
| Soils                        | Roadway  |    | Sidewalk |    | Driveway |    | Building  |    | SWMF     |    | Subtotals |      |  |
|                              | Area     | CN | Area     | CN | Area     | CN | Area (ha) | CN | Area     | CN | Area      | A*CN |  |
| LTW                          | 0        | 98 | 0        | 98 | 0        | 98 | 0.000     | 98 | 0        | 98 | 0.00      | 0.00 |  |
|                              | 0        | 98 | 0        | 98 | 0        | 98 | 0         | 98 |          | 98 | 0         | 0    |  |
|                              | 0        | 98 |          | 98 |          | 98 |           | 98 |          | 98 | 0         | 0    |  |
|                              | 0        | 98 |          | 98 |          | 98 |           | 98 |          | 98 | 0         | 0    |  |
| <b>Subtotal Area</b>         | <b>0</b> |    | <b>0</b> |    | <b>0</b> |    | <b>0</b>  |    | <b>0</b> |    |           |      |  |

| Pervious Landuses Present:  |             |    |             |    |             |    |             |    |            |                        |           |        |  |
|-----------------------------|-------------|----|-------------|----|-------------|----|-------------|----|------------|------------------------|-----------|--------|--|
| Soils                       | Woodland    |    | Meadow      |    | Wetland     |    | Lawn        |    | Cultivated |                        | Subtotals |        |  |
|                             | Area        | CN | Area        | CN | Area        | CN | Area (ha)   | CN | Area       | CN                     | Area      | A*CN   |  |
| LTW                         | 0.00        |    | 0.00        |    | 0.00        |    | 0.00        |    | 3.0        | 74                     | 3.00      | 222.00 |  |
|                             | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |                        | 0.00      | 0.00   |  |
|                             | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |                        | 0.00      | 0.00   |  |
|                             | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |                        | 0.00      | 0.00   |  |
| <b>Subtotal Area</b>        | <b>0.00</b> |    | <b>0.00</b> |    | <b>0.00</b> |    | <b>0.00</b> |    | <b>3.0</b> |                        |           |        |  |
| Composite Area Calculations |             |    |             |    |             |    |             |    |            | Total Pervious Area    |           | 3.0    |  |
|                             |             |    |             |    |             |    |             |    |            | Total Impervious Area  |           | 0.0    |  |
|                             |             |    |             |    |             |    |             |    |            | % Impervious           |           | 0.0%   |  |
|                             |             |    |             |    |             |    |             |    |            | Composite Curve Number |           | 74.0   |  |
| Total Area Check            |             |    |             |    |             |    |             |    |            |                        |           | 3.0    |  |

**Initial Abstraction and Tp Calculations**

| Initial Abstraction |         |             |          | Composite Curve Number              |      |    |      |    |      |    |      |              |
|---------------------|---------|-------------|----------|-------------------------------------|------|----|------|----|------|----|------|--------------|
| Landuse             | IA (mm) | Area (ha)   | A * IA   | Listowel Silt                       |      | 0  |      | 0  |      | 0  |      | A*RC         |
|                     |         |             |          | RC                                  | Area | RC | Area | RC | Area | RC | Area |              |
| Woodland            | 10      | 0.00        | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0            |
| Meadow              | 8       | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0            |
| Wetland             | 16      | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0            |
| Lawn                | 5       | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0.000        |
| Cultivated          | 7       | 3           | 21       | 0.35                                | 3    |    | 0    |    | 0    |    | 0    | 1.050        |
| Impervious          | 2       | 0           | 0        |                                     | 0    |    | 0    |    | 0    |    | 0    | 0.000        |
| <b>Composite IA</b> |         | <b>3.00</b> | <b>7</b> | <b>Composite Runoff Coefficient</b> |      |    |      |    |      |    |      | <b>0.350</b> |

| Time to Peak Inputs   |            |          |           |                    |                | Uplands |        |               | Bransby Williams |        | Airport |        |
|-----------------------|------------|----------|-----------|--------------------|----------------|---------|--------|---------------|------------------|--------|---------|--------|
| Flow Path Description | Length (m) | Drop (m) | Slope (%) | V/S <sup>0.5</sup> | Velocity (m/s) | Tc (hr) | Tp(hr) | TOTAL Tp (hr) | Tc (hr)          | Tp(hr) | Tc (hr) | Tp(hr) |
| Overland              | 105.7      | 4.5      | 4.26%     | 2.7                | 0.56           | 0.05    | 0.04   | 0.04          | 0.07             | 0.05   | 0.26    | 0.17   |

Appropriate calculated time to 0.17 Appropriate Method: Airport



PROJECT: Glenelg Expansion Lands  
PROJECT No.: 1060-6220  
FILE: Post-Development Breakdown  
DATE: 2022.08.22  
DESIGN: K. Swain  
CHECK: A. West

**Drainage Areas**  
Post-Development Breakdown

| Description               | Area (sqm) | Area (ha) | Runoff Coeff. | TIMP | A*TIMP | XIMP | A*XIMP |
|---------------------------|------------|-----------|---------------|------|--------|------|--------|
| Single Family Residential | 96660      | 9.67      | 0.59          | 55%  | 5.32   | 45%  | 4.35   |
| Semi-Detached Residential | 4751       | 0.48      | 0.66          | 65%  | 0.31   | 45%  | 0.21   |
| Townhouses                | 16596      | 1.66      | 0.73          | 75%  | 1.24   | 55%  | 0.91   |
| SWM Facility              | 14329      | 1.43      | 0.55          | 50%  | 0.72   | 50%  | 0.72   |
| Park                      | 13915      | 1.39      | 0.30          | 14%  | 0.20   | 14%  | 0.19   |
| Roads                     | 76081      | 7.61      | 0.75          | 79%  | 5.98   | 79%  | 5.98   |
| Walkways                  | 3441       | 0.34      | 0.90          | 100% | 0.34   | 100% | 0.34   |
| Total                     | 259900     | 25.99     | 0.63          | 62%  | 16.05  | 52%  | 13.51  |



Project Name: Glenelg Expansion Lands  
 Project Number: 1060-6220  
 Date: 2022-08-11  
 By: KS

**D.A. NAME** NYHD185  
**D.A. AREA (ha)** 1.66

**Hydrologic Parameters: CALIB NASHYD Command**  
**Post-Development Drainage Area: Catchment NYHD185**  
**Draining to Bioretention and PP**

**Curve Number Calculation**

| Soil Types Present: |     |            |        |            |
|---------------------|-----|------------|--------|------------|
| Type                | ID  | Hydrologic | % Area | Area       |
| Listowel Silt Loam  | LTW | B          | 100.0% | 1.7        |
|                     |     |            |        | 0          |
|                     |     |            |        | 0          |
|                     |     |            |        | 0          |
| <b>Total Area</b>   |     |            |        | <b>1.7</b> |

| Impervious Landuses Present: |          |    |                |    |                |    |                |    |          |    |           |          |  |
|------------------------------|----------|----|----------------|----|----------------|----|----------------|----|----------|----|-----------|----------|--|
| Soils                        | Roadway  |    | Sidewalk       |    | Driveway       |    | Building       |    | SWMF     |    | Subtotals |          |  |
|                              | Area     | CN | Area           | CN | Area           | CN | Area (ha)      | CN | Area     | CN | Area      | A*CN     |  |
| LTW                          | 0        | 98 | 0.01447        | 98 | 0.05788        | 98 | 0.217          | 98 | 0        | 98 | 0.29      | 28.36    |  |
|                              | 0        | 98 | 0              | 98 | 0              | 98 | 0              | 98 |          | 98 | 0         | 0        |  |
|                              | 0        | 98 |                | 98 |                | 98 |                | 98 |          | 98 | 0         | 0        |  |
|                              | 0        | 98 |                | 98 |                | 98 |                | 98 |          | 98 | 0         | 0        |  |
| <b>Subtotal Area</b>         | <b>0</b> |    | <b>0.01447</b> |    | <b>0.05788</b> |    | <b>0.21705</b> |    | <b>0</b> |    |           | <b>0</b> |  |

| Pervious Landuses Present: |             |    |             |    |             |    |             |    |            |    |           |             |  |
|----------------------------|-------------|----|-------------|----|-------------|----|-------------|----|------------|----|-----------|-------------|--|
| Soils                      | Woodland    |    | Meadow      |    | Wetland     |    | Lawn        |    | Cultivated |    | Subtotals |             |  |
|                            | Area        | CN | Area        | CN | Area        | CN | Area (ha)   | CN | Area       | CN | Area      | A*CN        |  |
| LTW                        | 0.00        |    | 0.00        |    | 0.00        |    | 1.27        | 69 | 0.0        | 74 | 1.27      | 87.42       |  |
|                            | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |    | 0.00      | 0.00        |  |
|                            | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |    | 0.00      | 0.00        |  |
|                            | 0           |    | 0.00        |    | 0.00        |    | 0.00        |    | 0.00       |    | 0.00      | 0.00        |  |
| <b>Subtotal Area</b>       | <b>0.00</b> |    | <b>0.00</b> |    | <b>0.00</b> |    | <b>1.27</b> |    | <b>0.0</b> |    |           | <b>0.00</b> |  |

|                             |  |                        |       |
|-----------------------------|--|------------------------|-------|
| Composite Area Calculations |  | Total Pervious Area    | 1.3   |
|                             |  | Total Impervious Area  | 0.4   |
|                             |  | % Impervious           | 23.9% |
|                             |  | Composite Curve Number | 69.6  |
| Total Area Check            |  |                        | 1.7   |

**Initial Abstraction and Tp Calculations**

| Initial Abstraction |             |                |                | Composite Curve Number              |      |    |      |    |      |   |   |              |
|---------------------|-------------|----------------|----------------|-------------------------------------|------|----|------|----|------|---|---|--------------|
| Landuse             | IA (mm)     | Area (ha)      | A * IA         | Listowel Silt                       |      | 0  |      | 0  |      | 0 |   | A*RC         |
|                     |             |                |                | RC                                  | Area | RC | Area | RC | Area |   |   |              |
| Woodland            | 10          | 0.00           | 0              |                                     | 0    |    | 0    |    | 0    |   | 0 | 0            |
| Meadow              | 8           | 0              | 0              |                                     | 0    |    | 0    |    | 0    |   | 0 | 0            |
| Wetland             | 16          | 0              | 0              |                                     | 0    |    | 0    |    | 0    |   | 0 | 0            |
| Lawn                | 5           | 1.2669         | 6.3345         | 0.25                                | 1.3  |    | 0    |    | 0    |   | 0 | 0.317        |
| Cultivated          | 7           | 0              | 0              |                                     | 0    |    | 0    |    | 0    |   | 0 | 0.000        |
| Impervious          | 2           | 0.2894         | 0.5788         | 0.90                                | 0.29 |    | 0    |    | 0    |   | 0 | 0.260        |
| <b>Composite IA</b> | <b>1.56</b> | <b>4.44214</b> | <b>4.44214</b> | <b>Composite Runoff Coefficient</b> |      |    |      |    |      |   |   | <b>0.347</b> |

| Time to Peak Inputs   |            |          |           |                    |                | Uplands |         |               | Bransby Williams |         | Airport |         |
|-----------------------|------------|----------|-----------|--------------------|----------------|---------|---------|---------------|------------------|---------|---------|---------|
| Flow Path Description | Length (m) | Drop (m) | Slope (%) | V/S <sup>0.5</sup> | Velocity (m/s) | Tc (hr) | Tp (hr) | TOTAL Tp (hr) | Tc (hr)          | Tp (hr) | Tc (hr) | Tp (hr) |
| Overland              | 121        | 2.42     | 2.00%     | 2.7                | 0.38           | 0.09    | 0.06    | 0.06          | 0.10             | 0.06    | 0.36    | 0.24    |

Appropriate calculated time to **0.24** Appropriate Method: **Airport**

# APPENDIX D

## VO6 Model Input & Output Files

## Visual OTTHYMO 6.0 Model Schematic

### Pre-Development

|   |  |   |   |   |   |   |   |   |  |
|---|--|---|---|---|---|---|---|---|--|
| 1 |  <p><b>PRE-1 (CP TRAIL)</b><br/>AREA [ha] - 4.320<br/>PKFW [m<sup>3</sup>/s] - 0.244<br/>TP [hr] - 1.333<br/>RV [mm] - 26.684</p> | 2 |  <p><b>PRE-2 (NORTH TILE DRAIN)</b><br/>AREA [ha] - 13.330<br/>PKFW [m<sup>3</sup>/s] - 0.528<br/>TP [hr] - 1.750<br/>RV [mm] - 26.689</p> | 3 |  <p><b>PRE-3 (EAST TILE DRAIN)</b><br/>AREA [ha] - 3.050<br/>PKFW [m<sup>3</sup>/s] - 0.241<br/>TP [hr] - 1.167<br/>RV [mm] - 26.654</p> | 5 |  <p><b>PRE-4 (SOUTH RESIDENTIAL)</b><br/>AREA [ha] - 2.290<br/>PKFW [m<sup>3</sup>/s] - 0.121<br/>TP [hr] - 1.417<br/>RV [mm] - 26.686</p> | 4 |  <p><b>PRE-5 (SOUTHEAST TILE DRAIN)</b><br/>AREA [ha] - 3.000<br/>PKFW [m<sup>3</sup>/s] - 0.278<br/>TP [hr] - 1.083<br/>RV [mm] - 26.594</p> |
|---|--|---|---|---|---|---|---|---|--|



=====

V V I SSSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voindat

Output filename:
C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\6f6bb
d7d-7403-423d-925d-c82993f4def1\scena

Summary filename:
C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\6f6bb
d7d-7403-423d-925d-c82993f4def1\scena

DATE: 08-23-2022 TIME: 11:44:29

USER:

COMMENTS: \_\_\_\_\_

\*\* SIMULATION : 25mm \*\*

READ STORM | Filename: C:\Users\kswain\AppData\Local\Temp\

Ptotal= 24.99 mm | Comments: ac87f79d-69e0-48a7-b484-801844eb7b93\ac52d15
25mm

Table with 4 columns: TIME, RAIN, TIME, RAIN. Rows show rainfall intensity and volume over time.

CALIB NASHYD ( 0001) Area (ha)= 4.32 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.36

Unit Hyd Qpeak (cms)= 0.458
PEAK FLOW (cms)= 0.024 (i)
TIME TO PEAK (hrs)= 1.917
RUNOFF VOLUME (mm)= 3.018
TOTAL RAINFALL (mm)= 24.991
RUNOFF COEFFICIENT = 0.121

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0002) Area (ha)= 13.33 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.62

Unit Hyd Qpeak (cms)= 0.821
PEAK FLOW (cms)= 0.054 (i)
TIME TO PEAK (hrs)= 2.250
RUNOFF VOLUME (mm)= 3.018
TOTAL RAINFALL (mm)= 24.991
RUNOFF COEFFICIENT = 0.121

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0003) Area (ha)= 3.05 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.22

Unit Hyd Qpeak (cms)= 0.530
PEAK FLOW (cms)= 0.022 (i)
TIME TO PEAK (hrs)= 1.667
RUNOFF VOLUME (mm)= 3.014
TOTAL RAINFALL (mm)= 24.991
RUNOFF COEFFICIENT = 0.121

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0004) Area (ha)= 3.00 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.17

Unit Hyd Qpeak (cms)= 0.674
PEAK FLOW (cms)= 0.024 (i)
TIME TO PEAK (hrs)= 1.583
RUNOFF VOLUME (mm)= 3.007
TOTAL RAINFALL (mm)= 24.991
RUNOFF COEFFICIENT = 0.120

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0005) Area (ha)= 2.29 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.40

Unit Hyd Qpeak (cms)= 0.219
PEAK FLOW (cms)= 0.012 (i)
TIME TO PEAK (hrs)= 1.917
RUNOFF VOLUME (mm)= 3.018
TOTAL RAINFALL (mm)= 24.991

RUNOFF COEFFICIENT = 0.121

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

FINISH
=====

```

=====
V V I SSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000
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```

\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voindat

Output filename:  
C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\1ed6f7fc-248e-41ed-aed5-12089c5bbc2f\scena

Summary filename:  
C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\1ed6f7fc-248e-41ed-aed5-12089c5bbc2f\scena

DATE: 08-23-2022 TIME: 11:41:13

USER:

COMMENTS: \_\_\_\_\_

```

*****
** SIMULATION : A. 2yr 3hr 10min Chicago **
*****

```

|                  |                                  |
|------------------|----------------------------------|
| CHICAGO STORM    | IDF curve parameters: A= 404.147 |
| Ptotal= 32.13 mm | B= 0.000                         |
|                  | C= 0.699                         |

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

| TIME | RAIN  | TIME | RAIN  | TIME | RAIN  | TIME | RAIN  |
|------|-------|------|-------|------|-------|------|-------|
| hrs  | mm/hr | hrs  | mm/hr | hrs  | mm/hr | hrs  | mm/hr |
| 0.00 | 3.76  | 0.83 | 80.82 | 1.67 | 5.83  | 2.50 | 3.63  |
| 0.17 | 4.42  | 1.00 | 17.11 | 1.83 | 5.15  | 2.67 | 3.39  |
| 0.33 | 5.48  | 1.17 | 10.79 | 2.00 | 4.63  | 2.83 | 3.20  |
| 0.50 | 7.50  | 1.33 | 8.23  | 2.17 | 4.23  |      |       |
| 0.67 | 13.95 | 1.50 | 6.78  | 2.33 | 3.90  |      |       |

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 4.32    | Curve Number (CN)= 74.0   |
| NASHYD ( 0001)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.36 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN  | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|-------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 3.76  | 0.833 | 13.95 | 1.583 | 6.78  | 2.33 | 4.23  |
| 0.167 | 3.76  | 0.917 | 80.82 | 1.667 | 6.78  | 2.42 | 3.90  |
| 0.250 | 4.42  | 1.000 | 80.82 | 1.750 | 5.83  | 2.50 | 3.90  |
| 0.333 | 4.42  | 1.083 | 17.11 | 1.833 | 5.83  | 2.58 | 3.63  |
| 0.417 | 5.48  | 1.167 | 17.11 | 1.917 | 5.15  | 2.67 | 3.63  |
| 0.500 | 5.48  | 1.250 | 10.79 | 2.000 | 5.15  | 2.75 | 3.39  |
| 0.583 | 7.50  | 1.333 | 10.79 | 2.083 | 4.63  | 2.83 | 3.39  |
| 0.667 | 7.50  | 1.417 | 8.23  | 2.167 | 4.63  | 2.92 | 3.20  |
| 0.750 | 13.95 | 1.500 | 8.23  | 2.250 | 4.23  | 3.00 | 3.20  |

Unit Hyd Qpeak (cms)= 0.458

PEAK FLOW (cms)= 0.044 (i)  
TIME TO PEAK (hrs)= 1.417  
RUNOFF VOLUME (mm)= 5.521  
TOTAL RAINFALL (mm)= 32.132  
RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 13.33   | Curve Number (CN)= 74.0   |
| NASHYD ( 0002)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.62 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN  | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|-------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 3.76  | 0.833 | 13.95 | 1.583 | 6.78  | 2.33 | 4.23  |
| 0.167 | 3.76  | 0.917 | 80.82 | 1.667 | 6.78  | 2.42 | 3.90  |
| 0.250 | 4.42  | 1.000 | 80.82 | 1.750 | 5.83  | 2.50 | 3.90  |
| 0.333 | 4.42  | 1.083 | 17.11 | 1.833 | 5.83  | 2.58 | 3.63  |
| 0.417 | 5.48  | 1.167 | 17.11 | 1.917 | 5.15  | 2.67 | 3.63  |
| 0.500 | 5.48  | 1.250 | 10.79 | 2.000 | 5.15  | 2.75 | 3.39  |
| 0.583 | 7.50  | 1.333 | 10.79 | 2.083 | 4.63  | 2.83 | 3.39  |
| 0.667 | 7.50  | 1.417 | 8.23  | 2.167 | 4.63  | 2.92 | 3.20  |
| 0.750 | 13.95 | 1.500 | 8.23  | 2.250 | 4.23  | 3.00 | 3.20  |

Unit Hyd Qpeak (cms)= 0.821

PEAK FLOW (cms)= 0.100 (i)  
TIME TO PEAK (hrs)= 1.917  
RUNOFF VOLUME (mm)= 5.522  
TOTAL RAINFALL (mm)= 32.132  
RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 3.05    | Curve Number (CN)= 74.0   |
| NASHYD ( 0003)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.22 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN  | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|-------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 3.76  | 0.833 | 13.95 | 1.583 | 6.78  | 2.33 | 4.23  |
| 0.167 | 3.76  | 0.917 | 80.82 | 1.667 | 6.78  | 2.42 | 3.90  |
| 0.250 | 4.42  | 1.000 | 80.82 | 1.750 | 5.83  | 2.50 | 3.90  |
| 0.333 | 4.42  | 1.083 | 17.11 | 1.833 | 5.83  | 2.58 | 3.63  |
| 0.417 | 5.48  | 1.167 | 17.11 | 1.917 | 5.15  | 2.67 | 3.63  |
| 0.500 | 5.48  | 1.250 | 10.79 | 2.000 | 5.15  | 2.75 | 3.39  |

|       |       |       |       |       |      |      |      |
|-------|-------|-------|-------|-------|------|------|------|
| 0.583 | 7.50  | 1.333 | 10.79 | 2.083 | 4.63 | 2.83 | 3.39 |
| 0.667 | 7.50  | 1.417 | 8.23  | 2.167 | 4.63 | 2.92 | 3.20 |
| 0.750 | 13.95 | 1.500 | 8.23  | 2.250 | 4.23 | 3.00 | 3.20 |

Unit Hyd Qpeak (cms)= 0.530

PEAK FLOW (cms)= 0.041 (i)  
TIME TO PEAK (hrs)= 1.250  
RUNOFF VOLUME (mm)= 5.515  
TOTAL RAINFALL (mm)= 32.132  
RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 3.00    | Curve Number (CN)= 74.0   |
| NASHYD ( 0004)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.17 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN  | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|-------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 3.76  | 0.833 | 13.95 | 1.583 | 6.78  | 2.33 | 4.23  |
| 0.167 | 3.76  | 0.917 | 80.82 | 1.667 | 6.78  | 2.42 | 3.90  |
| 0.250 | 4.42  | 1.000 | 80.82 | 1.750 | 5.83  | 2.50 | 3.90  |
| 0.333 | 4.42  | 1.083 | 17.11 | 1.833 | 5.83  | 2.58 | 3.63  |
| 0.417 | 5.48  | 1.167 | 17.11 | 1.917 | 5.15  | 2.67 | 3.63  |
| 0.500 | 5.48  | 1.250 | 10.79 | 2.000 | 5.15  | 2.75 | 3.39  |
| 0.583 | 7.50  | 1.333 | 10.79 | 2.083 | 4.63  | 2.83 | 3.39  |
| 0.667 | 7.50  | 1.417 | 8.23  | 2.167 | 4.63  | 2.92 | 3.20  |
| 0.750 | 13.95 | 1.500 | 8.23  | 2.250 | 4.23  | 3.00 | 3.20  |

Unit Hyd Qpeak (cms)= 0.674

PEAK FLOW (cms)= 0.047 (i)  
TIME TO PEAK (hrs)= 1.167  
RUNOFF VOLUME (mm)= 5.502  
TOTAL RAINFALL (mm)= 32.132  
RUNOFF COEFFICIENT = 0.171

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|       |  |
|-------|--|
| CALIB |  |
|-------|--|

NASHYD ( 0005) | Area (ha)= 2.29 | Curve Number (CN)= 74.0  
 ID= 1 DT= 5.0 min | Ia (mm)= 7.00 | # of Linear Res.(N)= 3.00  
 -----  
 U.H. Tp(hrs)= 0.40

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

| TIME  | RAIN  | TIME  | RAIN  | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|-------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 3.76  | 0.833 | 13.95 | 1.583 | 6.78  | 2.33 | 4.23  |
| 0.167 | 3.76  | 0.917 | 80.82 | 1.667 | 6.78  | 2.42 | 3.90  |
| 0.250 | 4.42  | 1.000 | 80.82 | 1.750 | 5.83  | 2.50 | 3.90  |
| 0.333 | 4.42  | 1.083 | 17.11 | 1.833 | 5.83  | 2.58 | 3.63  |
| 0.417 | 5.48  | 1.167 | 17.11 | 1.917 | 5.15  | 2.67 | 3.63  |
| 0.500 | 5.48  | 1.250 | 10.79 | 2.000 | 5.15  | 2.75 | 3.39  |
| 0.583 | 7.50  | 1.333 | 10.79 | 2.083 | 4.63  | 2.83 | 3.39  |
| 0.667 | 7.50  | 1.417 | 8.23  | 2.167 | 4.63  | 2.92 | 3.20  |
| 0.750 | 13.95 | 1.500 | 8.23  | 2.250 | 4.23  | 3.00 | 3.20  |

Unit Hyd Qpeak (cms)= 0.219

PEAK FLOW (cms)= 0.022 (i)  
 TIME TO PEAK (hrs)= 1.500  
 RUNOFF VOLUME (mm)= 5.521  
 TOTAL RAINFALL (mm)= 32.132  
 RUNOFF COEFFICIENT = 0.172

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----

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=====
V V I SSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

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```

```

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voindat
Output filename:
C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\6363c
f35-5dd6-44fd-aa1-674dc05eef3c\scena
Summary filename:
C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\6363c
f35-5dd6-44fd-aa1-674dc05eef3c\scena

DATE: 08-23-2022 TIME: 11:41:14

USER:

COMMENTS:

```

```

***** SIMULATION : B. Syr 3hr 10min Chicago *****
CHICAGO STORM IDF curve parameters: A= 535.364
Ptotal= 42.56 mm B= 0.000
C= 0.699

```

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

| TIME | RAIN  | TIME | RAIN   | TIME | RAIN  | TIME | RAIN  |
|------|-------|------|--------|------|-------|------|-------|
| hrs  | mm/hr | hrs  | mm/hr  | hrs  | mm/hr | hrs  | mm/hr |
| 0.00 | 4.98  | 0.83 | 107.07 | 1.67 | 7.72  | 2.50 | 4.80  |
| 0.17 | 5.86  | 1.00 | 22.67  | 1.83 | 6.82  | 2.67 | 4.50  |
| 0.33 | 7.26  | 1.17 | 14.30  | 2.00 | 6.14  | 2.83 | 4.24  |
| 0.50 | 9.93  | 1.33 | 10.90  | 2.17 | 5.60  |      |       |
| 0.67 | 18.47 | 1.50 | 8.98   | 2.33 | 5.16  |      |       |

```

CALIB
NASHYD ( 0001) Area (ha)= 4.32 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.36

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

```

---- TRANSFORMED HYETOGRAPH ----

```

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 4.98  | 0.833 | 18.47  | 1.583 | 8.98  | 2.33 | 5.60  |
| 0.167 | 4.98  | 0.917 | 107.07 | 1.667 | 8.98  | 2.42 | 5.16  |
| 0.250 | 5.86  | 1.000 | 107.07 | 1.750 | 7.72  | 2.50 | 5.16  |
| 0.333 | 5.86  | 1.083 | 22.67  | 1.833 | 7.72  | 2.58 | 4.80  |
| 0.417 | 7.26  | 1.167 | 22.67  | 1.917 | 6.82  | 2.67 | 4.80  |
| 0.500 | 7.26  | 1.250 | 14.30  | 2.000 | 6.82  | 2.75 | 4.50  |
| 0.583 | 9.93  | 1.333 | 14.30  | 2.083 | 6.14  | 2.83 | 4.50  |
| 0.667 | 9.93  | 1.417 | 10.90  | 2.167 | 6.14  | 2.92 | 4.24  |
| 0.750 | 18.47 | 1.500 | 10.90  | 2.250 | 5.60  | 3.00 | 4.24  |

```

Unit Hyd Qpeak (cms)= 0.458
PEAK FLOW (cms)= 0.086 (i)
TIME TO PEAK (hrs)= 1.417
RUNOFF VOLUME (mm)= 10.132
TOTAL RAINFALL (mm)= 42.565
RUNOFF COEFFICIENT = 0.238

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD ( 0002) Area (ha)= 13.33 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.62

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

```

---- TRANSFORMED HYETOGRAPH ----

```

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 4.98  | 0.833 | 18.47  | 1.583 | 8.98  | 2.33 | 5.60  |
| 0.167 | 4.98  | 0.917 | 107.07 | 1.667 | 8.98  | 2.42 | 5.16  |
| 0.250 | 5.86  | 1.000 | 107.07 | 1.750 | 7.72  | 2.50 | 5.16  |
| 0.333 | 5.86  | 1.083 | 22.67  | 1.833 | 7.72  | 2.58 | 4.80  |
| 0.417 | 7.26  | 1.167 | 22.67  | 1.917 | 6.82  | 2.67 | 4.80  |
| 0.500 | 7.26  | 1.250 | 14.30  | 2.000 | 6.82  | 2.75 | 4.50  |
| 0.583 | 9.93  | 1.333 | 14.30  | 2.083 | 6.14  | 2.83 | 4.50  |
| 0.667 | 9.93  | 1.417 | 10.90  | 2.167 | 6.14  | 2.92 | 4.24  |
| 0.750 | 18.47 | 1.500 | 10.90  | 2.250 | 5.60  | 3.00 | 4.24  |

```

Unit Hyd Qpeak (cms)= 0.821
PEAK FLOW (cms)= 0.190 (i)
TIME TO PEAK (hrs)= 1.833
RUNOFF VOLUME (mm)= 10.134
TOTAL RAINFALL (mm)= 42.565
RUNOFF COEFFICIENT = 0.238

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD ( 0003) Area (ha)= 3.05 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.22

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

```

---- TRANSFORMED HYETOGRAPH ----

```

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 4.98  | 0.833 | 18.47  | 1.583 | 8.98  | 2.33 | 5.60  |
| 0.167 | 4.98  | 0.917 | 107.07 | 1.667 | 8.98  | 2.42 | 5.16  |
| 0.250 | 5.86  | 1.000 | 107.07 | 1.750 | 7.72  | 2.50 | 5.16  |
| 0.333 | 5.86  | 1.083 | 22.67  | 1.833 | 7.72  | 2.58 | 4.80  |
| 0.417 | 7.26  | 1.167 | 22.67  | 1.917 | 6.82  | 2.67 | 4.80  |
| 0.500 | 7.26  | 1.250 | 14.30  | 2.000 | 6.82  | 2.75 | 4.50  |

|       |       |       |       |       |      |      |      |
|-------|-------|-------|-------|-------|------|------|------|
| 0.583 | 9.93  | 1.333 | 14.30 | 2.083 | 6.14 | 2.83 | 4.50 |
| 0.667 | 9.93  | 1.417 | 10.90 | 2.167 | 6.14 | 2.92 | 4.24 |
| 0.750 | 18.47 | 1.500 | 10.90 | 2.250 | 5.60 | 3.00 | 4.24 |

```

Unit Hyd Qpeak (cms)= 0.530
PEAK FLOW (cms)= 0.081 (i)
TIME TO PEAK (hrs)= 1.167
RUNOFF VOLUME (mm)= 10.121
TOTAL RAINFALL (mm)= 42.565
RUNOFF COEFFICIENT = 0.238

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD ( 0004) Area (ha)= 3.00 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.17

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

```

---- TRANSFORMED HYETOGRAPH ----

```

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 4.98  | 0.833 | 18.47  | 1.583 | 8.98  | 2.33 | 5.60  |
| 0.167 | 4.98  | 0.917 | 107.07 | 1.667 | 8.98  | 2.42 | 5.16  |
| 0.250 | 5.86  | 1.000 | 107.07 | 1.750 | 7.72  | 2.50 | 5.16  |
| 0.333 | 5.86  | 1.083 | 22.67  | 1.833 | 7.72  | 2.58 | 4.80  |
| 0.417 | 7.26  | 1.167 | 22.67  | 1.917 | 6.82  | 2.67 | 4.80  |
| 0.500 | 7.26  | 1.250 | 14.30  | 2.000 | 6.82  | 2.75 | 4.50  |
| 0.583 | 9.93  | 1.333 | 14.30  | 2.083 | 6.14  | 2.83 | 4.50  |
| 0.667 | 9.93  | 1.417 | 10.90  | 2.167 | 6.14  | 2.92 | 4.24  |
| 0.750 | 18.47 | 1.500 | 10.90  | 2.250 | 5.60  | 3.00 | 4.24  |

```

Unit Hyd Qpeak (cms)= 0.674
PEAK FLOW (cms)= 0.093 (i)
TIME TO PEAK (hrs)= 1.167
RUNOFF VOLUME (mm)= 10.098
TOTAL RAINFALL (mm)= 42.565
RUNOFF COEFFICIENT = 0.237

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB

```

NASHYD ( 0005) | Area (ha)= 2.29 | Curve Number (CN)= 74.0  
 ID= 1 DT= 5.0 min | Ia (mm)= 7.00 | # of Linear Res.(N)= 3.00  
 -----  
 U.H. Tp(hrs)= 0.40

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 4.98  | 0.833 | 18.47  | 1.583 | 8.98  | 2.33 | 5.60  |
| 0.167 | 4.98  | 0.917 | 107.07 | 1.667 | 8.98  | 2.42 | 5.16  |
| 0.250 | 5.86  | 1.000 | 107.07 | 1.750 | 7.72  | 2.50 | 5.16  |
| 0.333 | 5.86  | 1.083 | 22.67  | 1.833 | 7.72  | 2.58 | 4.80  |
| 0.417 | 7.26  | 1.167 | 22.67  | 1.917 | 6.82  | 2.67 | 4.80  |
| 0.500 | 7.26  | 1.250 | 14.30  | 2.000 | 6.82  | 2.75 | 4.50  |
| 0.583 | 9.93  | 1.333 | 14.30  | 2.083 | 6.14  | 2.83 | 4.50  |
| 0.667 | 9.93  | 1.417 | 10.90  | 2.167 | 6.14  | 2.92 | 4.24  |
| 0.750 | 18.47 | 1.500 | 10.90  | 2.250 | 5.60  | 3.00 | 4.24  |

Unit Hyd Qpeak (cms)= 0.219

PEAK FLOW (cms)= 0.042 (i)  
 TIME TO PEAK (hrs)= 1.500  
 RUNOFF VOLUME (mm)= 10.133  
 TOTAL RAINFALL (mm)= 42.565  
 RUNOFF COEFFICIENT = 0.238

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----

```

=====
V V I SSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000
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```

\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

```

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat
Output filename:
C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\26c8c
d4b-b0a5-4ee8-938e-92ef2ca0db10\scena
Summary filename:
C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\26c8c
d4b-b0a5-4ee8-938e-92ef2ca0db10\scena
DATE: 08-23-2022 TIME: 11:41:14
USER:
COMMENTS:

```

```

*****
** SIMULATION : C. 10yr 3hr 10min Chicago **
*****

```

```

CHICAGO STORM IDF curve parameters: A= 622.842
Ptotal= 49.52 mm B= 0.000
C= 0.699

```

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

| TIME | RAIN  | TIME | RAIN   | TIME | RAIN  | TIME | RAIN  |
|------|-------|------|--------|------|-------|------|-------|
| hrs  | mm/hr | hrs  | mm/hr  | hrs  | mm/hr | hrs  | mm/hr |
| 0.00 | 5.79  | 0.83 | 124.56 | 1.67 | 8.98  | 2.50 | 5.59  |
| 0.17 | 6.82  | 1.00 | 26.38  | 1.83 | 7.93  | 2.67 | 5.23  |
| 0.33 | 8.45  | 1.17 | 16.63  | 2.00 | 7.14  | 2.83 | 4.93  |
| 0.50 | 11.56 | 1.33 | 12.68  | 2.17 | 6.51  |      |       |
| 0.67 | 21.49 | 1.50 | 10.45  | 2.33 | 6.01  |      |       |

```

CALIB
NASHYD ( 0001) Area (ha)= 4.32 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.36

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 5.79  | 0.833 | 21.49  | 1.583 | 10.45 | 2.33 | 6.51  |
| 0.167 | 5.79  | 0.917 | 124.56 | 1.667 | 10.45 | 2.42 | 6.01  |
| 0.250 | 6.82  | 1.000 | 124.56 | 1.750 | 8.98  | 2.50 | 6.01  |
| 0.333 | 6.82  | 1.083 | 26.38  | 1.833 | 8.98  | 2.58 | 5.59  |
| 0.417 | 8.45  | 1.167 | 26.38  | 1.917 | 7.93  | 2.67 | 5.59  |
| 0.500 | 8.45  | 1.250 | 16.63  | 2.000 | 7.93  | 2.75 | 5.23  |
| 0.583 | 11.56 | 1.333 | 16.63  | 2.083 | 7.14  | 2.83 | 5.23  |
| 0.667 | 11.56 | 1.417 | 12.68  | 2.167 | 7.14  | 2.92 | 4.93  |
| 0.750 | 21.49 | 1.500 | 12.68  | 2.250 | 6.51  | 3.00 | 4.93  |

Unit Hyd Qpeak (cms) = 0.458

PEAK FLOW (cms) = 0.119 (i)  
TIME TO PEAK (hrs) = 1.417  
RUNOFF VOLUME (mm) = 13.718  
TOTAL RAINFALL (mm) = 49.520  
RUNOFF COEFFICIENT = 0.277

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD ( 0002) Area (ha)= 13.33 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.62

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 5.79  | 0.833 | 21.49  | 1.583 | 10.45 | 2.33 | 6.51  |
| 0.167 | 5.79  | 0.917 | 124.56 | 1.667 | 10.45 | 2.42 | 6.01  |
| 0.250 | 6.82  | 1.000 | 124.56 | 1.750 | 8.98  | 2.50 | 6.01  |
| 0.333 | 6.82  | 1.083 | 26.38  | 1.833 | 8.98  | 2.58 | 5.59  |
| 0.417 | 8.45  | 1.167 | 26.38  | 1.917 | 7.93  | 2.67 | 5.59  |
| 0.500 | 8.45  | 1.250 | 16.63  | 2.000 | 7.93  | 2.75 | 5.23  |
| 0.583 | 11.56 | 1.333 | 16.63  | 2.083 | 7.14  | 2.83 | 5.23  |
| 0.667 | 11.56 | 1.417 | 12.68  | 2.167 | 7.14  | 2.92 | 4.93  |
| 0.750 | 21.49 | 1.500 | 12.68  | 2.250 | 6.51  | 3.00 | 4.93  |

Unit Hyd Qpeak (cms) = 0.821

PEAK FLOW (cms) = 0.261 (i)  
TIME TO PEAK (hrs) = 1.833  
RUNOFF VOLUME (mm) = 13.721  
TOTAL RAINFALL (mm) = 49.520  
RUNOFF COEFFICIENT = 0.277

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD ( 0003) Area (ha)= 3.05 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.22

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 5.79  | 0.833 | 21.49  | 1.583 | 10.45 | 2.33 | 6.51  |
| 0.167 | 5.79  | 0.917 | 124.56 | 1.667 | 10.45 | 2.42 | 6.01  |
| 0.250 | 6.82  | 1.000 | 124.56 | 1.750 | 8.98  | 2.50 | 6.01  |
| 0.333 | 6.82  | 1.083 | 26.38  | 1.833 | 8.98  | 2.58 | 5.59  |
| 0.417 | 8.45  | 1.167 | 26.38  | 1.917 | 7.93  | 2.67 | 5.59  |
| 0.500 | 8.45  | 1.250 | 16.63  | 2.000 | 7.93  | 2.75 | 5.23  |

|       |       |       |       |       |      |      |      |
|-------|-------|-------|-------|-------|------|------|------|
| 0.583 | 11.56 | 1.333 | 16.63 | 2.083 | 7.14 | 2.83 | 5.23 |
| 0.667 | 11.56 | 1.417 | 12.68 | 2.167 | 7.14 | 2.92 | 4.93 |
| 0.750 | 21.49 | 1.500 | 12.68 | 2.250 | 6.51 | 3.00 | 4.93 |

Unit Hyd Qpeak (cms) = 0.530

PEAK FLOW (cms) = 0.115 (i)  
TIME TO PEAK (hrs) = 1.167  
RUNOFF VOLUME (mm) = 13.703  
TOTAL RAINFALL (mm) = 49.520  
RUNOFF COEFFICIENT = 0.277

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD ( 0004) Area (ha)= 3.00 Curve Number (CN)= 74.0
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.17

```

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 5.79  | 0.833 | 21.49  | 1.583 | 10.45 | 2.33 | 6.51  |
| 0.167 | 5.79  | 0.917 | 124.56 | 1.667 | 10.45 | 2.42 | 6.01  |
| 0.250 | 6.82  | 1.000 | 124.56 | 1.750 | 8.98  | 2.50 | 6.01  |
| 0.333 | 6.82  | 1.083 | 26.38  | 1.833 | 8.98  | 2.58 | 5.59  |
| 0.417 | 8.45  | 1.167 | 26.38  | 1.917 | 7.93  | 2.67 | 5.59  |
| 0.500 | 8.45  | 1.250 | 16.63  | 2.000 | 7.93  | 2.75 | 5.23  |
| 0.583 | 11.56 | 1.333 | 16.63  | 2.083 | 7.14  | 2.83 | 5.23  |
| 0.667 | 11.56 | 1.417 | 12.68  | 2.167 | 7.14  | 2.92 | 4.93  |
| 0.750 | 21.49 | 1.500 | 12.68  | 2.250 | 6.51  | 3.00 | 4.93  |

Unit Hyd Qpeak (cms) = 0.674

PEAK FLOW (cms) = 0.131 (i)  
TIME TO PEAK (hrs) = 1.083  
RUNOFF VOLUME (mm) = 13.672  
TOTAL RAINFALL (mm) = 49.520  
RUNOFF COEFFICIENT = 0.276

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB

```

NASHYD ( 0005) | Area (ha)= 2.29 | Curve Number (CN)= 74.0  
 ID= 1 DT= 5.0 min | Ia (mm)= 7.00 | # of Linear Res.(N)= 3.00  
 U.H. Tp(hrs)= 0.40

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 5.79  | 0.833 | 21.49  | 1.583 | 10.45 | 2.33 | 6.51  |
| 0.167 | 5.79  | 0.917 | 124.56 | 1.667 | 10.45 | 2.42 | 6.01  |
| 0.250 | 6.82  | 1.000 | 124.56 | 1.750 | 8.98  | 2.50 | 6.01  |
| 0.333 | 6.82  | 1.083 | 26.38  | 1.833 | 8.98  | 2.58 | 5.59  |
| 0.417 | 8.45  | 1.167 | 26.38  | 1.917 | 7.93  | 2.67 | 5.59  |
| 0.500 | 8.45  | 1.250 | 16.63  | 2.000 | 7.93  | 2.75 | 5.23  |
| 0.583 | 11.56 | 1.333 | 16.63  | 2.083 | 7.14  | 2.83 | 5.23  |
| 0.667 | 11.56 | 1.417 | 12.68  | 2.167 | 7.14  | 2.92 | 4.93  |
| 0.750 | 21.49 | 1.500 | 12.68  | 2.250 | 6.51  | 3.00 | 4.93  |

Unit Hyd Qpeak (cms)= 0.219

PEAK FLOW (cms)= 0.059 (i)  
 TIME TO PEAK (hrs)= 1.500  
 RUNOFF VOLUME (mm)= 13.719  
 TOTAL RAINFALL (mm)= 49.520  
 RUNOFF COEFFICIENT = 0.277

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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V V I SSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

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```

\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voindat

Output filename:  
C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\2a293c  
c6c-5f29-47bd-a0cf-3c1334c578f2\scena

Summary filename:  
C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\2a293c  
c6c-5f29-47bd-a0cf-3c1334c578f2\scena

DATE: 08-23-2022 TIME: 11:41:14

USER:

COMMENTS: \_\_\_\_\_

```

*****
** SIMULATION : D. 25yr 3hr 10min Chicago **
*****

```

|                  |                                  |
|------------------|----------------------------------|
| CHICAGO STORM    | IDF curve parameters: A= 731.314 |
| Ptotal= 58.14 mm | B= 0.000                         |
|                  | C= 0.699                         |

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

| TIME | RAIN  | TIME | RAIN   | TIME | RAIN  | TIME | RAIN  |
|------|-------|------|--------|------|-------|------|-------|
| hrs  | mm/hr | hrs  | mm/hr  | hrs  | mm/hr | hrs  | mm/hr |
| 0.00 | 6.80  | 0.83 | 146.25 | 1.67 | 10.54 | 2.50 | 6.56  |
| 0.17 | 8.01  | 1.00 | 30.97  | 1.83 | 9.31  | 2.67 | 6.14  |
| 0.33 | 9.92  | 1.17 | 19.53  | 2.00 | 8.38  | 2.83 | 5.79  |
| 0.50 | 13.57 | 1.33 | 14.89  | 2.17 | 7.65  |      |       |
| 0.67 | 25.24 | 1.50 | 12.27  | 2.33 | 7.05  |      |       |

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 4.32    | Curve Number (CN)= 74.0   |
| NASHYD ( 0001)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.36 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 6.80  | 0.833 | 25.24  | 1.583 | 12.27 | 2.33 | 7.65  |
| 0.167 | 6.80  | 0.917 | 146.25 | 1.667 | 12.27 | 2.42 | 7.05  |
| 0.250 | 8.01  | 1.000 | 146.25 | 1.750 | 10.54 | 2.50 | 7.05  |
| 0.333 | 8.01  | 1.083 | 30.97  | 1.833 | 10.54 | 2.58 | 6.56  |
| 0.417 | 9.92  | 1.167 | 30.97  | 1.917 | 9.31  | 2.67 | 6.56  |
| 0.500 | 9.92  | 1.250 | 19.53  | 2.000 | 9.31  | 2.75 | 6.14  |
| 0.583 | 13.57 | 1.333 | 19.53  | 2.083 | 8.38  | 2.83 | 6.14  |
| 0.667 | 13.57 | 1.417 | 14.89  | 2.167 | 8.38  | 2.92 | 5.79  |
| 0.750 | 25.24 | 1.500 | 14.89  | 2.250 | 7.65  | 3.00 | 5.79  |

Unit Hyd Qpeak (cms)= 0.458

PEAK FLOW (cms)= 0.166 (i)  
TIME TO PEAK (hrs)= 1.417  
RUNOFF VOLUME (mm)= 18.628  
TOTAL RAINFALL (mm)= 58.144  
RUNOFF COEFFICIENT = 0.320

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 13.33   | Curve Number (CN)= 74.0   |
| NASHYD ( 0002)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.62 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 6.80  | 0.833 | 25.24  | 1.583 | 12.27 | 2.33 | 7.65  |
| 0.167 | 6.80  | 0.917 | 146.25 | 1.667 | 12.27 | 2.42 | 7.05  |
| 0.250 | 8.01  | 1.000 | 146.25 | 1.750 | 10.54 | 2.50 | 7.05  |
| 0.333 | 8.01  | 1.083 | 30.97  | 1.833 | 10.54 | 2.58 | 6.56  |
| 0.417 | 9.92  | 1.167 | 30.97  | 1.917 | 9.31  | 2.67 | 6.56  |
| 0.500 | 9.92  | 1.250 | 19.53  | 2.000 | 9.31  | 2.75 | 6.14  |
| 0.583 | 13.57 | 1.333 | 19.53  | 2.083 | 8.38  | 2.83 | 6.14  |
| 0.667 | 13.57 | 1.417 | 14.89  | 2.167 | 8.38  | 2.92 | 5.79  |
| 0.750 | 25.24 | 1.500 | 14.89  | 2.250 | 7.65  | 3.00 | 5.79  |

Unit Hyd Qpeak (cms)= 0.821

PEAK FLOW (cms)= 0.361 (i)  
TIME TO PEAK (hrs)= 1.750  
RUNOFF VOLUME (mm)= 18.631  
TOTAL RAINFALL (mm)= 58.144  
RUNOFF COEFFICIENT = 0.320

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 3.05    | Curve Number (CN)= 74.0   |
| NASHYD ( 0003)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.22 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 6.80  | 0.833 | 25.24  | 1.583 | 12.27 | 2.33 | 7.65  |
| 0.167 | 6.80  | 0.917 | 146.25 | 1.667 | 12.27 | 2.42 | 7.05  |
| 0.250 | 8.01  | 1.000 | 146.25 | 1.750 | 10.54 | 2.50 | 7.05  |
| 0.333 | 8.01  | 1.083 | 30.97  | 1.833 | 10.54 | 2.58 | 6.56  |
| 0.417 | 9.92  | 1.167 | 30.97  | 1.917 | 9.31  | 2.67 | 6.56  |
| 0.500 | 9.92  | 1.250 | 19.53  | 2.000 | 9.31  | 2.75 | 6.14  |

|       |       |       |       |       |      |      |      |
|-------|-------|-------|-------|-------|------|------|------|
| 0.583 | 13.57 | 1.333 | 19.53 | 2.083 | 8.38 | 2.83 | 6.14 |
| 0.667 | 13.57 | 1.417 | 14.89 | 2.167 | 8.38 | 2.92 | 5.79 |
| 0.750 | 25.24 | 1.500 | 14.89 | 2.250 | 7.65 | 3.00 | 5.79 |

Unit Hyd Qpeak (cms)= 0.530

PEAK FLOW (cms)= 0.162 (i)  
TIME TO PEAK (hrs)= 1.167  
RUNOFF VOLUME (mm)= 18.607  
TOTAL RAINFALL (mm)= 58.144  
RUNOFF COEFFICIENT = 0.320

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 3.00    | Curve Number (CN)= 74.0   |
| NASHYD ( 0004)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.17 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 6.80  | 0.833 | 25.24  | 1.583 | 12.27 | 2.33 | 7.65  |
| 0.167 | 6.80  | 0.917 | 146.25 | 1.667 | 12.27 | 2.42 | 7.05  |
| 0.250 | 8.01  | 1.000 | 146.25 | 1.750 | 10.54 | 2.50 | 7.05  |
| 0.333 | 8.01  | 1.083 | 30.97  | 1.833 | 10.54 | 2.58 | 6.56  |
| 0.417 | 9.92  | 1.167 | 30.97  | 1.917 | 9.31  | 2.67 | 6.56  |
| 0.500 | 9.92  | 1.250 | 19.53  | 2.000 | 9.31  | 2.75 | 6.14  |
| 0.583 | 13.57 | 1.333 | 19.53  | 2.083 | 8.38  | 2.83 | 6.14  |
| 0.667 | 13.57 | 1.417 | 14.89  | 2.167 | 8.38  | 2.92 | 5.79  |
| 0.750 | 25.24 | 1.500 | 14.89  | 2.250 | 7.65  | 3.00 | 5.79  |

Unit Hyd Qpeak (cms)= 0.674

PEAK FLOW (cms)= 0.186 (i)  
TIME TO PEAK (hrs)= 1.083  
RUNOFF VOLUME (mm)= 18.565  
TOTAL RAINFALL (mm)= 58.144  
RUNOFF COEFFICIENT = 0.319

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|       |  |
|-------|--|
| CALIB |  |
|-------|--|



NASHYD ( 0005) | Area (ha)= 2.29 | Curve Number (CN)= 74.0  
 ID= 1 DT= 5.0 min | Ia (mm)= 7.00 | # of Linear Res.(N)= 3.00  
 -----  
 U.H. Tp(hrs)= 0.40

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 6.80  | 0.833 | 25.24  | 1.583 | 12.27 | 2.33 | 7.65  |
| 0.167 | 6.80  | 0.917 | 146.25 | 1.667 | 12.27 | 2.42 | 7.05  |
| 0.250 | 8.01  | 1.000 | 146.25 | 1.750 | 10.54 | 2.50 | 7.05  |
| 0.333 | 8.01  | 1.083 | 30.97  | 1.833 | 10.54 | 2.58 | 6.56  |
| 0.417 | 9.92  | 1.167 | 30.97  | 1.917 | 9.31  | 2.67 | 6.56  |
| 0.500 | 9.92  | 1.250 | 19.53  | 2.000 | 9.31  | 2.75 | 6.14  |
| 0.583 | 13.57 | 1.333 | 19.53  | 2.083 | 8.38  | 2.83 | 6.14  |
| 0.667 | 13.57 | 1.417 | 14.89  | 2.167 | 8.38  | 2.92 | 5.79  |
| 0.750 | 25.24 | 1.500 | 14.89  | 2.250 | 7.65  | 3.00 | 5.79  |

Unit Hyd Qpeak (cms)= 0.219

PEAK FLOW (cms)= 0.082 (i)  
 TIME TO PEAK (hrs)= 1.417  
 RUNOFF VOLUME (mm)= 18.629  
 TOTAL RAINFALL (mm)= 58.144  
 RUNOFF COEFFICIENT = 0.320

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----

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V V I SSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

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```

\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voin.dat

Output filename:  
C:\Users\kswin\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\c7d917e4-5d43-485b-9fd7-58c9707dae8e\scena

Summary filename:  
C:\Users\kswin\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\c7d917e4-5d43-485b-9fd7-58c9707dae8e\scena

DATE: 08-23-2022 TIME: 11:41:14

USER:

COMMENTS: \_\_\_\_\_

```

*****
** SIMULATION : E. 50yr 3hr 10min Chicago **
*****

```

|                  |                                  |
|------------------|----------------------------------|
| CHICAGO STORM    | IDF curve parameters: A= 811.794 |
| Ptotal= 64.54 mm | B= 0.000                         |
|                  | C= 0.699                         |

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

| TIME | RAIN  | TIME  | RAIN   | TIME | RAIN  | TIME | RAIN  |
|------|-------|-------|--------|------|-------|------|-------|
| hrs  | mm/hr | hrs   | mm/hr  | hrs  | mm/hr | hrs  | mm/hr |
| 0.00 | 7.55  | 0.833 | 162.35 | 1.67 | 11.70 | 2.50 | 7.28  |
| 0.17 | 8.89  | 1.00  | 34.38  | 1.83 | 10.34 | 2.67 | 6.82  |
| 0.33 | 11.01 | 1.17  | 21.68  | 2.00 | 9.30  | 2.83 | 6.42  |
| 0.50 | 15.06 | 1.33  | 16.53  | 2.17 | 8.49  |      |       |
| 0.67 | 28.01 | 1.50  | 13.62  | 2.33 | 7.83  |      |       |

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 4.32    | Curve Number (CN)= 74.0   |
| NASHYD ( 0001)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.36 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 7.55  | 0.833 | 28.01  | 1.583 | 13.62 | 2.33 | 8.49  |
| 0.167 | 7.55  | 0.917 | 162.35 | 1.667 | 13.62 | 2.42 | 7.83  |
| 0.250 | 8.89  | 1.000 | 162.35 | 1.750 | 11.70 | 2.50 | 7.83  |
| 0.333 | 8.89  | 1.083 | 34.38  | 1.833 | 11.70 | 2.58 | 7.28  |
| 0.417 | 11.01 | 1.167 | 34.38  | 1.917 | 10.34 | 2.67 | 7.28  |
| 0.500 | 11.01 | 1.250 | 21.68  | 2.000 | 10.34 | 2.75 | 6.82  |
| 0.583 | 15.06 | 1.333 | 21.68  | 2.083 | 9.30  | 2.83 | 6.82  |
| 0.667 | 15.06 | 1.417 | 16.53  | 2.167 | 9.30  | 2.92 | 6.42  |
| 0.750 | 28.01 | 1.500 | 16.53  | 2.250 | 8.49  | 3.00 | 6.42  |

Unit Hyd Qpeak (cms)= 0.458

PEAK FLOW (cms)= 0.204 (i)  
TIME TO PEAK (hrs)= 1.417  
RUNOFF VOLUME (mm)= 22.553  
TOTAL RAINFALL (mm)= 64.542  
RUNOFF COEFFICIENT = 0.349

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 13.33   | Curve Number (CN)= 74.0   |
| NASHYD ( 0002)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.62 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 7.55  | 0.833 | 28.01  | 1.583 | 13.62 | 2.33 | 8.49  |
| 0.167 | 7.55  | 0.917 | 162.35 | 1.667 | 13.62 | 2.42 | 7.83  |
| 0.250 | 8.89  | 1.000 | 162.35 | 1.750 | 11.70 | 2.50 | 7.83  |
| 0.333 | 8.89  | 1.083 | 34.38  | 1.833 | 11.70 | 2.58 | 7.28  |
| 0.417 | 11.01 | 1.167 | 34.38  | 1.917 | 10.34 | 2.67 | 7.28  |
| 0.500 | 11.01 | 1.250 | 21.68  | 2.000 | 10.34 | 2.75 | 6.82  |
| 0.583 | 15.06 | 1.333 | 21.68  | 2.083 | 9.30  | 2.83 | 6.82  |
| 0.667 | 15.06 | 1.417 | 16.53  | 2.167 | 9.30  | 2.92 | 6.42  |
| 0.750 | 28.01 | 1.500 | 16.53  | 2.250 | 8.49  | 3.00 | 6.42  |

Unit Hyd Qpeak (cms)= 0.821

PEAK FLOW (cms)= 0.442 (i)  
TIME TO PEAK (hrs)= 1.750  
RUNOFF VOLUME (mm)= 22.557  
TOTAL RAINFALL (mm)= 64.542  
RUNOFF COEFFICIENT = 0.349

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 3.05    | Curve Number (CN)= 74.0   |
| NASHYD ( 0003)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.22 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 7.55  | 0.833 | 28.01  | 1.583 | 13.62 | 2.33 | 8.49  |
| 0.167 | 7.55  | 0.917 | 162.35 | 1.667 | 13.62 | 2.42 | 7.83  |
| 0.250 | 8.89  | 1.000 | 162.35 | 1.750 | 11.70 | 2.50 | 7.83  |
| 0.333 | 8.89  | 1.083 | 34.38  | 1.833 | 11.70 | 2.58 | 7.28  |
| 0.417 | 11.01 | 1.167 | 34.38  | 1.917 | 10.34 | 2.67 | 7.28  |
| 0.500 | 11.01 | 1.250 | 21.68  | 2.000 | 10.34 | 2.75 | 6.82  |

|       |       |       |       |       |      |      |      |
|-------|-------|-------|-------|-------|------|------|------|
| 0.583 | 15.06 | 1.333 | 21.68 | 2.083 | 9.30 | 2.83 | 6.82 |
| 0.667 | 15.06 | 1.417 | 16.53 | 2.167 | 9.30 | 2.92 | 6.42 |
| 0.750 | 28.01 | 1.500 | 16.53 | 2.250 | 8.49 | 3.00 | 6.42 |

Unit Hyd Qpeak (cms)= 0.530

PEAK FLOW (cms)= 0.200 (i)  
TIME TO PEAK (hrs)= 1.167  
RUNOFF VOLUME (mm)= 22.528  
TOTAL RAINFALL (mm)= 64.542  
RUNOFF COEFFICIENT = 0.349

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 3.00    | Curve Number (CN)= 74.0   |
| NASHYD ( 0004)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.17 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 7.55  | 0.833 | 28.01  | 1.583 | 13.62 | 2.33 | 8.49  |
| 0.167 | 7.55  | 0.917 | 162.35 | 1.667 | 13.62 | 2.42 | 7.83  |
| 0.250 | 8.89  | 1.000 | 162.35 | 1.750 | 11.70 | 2.50 | 7.83  |
| 0.333 | 8.89  | 1.083 | 34.38  | 1.833 | 11.70 | 2.58 | 7.28  |
| 0.417 | 11.01 | 1.167 | 34.38  | 1.917 | 10.34 | 2.67 | 7.28  |
| 0.500 | 11.01 | 1.250 | 21.68  | 2.000 | 10.34 | 2.75 | 6.82  |
| 0.583 | 15.06 | 1.333 | 21.68  | 2.083 | 9.30  | 2.83 | 6.82  |
| 0.667 | 15.06 | 1.417 | 16.53  | 2.167 | 9.30  | 2.92 | 6.42  |
| 0.750 | 28.01 | 1.500 | 16.53  | 2.250 | 8.49  | 3.00 | 6.42  |

Unit Hyd Qpeak (cms)= 0.674

PEAK FLOW (cms)= 0.230 (i)  
TIME TO PEAK (hrs)= 1.083  
RUNOFF VOLUME (mm)= 22.477  
TOTAL RAINFALL (mm)= 64.542  
RUNOFF COEFFICIENT = 0.348

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|       |  |
|-------|--|
| CALIB |  |
|-------|--|

NASHYD ( 0005) | Area (ha)= 2.29 | Curve Number (CN)= 74.0  
 ID= 1 DT= 5.0 min | Ia (mm)= 7.00 | # of Linear Res.(N)= 3.00  
 -----  
 U.H. Tp(hrs)= 0.40

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 7.55  | 0.833 | 28.01  | 1.583 | 13.62 | 2.33 | 8.49  |
| 0.167 | 7.55  | 0.917 | 162.35 | 1.667 | 13.62 | 2.42 | 7.83  |
| 0.250 | 8.89  | 1.000 | 162.35 | 1.750 | 11.70 | 2.50 | 7.83  |
| 0.333 | 8.89  | 1.083 | 34.38  | 1.833 | 11.70 | 2.58 | 7.28  |
| 0.417 | 11.01 | 1.167 | 34.38  | 1.917 | 10.34 | 2.67 | 7.28  |
| 0.500 | 11.01 | 1.250 | 21.68  | 2.000 | 10.34 | 2.75 | 6.82  |
| 0.583 | 15.06 | 1.333 | 21.68  | 2.083 | 9.30  | 2.83 | 6.82  |
| 0.667 | 15.06 | 1.417 | 16.53  | 2.167 | 9.30  | 2.92 | 6.42  |
| 0.750 | 28.01 | 1.500 | 16.53  | 2.250 | 8.49  | 3.00 | 6.42  |

Unit Hyd Qpeak (cms)= 0.219

PEAK FLOW (cms)= 0.101 (i)  
 TIME TO PEAK (hrs)= 1.417  
 RUNOFF VOLUME (mm)= 22.554  
 TOTAL RAINFALL (mm)= 64.542  
 RUNOFF COEFFICIENT = 0.349

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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V V I SSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

```

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vojn.dat  
 Output filename:  
 C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\aa5ac  
 d95-4301-4ff3-8e1d-3d4cf191cf4\scena  
 Summary filename:  
 C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\aa5ac  
 d95-4301-4ff3-8e1d-3d4cf191cf4\scena

DATE: 08-23-2022 TIME: 11:41:14

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
 \*\* SIMULATION : F. 100yr 3hr 10min Chicago \*\*  
 \*\*\*\*\*

|                  |                                  |
|------------------|----------------------------------|
| CHICAGO STORM    | IDF curve parameters: A= 892.273 |
| Ptotal= 70.94 mm | B= 0.000                         |
|                  | C= 0.699                         |

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

| TIME | RAIN  | TIME | RAIN   | TIME | RAIN  | TIME | RAIN  |
|------|-------|------|--------|------|-------|------|-------|
| hrs  | mm/hr | hrs  | mm/hr  | hrs  | mm/hr | hrs  | mm/hr |
| 0.00 | 8.30  | 0.83 | 178.44 | 1.67 | 12.86 | 2.50 | 8.00  |
| 0.17 | 9.77  | 1.00 | 37.79  | 1.83 | 11.36 | 2.67 | 7.50  |
| 0.33 | 12.10 | 1.17 | 23.83  | 2.00 | 10.23 | 2.83 | 7.06  |
| 0.50 | 16.55 | 1.33 | 18.17  | 2.17 | 9.33  |      |       |
| 0.67 | 30.79 | 1.50 | 14.97  | 2.33 | 8.61  |      |       |

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 4.32    | Curve Number (CN)= 74.0   |
| NASHYD ( 0001)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.36 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 8.30  | 0.833 | 30.79  | 1.583 | 14.97 | 2.33 | 9.33  |
| 0.167 | 8.30  | 0.917 | 178.44 | 1.667 | 14.97 | 2.42 | 8.61  |
| 0.250 | 9.77  | 1.000 | 178.44 | 1.750 | 12.86 | 2.50 | 8.61  |
| 0.333 | 9.77  | 1.083 | 37.79  | 1.833 | 12.86 | 2.58 | 8.00  |
| 0.417 | 12.10 | 1.167 | 37.79  | 1.917 | 11.36 | 2.67 | 8.00  |
| 0.500 | 12.10 | 1.250 | 23.83  | 2.000 | 11.36 | 2.75 | 7.50  |
| 0.583 | 16.55 | 1.333 | 23.83  | 2.083 | 10.23 | 2.83 | 7.50  |
| 0.667 | 16.55 | 1.417 | 18.17  | 2.167 | 10.23 | 2.92 | 7.06  |
| 0.750 | 30.79 | 1.500 | 18.17  | 2.250 | 9.33  | 3.00 | 7.06  |

Unit Hyd Qpeak (cms)= 0.458

PEAK FLOW (cms)= 0.244 (i)  
 TIME TO PEAK (hrs)= 1.333  
 RUNOFF VOLUME (mm)= 26.684  
 TOTAL RAINFALL (mm)= 70.941  
 RUNOFF COEFFICIENT = 0.376

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 13.33   | Curve Number (CN)= 74.0   |
| NASHYD ( 0002)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.62 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 8.30  | 0.833 | 30.79  | 1.583 | 14.97 | 2.33 | 9.33  |
| 0.167 | 8.30  | 0.917 | 178.44 | 1.667 | 14.97 | 2.42 | 8.61  |
| 0.250 | 9.77  | 1.000 | 178.44 | 1.750 | 12.86 | 2.50 | 8.61  |
| 0.333 | 9.77  | 1.083 | 37.79  | 1.833 | 12.86 | 2.58 | 8.00  |
| 0.417 | 12.10 | 1.167 | 37.79  | 1.917 | 11.36 | 2.67 | 8.00  |
| 0.500 | 12.10 | 1.250 | 23.83  | 2.000 | 11.36 | 2.75 | 7.50  |
| 0.583 | 16.55 | 1.333 | 23.83  | 2.083 | 10.23 | 2.83 | 7.50  |
| 0.667 | 16.55 | 1.417 | 18.17  | 2.167 | 10.23 | 2.92 | 7.06  |
| 0.750 | 30.79 | 1.500 | 18.17  | 2.250 | 9.33  | 3.00 | 7.06  |

Unit Hyd Qpeak (cms)= 0.821

PEAK FLOW (cms)= 0.528 (i)  
 TIME TO PEAK (hrs)= 1.750  
 RUNOFF VOLUME (mm)= 26.689  
 TOTAL RAINFALL (mm)= 70.941  
 RUNOFF COEFFICIENT = 0.376

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 3.05    | Curve Number (CN)= 74.0   |
| NASHYD ( 0003)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.22 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 8.30  | 0.833 | 30.79  | 1.583 | 14.97 | 2.33 | 9.33  |
| 0.167 | 8.30  | 0.917 | 178.44 | 1.667 | 14.97 | 2.42 | 8.61  |
| 0.250 | 9.77  | 1.000 | 178.44 | 1.750 | 12.86 | 2.50 | 8.61  |
| 0.333 | 9.77  | 1.083 | 37.79  | 1.833 | 12.86 | 2.58 | 8.00  |
| 0.417 | 12.10 | 1.167 | 37.79  | 1.917 | 11.36 | 2.67 | 8.00  |
| 0.500 | 12.10 | 1.250 | 23.83  | 2.000 | 11.36 | 2.75 | 7.50  |

|       |       |       |       |       |       |      |      |
|-------|-------|-------|-------|-------|-------|------|------|
| 0.583 | 16.55 | 1.333 | 23.83 | 2.083 | 10.23 | 2.83 | 7.50 |
| 0.667 | 16.55 | 1.417 | 18.17 | 2.167 | 10.23 | 2.92 | 7.06 |
| 0.750 | 30.79 | 1.500 | 18.17 | 2.250 | 9.33  | 3.00 | 7.06 |

Unit Hyd Qpeak (cms)= 0.530

PEAK FLOW (cms)= 0.241 (i)  
 TIME TO PEAK (hrs)= 1.167  
 RUNOFF VOLUME (mm)= 26.654  
 TOTAL RAINFALL (mm)= 70.941  
 RUNOFF COEFFICIENT = 0.376

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|                   |                    |                           |
|-------------------|--------------------|---------------------------|
| CALIB             | Area (ha)= 3.00    | Curve Number (CN)= 74.0   |
| NASHYD ( 0004)    | Ia (mm)= 7.00      | # of Linear Res.(N)= 3.00 |
| ID= 1 DT= 5.0 min | U.H. Tp(hrs)= 0.17 |                           |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 8.30  | 0.833 | 30.79  | 1.583 | 14.97 | 2.33 | 9.33  |
| 0.167 | 8.30  | 0.917 | 178.44 | 1.667 | 14.97 | 2.42 | 8.61  |
| 0.250 | 9.77  | 1.000 | 178.44 | 1.750 | 12.86 | 2.50 | 8.61  |
| 0.333 | 9.77  | 1.083 | 37.79  | 1.833 | 12.86 | 2.58 | 8.00  |
| 0.417 | 12.10 | 1.167 | 37.79  | 1.917 | 11.36 | 2.67 | 8.00  |
| 0.500 | 12.10 | 1.250 | 23.83  | 2.000 | 11.36 | 2.75 | 7.50  |
| 0.583 | 16.55 | 1.333 | 23.83  | 2.083 | 10.23 | 2.83 | 7.50  |
| 0.667 | 16.55 | 1.417 | 18.17  | 2.167 | 10.23 | 2.92 | 7.06  |
| 0.750 | 30.79 | 1.500 | 18.17  | 2.250 | 9.33  | 3.00 | 7.06  |

Unit Hyd Qpeak (cms)= 0.674

PEAK FLOW (cms)= 0.278 (i)  
 TIME TO PEAK (hrs)= 1.083  
 RUNOFF VOLUME (mm)= 26.594  
 TOTAL RAINFALL (mm)= 70.941  
 RUNOFF COEFFICIENT = 0.375

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|       |  |
|-------|--|
| CALIB |  |
|-------|--|

NASHYD ( 0005) | Area (ha)= 2.29 | Curve Number (CN)= 74.0  
 ID= 1 DT= 5.0 min | Ia (mm)= 7.00 | # of Linear Res.(N)= 3.00  
 ----- | U.H. Tp(hrs)= 0.40

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 8.30  | 0.833 | 30.79  | 1.583 | 14.97 | 2.33 | 9.33  |
| 0.167 | 8.30  | 0.917 | 178.44 | 1.667 | 14.97 | 2.42 | 8.61  |
| 0.250 | 9.77  | 1.000 | 178.44 | 1.750 | 12.86 | 2.50 | 8.61  |
| 0.333 | 9.77  | 1.083 | 37.79  | 1.833 | 12.86 | 2.58 | 8.00  |
| 0.417 | 12.10 | 1.167 | 37.79  | 1.917 | 11.36 | 2.67 | 8.00  |
| 0.500 | 12.10 | 1.250 | 23.83  | 2.000 | 11.36 | 2.75 | 7.50  |
| 0.583 | 16.55 | 1.333 | 23.83  | 2.083 | 10.23 | 2.83 | 7.50  |
| 0.667 | 16.55 | 1.417 | 18.17  | 2.167 | 10.23 | 2.92 | 7.06  |
| 0.750 | 30.79 | 1.500 | 18.17  | 2.250 | 9.33  | 3.00 | 7.06  |

Unit Hyd Qpeak (cms)= 0.219

PEAK FLOW (cms)= 0.121 (i)  
 TIME TO PEAK (hrs)= 1.417  
 RUNOFF VOLUME (mm)= 26.686  
 TOTAL RAINFALL (mm)= 70.941  
 RUNOFF COEFFICIENT = 0.376

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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 FINISH  
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V V I SSSS U U A L (v 6.2.2008)
V V I SS U U A A L
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000 TTTT TTTT H H Y Y M M 000 TM
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O O T T H H Y Y M M O O
000 T T H H Y Y M M 000

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\*\*\*\* DETAILED OUTPUT \*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vojn.dat

Output filename:
C:\Users\kswin\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\c5ec4
37e-755f-46f3-a3ca-11f378bd0c1c\scena
Summary filename:
C:\Users\kswin\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\c5ec4
37e-755f-46f3-a3ca-11f378bd0c1c\scena

DATE: 08-23-2022 TIME: 11:47:06

USER:

COMMENTS:

\*\* SIMULATION : G. 2yr 24hr 15min SCS Type II \*\*

READ STORM Filename: C:\Users\kswin\AppData\Local\Temp\

Ptotal= 60.13 mm | 8ab56956-7981-44cd-9ed6-5a18ca9f22ac\16ec51c6
Comments: G. 2yr 24hr 15min SCS Type II

Table with 10 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals and corresponding rainfall amounts in mm/hr.

CALIB NASHDY ( 0001) Area (ha)= 4.32 Curve Number (CN)= 74.0
Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.36

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 10 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data.

Table with 10 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show detailed simulation data for various time intervals.

Table with 10 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show detailed simulation data for various time intervals.

Unit Hyd Qpeak (cms)= 0.458

PEAK FLOW (cms)= 0.143 (i)
TIME TO PEAK (hrs)= 12.500
RUNOFF VOLUME (mm)= 19.823
TOTAL RAINFALL (mm)= 60.130
RUNOFF COEFFICIENT = 0.330

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHDY ( 0002) Area (ha)= 13.33 Curve Number (CN)= 74.0
Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.62

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 10 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data for the second simulation.







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V V I SSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

```

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voindat  
 Output filename:  
 C:\Users\kswin\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\4e86fb0-1ea3-43b7-9518-2160a4ec80a1\scena  
 Summary filename:  
 C:\Users\kswin\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\4e86fb0-1ea3-43b7-9518-2160a4ec80a1\scena

DATE: 08-23-2022 TIME: 11:47:06  
 USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
 \*\* SIMULATION : H. 5yr 24hr 15min SCS Type II \*\*  
 \*\*\*\*\*

READ STORM Filename: C:\Users\kswin\AppData\Local\Temp\

Ptotal= 79.65 mm | 8ab56956-7981-44cd-9ed6-5a18ca9f22ac\44344e77  
 Comments: H. 5yr 24hr 15min SCS Type II

| TIME | RAIN  | TIME  | RAIN  | TIME  | RAIN  | TIME  | RAIN  |
|------|-------|-------|-------|-------|-------|-------|-------|
| hrs  | mm/hr | hrs   | mm/hr | hrs   | mm/hr | hrs   | mm/hr |
| 0.00 | 0.00  | 6.25  | 1.43  | 12.50 | 11.47 | 18.75 | 1.43  |
| 0.25 | 0.88  | 6.50  | 1.43  | 12.75 | 5.89  | 19.00 | 1.43  |
| 0.50 | 0.88  | 6.75  | 1.43  | 13.00 | 5.89  | 19.25 | 1.43  |
| 0.75 | 0.88  | 7.00  | 1.43  | 13.25 | 4.30  | 19.50 | 1.43  |
| 1.00 | 0.88  | 7.25  | 1.75  | 13.50 | 4.30  | 19.75 | 1.43  |
| 1.25 | 0.88  | 7.50  | 1.75  | 13.75 | 3.35  | 20.00 | 1.43  |
| 1.50 | 0.88  | 7.75  | 1.75  | 14.00 | 3.35  | 20.25 | 0.96  |
| 1.75 | 0.88  | 8.00  | 1.75  | 14.25 | 2.39  | 20.50 | 0.96  |
| 2.00 | 0.88  | 8.25  | 2.07  | 14.50 | 2.39  | 20.75 | 0.96  |
| 2.25 | 1.04  | 8.50  | 2.07  | 14.75 | 2.39  | 21.00 | 0.96  |
| 2.50 | 1.04  | 8.75  | 2.23  | 15.00 | 2.39  | 21.25 | 0.96  |
| 2.75 | 1.04  | 9.00  | 2.23  | 15.25 | 2.39  | 21.50 | 0.96  |
| 3.00 | 1.04  | 9.25  | 2.55  | 15.50 | 2.39  | 21.75 | 0.96  |
| 3.25 | 1.04  | 9.50  | 2.55  | 15.75 | 2.39  | 22.00 | 0.96  |
| 3.50 | 1.04  | 9.75  | 2.87  | 16.00 | 2.39  | 22.25 | 0.96  |
| 3.75 | 1.04  | 10.00 | 2.87  | 16.25 | 1.43  | 22.50 | 0.96  |
| 4.00 | 1.04  | 10.25 | 3.66  | 16.50 | 1.43  | 22.75 | 0.96  |
| 4.25 | 1.27  | 10.50 | 3.66  | 16.75 | 1.43  | 23.00 | 0.96  |
| 4.50 | 1.27  | 10.75 | 4.94  | 17.00 | 1.43  | 23.25 | 0.96  |
| 4.75 | 1.27  | 11.00 | 4.94  | 17.25 | 1.43  | 23.50 | 0.96  |
| 5.00 | 1.27  | 11.25 | 7.65  | 17.50 | 1.43  | 23.75 | 0.96  |
| 5.25 | 1.27  | 11.50 | 7.65  | 17.75 | 1.43  | 24.00 | 0.96  |
| 5.50 | 1.27  | 11.75 | 23.58 | 18.00 | 1.43  |       |       |
| 5.75 | 1.27  | 12.00 | 97.49 | 18.25 | 1.43  |       |       |
| 6.00 | 1.27  | 12.25 | 11.47 | 18.50 | 1.43  |       |       |

CALIB NASHYD ( 0001) Area (ha)= 4.32 Curve Number (CN)= 74.0  
 ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00  
 U.H. Tp(hrs)= 0.36

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN  | TIME   | RAIN  | TIME  | RAIN  |
|-------|-------|-------|-------|--------|-------|-------|-------|
| hrs   | mm/hr | hrs   | mm/hr | hrs    | mm/hr | hrs   | mm/hr |
| 0.083 | 0.00  | 6.167 | 1.27  | 12.250 | 97.49 | 18.33 | 1.43  |
| 0.167 | 0.00  | 6.250 | 1.27  | 12.333 | 11.48 | 18.42 | 1.43  |
| 0.250 | 0.00  | 6.333 | 1.43  | 12.417 | 11.47 | 18.50 | 1.43  |
| 0.333 | 0.88  | 6.417 | 1.43  | 12.500 | 11.47 | 18.58 | 1.43  |
| 0.417 | 0.88  | 6.500 | 1.43  | 12.583 | 11.47 | 18.67 | 1.43  |

|       |      |        |      |        |       |       |      |
|-------|------|--------|------|--------|-------|-------|------|
| 0.500 | 0.88 | 6.583  | 1.43 | 12.667 | 11.47 | 18.75 | 1.43 |
| 0.583 | 0.88 | 6.667  | 1.43 | 12.750 | 11.47 | 18.83 | 1.43 |
| 0.667 | 0.88 | 6.750  | 1.43 | 12.833 | 5.89  | 18.92 | 1.43 |
| 0.750 | 0.88 | 6.833  | 1.43 | 12.917 | 5.89  | 19.00 | 1.43 |
| 0.833 | 0.88 | 6.917  | 1.43 | 13.000 | 5.89  | 19.08 | 1.43 |
| 0.917 | 0.88 | 7.000  | 1.43 | 13.083 | 5.89  | 19.17 | 1.43 |
| 1.000 | 0.88 | 7.083  | 1.43 | 13.167 | 5.89  | 19.25 | 1.43 |
| 1.083 | 0.88 | 7.167  | 1.43 | 13.250 | 5.89  | 19.33 | 1.43 |
| 1.167 | 0.88 | 7.250  | 1.43 | 13.333 | 4.30  | 19.42 | 1.43 |
| 1.250 | 0.88 | 7.333  | 1.75 | 13.417 | 4.30  | 19.50 | 1.43 |
| 1.333 | 0.88 | 7.417  | 1.75 | 13.500 | 4.30  | 19.58 | 1.43 |
| 1.417 | 0.88 | 7.500  | 1.75 | 13.583 | 4.30  | 19.67 | 1.43 |
| 1.500 | 0.88 | 7.583  | 1.75 | 13.667 | 4.30  | 19.75 | 1.43 |
| 1.583 | 0.88 | 7.667  | 1.75 | 13.750 | 4.30  | 19.83 | 1.43 |
| 1.667 | 0.88 | 7.750  | 1.75 | 13.833 | 3.35  | 19.92 | 1.43 |
| 1.750 | 0.88 | 7.833  | 1.75 | 13.917 | 3.35  | 20.00 | 1.43 |
| 1.833 | 0.88 | 7.917  | 1.75 | 14.000 | 3.35  | 20.08 | 1.43 |
| 1.917 | 0.88 | 8.000  | 1.75 | 14.083 | 3.35  | 20.17 | 1.43 |
| 2.000 | 0.88 | 8.083  | 1.75 | 14.167 | 3.35  | 20.25 | 1.43 |
| 2.083 | 0.88 | 8.167  | 1.75 | 14.250 | 3.35  | 20.33 | 0.96 |
| 2.167 | 0.88 | 8.250  | 1.75 | 14.333 | 2.39  | 20.42 | 0.96 |
| 2.250 | 0.88 | 8.333  | 2.07 | 14.417 | 2.39  | 20.50 | 0.96 |
| 2.333 | 1.04 | 8.417  | 2.07 | 14.500 | 2.39  | 20.58 | 0.96 |
| 2.417 | 1.04 | 8.500  | 2.07 | 14.583 | 2.39  | 20.67 | 0.96 |
| 2.500 | 1.04 | 8.583  | 2.07 | 14.667 | 2.39  | 20.75 | 0.96 |
| 2.583 | 1.04 | 8.667  | 2.07 | 14.750 | 2.39  | 20.83 | 0.96 |
| 2.667 | 1.04 | 8.750  | 2.07 | 14.833 | 2.39  | 20.92 | 0.96 |
| 2.750 | 1.04 | 8.833  | 2.23 | 14.917 | 2.39  | 21.00 | 0.96 |
| 2.833 | 1.04 | 8.917  | 2.23 | 15.000 | 2.39  | 21.08 | 0.96 |
| 2.917 | 1.04 | 9.000  | 2.23 | 15.083 | 2.39  | 21.17 | 0.96 |
| 3.000 | 1.04 | 9.083  | 2.23 | 15.167 | 2.39  | 21.25 | 0.96 |
| 3.083 | 1.04 | 9.167  | 2.23 | 15.250 | 2.39  | 21.33 | 0.96 |
| 3.167 | 1.04 | 9.250  | 2.23 | 15.333 | 2.39  | 21.42 | 0.96 |
| 3.250 | 1.04 | 9.333  | 2.55 | 15.417 | 2.39  | 21.50 | 0.96 |
| 3.333 | 1.04 | 9.417  | 2.55 | 15.500 | 2.39  | 21.58 | 0.96 |
| 3.417 | 1.04 | 9.500  | 2.55 | 15.583 | 2.39  | 21.67 | 0.96 |
| 3.500 | 1.04 | 9.583  | 2.55 | 15.667 | 2.39  | 21.75 | 0.96 |
| 3.583 | 1.04 | 9.667  | 2.55 | 15.750 | 2.39  | 21.83 | 0.96 |
| 3.667 | 1.04 | 9.750  | 2.55 | 15.833 | 2.39  | 21.92 | 0.96 |
| 3.750 | 1.04 | 9.833  | 2.87 | 15.917 | 2.39  | 22.00 | 0.96 |
| 3.833 | 1.04 | 9.917  | 2.87 | 16.000 | 2.39  | 22.08 | 0.96 |
| 3.917 | 1.04 | 10.000 | 2.87 | 16.083 | 2.39  | 22.17 | 0.96 |
| 4.000 | 1.04 | 10.083 | 2.87 | 16.167 | 2.39  | 22.25 | 0.96 |
| 4.083 | 1.04 | 10.167 | 2.87 | 16.250 | 2.39  | 22.33 | 0.96 |
| 4.167 | 1.04 | 10.250 | 2.87 | 16.333 | 1.43  | 22.42 | 0.96 |
| 4.250 | 1.04 | 10.333 | 3.66 | 16.417 | 1.43  | 22.50 | 0.96 |
| 4.333 | 1.27 | 10.417 | 3.66 | 16.500 | 1.43  | 22.58 | 0.96 |
| 4.417 | 1.27 | 10.500 | 3.66 | 16.583 | 1.43  | 22.67 | 0.96 |
| 4.500 | 1.27 | 10.583 | 3.66 | 16.667 | 1.43  | 22.75 | 0.96 |
| 4.583 | 1.27 | 10.667 | 3.66 | 16.750 | 1.43  | 22.83 | 0.96 |

Unit Hyd Qpeak (cms)= 0.458  
 PEAK FLOW (cms)= 0.239 (i)  
 TIME TO PEAK (hrs)= 12.500  
 RUNOFF VOLUME (mm)= 32.596  
 TOTAL RAINFALL (mm)= 79.650  
 RUNOFF COEFFICIENT = 0.409

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0002) Area (ha)= 13.33 Curve Number (CN)= 74.0  
 ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00  
 U.H. Tp(hrs)= 0.62

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

| TIME  | RAIN  | TIME  | RAIN  | TIME   | RAIN  | TIME  | RAIN  |
|-------|-------|-------|-------|--------|-------|-------|-------|
| hrs   | mm/hr | hrs   | mm/hr | hrs    | mm/hr | hrs   | mm/hr |
| 0.083 | 0.00  | 6.167 | 1.27  | 12.250 | 97.49 | 18.33 | 1.43  |
| 0.167 | 0.00  | 6.250 | 1.27  | 12.333 | 11.48 | 18.42 | 1.43  |
| 0.250 | 0.00  | 6.333 | 1.43  | 12.417 | 11.47 | 18.50 | 1.43  |
| 0.333 | 0.88  | 6.417 | 1.43  | 12.500 | 11.47 | 18.58 | 1.43  |
| 0.417 | 0.88  | 6.500 | 1.43  | 12.583 | 11.47 | 18.67 | 1.43  |
| 0.500 | 0.88  | 6.583 | 1.43  | 12.667 | 11.47 | 18.75 | 1.43  |
| 0.583 | 0.88  | 6.667 | 1.43  | 12.750 | 11.47 | 18.83 | 1.43  |
| 0.667 | 0.88  | 6.750 | 1.43  | 12.833 | 5.89  | 18.92 | 1.43  |











V V I SSSS U U A L (v 6.2.2008)  
V V I SS U U A A L  
V V I SS U U A A L  
V V I SS U U A A L  
V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M O O TM  
O O T T H H Y Y M M O O  
O O T T H H Y Y M M O O  
000 T T H H Y M M O O

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voindat

Output filename:  
C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\2d543248-4dd9-46be-b608-32ce2c7c14d5\scena

Summary filename:  
C:\Users\kswain\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\2d543248-4dd9-46be-b608-32ce2c7c14d5\scena

DATE: 08-23-2022 TIME: 11:47:07

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\* SIMULATION : J. 25yr 24hr 15min SCS Type I \*\*  
\*\*\*\*\*

READ STORM File: C:\Users\kswain\AppData\Local\Temp\

Ptotal=108.80 mm Comments: J. 25yr 24hr 15min SCS Type II

| TIME | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME  | RAIN  |
|------|-------|-------|--------|-------|-------|-------|-------|
| hrs  | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs   | mm/hr |
| 0.00 | 0.00  | 6.25  | 1.96   | 12.50 | 15.67 | 18.75 | 1.96  |
| 0.25 | 1.20  | 6.50  | 1.96   | 12.75 | 8.05  | 19.00 | 1.96  |
| 0.50 | 1.20  | 6.75  | 1.96   | 13.00 | 8.05  | 19.25 | 1.96  |
| 0.75 | 1.20  | 7.00  | 1.96   | 13.25 | 5.88  | 19.50 | 1.96  |
| 1.00 | 1.20  | 7.25  | 2.39   | 13.50 | 5.88  | 19.75 | 1.96  |
| 1.25 | 1.20  | 7.50  | 2.39   | 13.75 | 4.57  | 20.00 | 1.96  |
| 1.50 | 1.20  | 7.75  | 2.39   | 14.00 | 4.57  | 20.25 | 1.31  |
| 1.75 | 1.20  | 8.00  | 2.39   | 14.25 | 3.26  | 20.50 | 1.31  |
| 2.00 | 1.20  | 8.25  | 2.83   | 14.50 | 3.26  | 20.75 | 1.31  |
| 2.25 | 1.41  | 8.50  | 2.83   | 14.75 | 3.26  | 21.00 | 1.31  |
| 2.50 | 1.41  | 8.75  | 3.05   | 15.00 | 3.26  | 21.25 | 1.31  |
| 2.75 | 1.41  | 9.00  | 3.05   | 15.25 | 3.26  | 21.50 | 1.31  |
| 3.00 | 1.41  | 9.25  | 3.48   | 15.50 | 3.26  | 21.75 | 1.31  |
| 3.25 | 1.41  | 9.50  | 3.48   | 15.75 | 3.26  | 22.00 | 1.31  |
| 3.50 | 1.41  | 9.75  | 3.92   | 16.00 | 3.26  | 22.25 | 1.31  |
| 3.75 | 1.41  | 10.00 | 3.92   | 16.25 | 1.96  | 22.50 | 1.31  |
| 4.00 | 1.41  | 10.25 | 5.00   | 16.50 | 1.96  | 22.75 | 1.31  |
| 4.25 | 1.74  | 10.50 | 5.00   | 16.75 | 1.96  | 23.00 | 1.31  |
| 4.50 | 1.74  | 10.75 | 6.75   | 17.00 | 1.96  | 23.25 | 1.31  |
| 4.75 | 1.74  | 11.00 | 6.75   | 17.25 | 1.96  | 23.50 | 1.31  |
| 5.00 | 1.74  | 11.25 | 10.44  | 17.50 | 1.96  | 23.75 | 1.31  |
| 5.25 | 1.74  | 11.50 | 10.44  | 17.75 | 1.96  | 24.00 | 1.31  |
| 5.50 | 1.74  | 11.75 | 32.20  | 18.00 | 1.96  |       |       |
| 5.75 | 1.74  | 12.00 | 133.17 | 18.25 | 1.96  |       |       |
| 6.00 | 1.74  | 12.25 | 15.67  | 18.50 | 1.96  |       |       |

CALIB NASHYD ( 0001) Area (ha)= 4.32 Curve Number (CN)= 74.0  
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00  
U.H. Tp(hrs)= 0.36

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

| TIME  | RAIN  | TIME  | RAIN  | TIME   | RAIN   | TIME  | RAIN  |
|-------|-------|-------|-------|--------|--------|-------|-------|
| hrs   | mm/hr | hrs   | mm/hr | hrs    | mm/hr  | hrs   | mm/hr |
| 0.083 | 0.00  | 6.167 | 1.74  | 12.250 | 133.17 | 18.33 | 1.96  |
| 0.167 | 0.00  | 6.250 | 1.74  | 12.333 | 15.68  | 18.42 | 1.96  |
| 0.250 | 0.00  | 6.333 | 1.96  | 12.417 | 15.67  | 18.50 | 1.96  |
| 0.333 | 1.20  | 6.417 | 1.96  | 12.500 | 15.67  | 18.58 | 1.96  |
| 0.417 | 1.20  | 6.500 | 1.96  | 12.583 | 15.67  | 18.67 | 1.96  |

|       |      |        |      |        |       |       |      |
|-------|------|--------|------|--------|-------|-------|------|
| 0.500 | 1.20 | 6.583  | 1.96 | 12.667 | 15.67 | 18.75 | 1.96 |
| 0.583 | 1.20 | 6.667  | 1.96 | 12.750 | 15.67 | 18.83 | 1.96 |
| 0.667 | 1.20 | 6.750  | 1.96 | 12.833 | 8.05  | 18.92 | 1.96 |
| 0.750 | 1.20 | 6.833  | 1.96 | 12.917 | 8.05  | 19.00 | 1.96 |
| 0.833 | 1.20 | 6.917  | 1.96 | 13.000 | 8.05  | 19.08 | 1.96 |
| 0.917 | 1.20 | 7.000  | 1.96 | 13.083 | 8.05  | 19.17 | 1.96 |
| 1.000 | 1.20 | 7.083  | 1.96 | 13.167 | 8.05  | 19.25 | 1.96 |
| 1.083 | 1.20 | 7.167  | 1.96 | 13.250 | 8.05  | 19.33 | 1.96 |
| 1.167 | 1.20 | 7.250  | 1.96 | 13.333 | 5.88  | 19.42 | 1.96 |
| 1.250 | 1.20 | 7.333  | 2.39 | 13.417 | 5.88  | 19.50 | 1.96 |
| 1.333 | 1.20 | 7.417  | 2.39 | 13.500 | 5.88  | 19.58 | 1.96 |
| 1.417 | 1.20 | 7.500  | 2.39 | 13.583 | 5.88  | 19.67 | 1.96 |
| 1.500 | 1.20 | 7.583  | 2.39 | 13.667 | 5.88  | 19.75 | 1.96 |
| 1.583 | 1.20 | 7.667  | 2.39 | 13.750 | 5.88  | 19.83 | 1.96 |
| 1.667 | 1.20 | 7.750  | 2.39 | 13.833 | 4.57  | 19.92 | 1.96 |
| 1.750 | 1.20 | 7.833  | 2.39 | 13.917 | 4.57  | 20.00 | 1.96 |
| 1.833 | 1.20 | 7.917  | 2.39 | 14.000 | 4.57  | 20.08 | 1.96 |
| 1.917 | 1.20 | 8.000  | 2.39 | 14.083 | 4.57  | 20.17 | 1.96 |
| 2.000 | 1.20 | 8.083  | 2.39 | 14.167 | 4.57  | 20.25 | 1.96 |
| 2.083 | 1.20 | 8.167  | 2.39 | 14.250 | 4.57  | 20.33 | 1.31 |
| 2.167 | 1.20 | 8.250  | 2.39 | 14.333 | 3.26  | 20.42 | 1.31 |
| 2.250 | 1.20 | 8.333  | 2.83 | 14.417 | 3.26  | 20.50 | 1.31 |
| 2.333 | 1.41 | 8.417  | 2.83 | 14.500 | 3.26  | 20.58 | 1.31 |
| 2.417 | 1.41 | 8.500  | 2.83 | 14.583 | 3.26  | 20.67 | 1.31 |
| 2.500 | 1.41 | 8.583  | 2.83 | 14.667 | 3.26  | 20.75 | 1.31 |
| 2.583 | 1.41 | 8.667  | 2.83 | 14.750 | 3.26  | 20.83 | 1.31 |
| 2.667 | 1.41 | 8.750  | 2.83 | 14.833 | 3.26  | 20.92 | 1.31 |
| 2.750 | 1.41 | 8.833  | 3.05 | 14.917 | 3.26  | 21.00 | 1.31 |
| 2.833 | 1.41 | 8.917  | 3.05 | 15.000 | 3.26  | 21.08 | 1.31 |
| 2.917 | 1.41 | 9.000  | 3.05 | 15.083 | 3.26  | 21.17 | 1.31 |
| 3.000 | 1.41 | 9.083  | 3.05 | 15.167 | 3.26  | 21.25 | 1.31 |
| 3.083 | 1.41 | 9.167  | 3.05 | 15.250 | 3.26  | 21.33 | 1.31 |
| 3.167 | 1.41 | 9.250  | 3.05 | 15.333 | 3.26  | 21.42 | 1.31 |
| 3.250 | 1.41 | 9.333  | 3.48 | 15.417 | 3.26  | 21.50 | 1.31 |
| 3.333 | 1.41 | 9.417  | 3.48 | 15.500 | 3.26  | 21.58 | 1.31 |
| 3.417 | 1.41 | 9.500  | 3.48 | 15.583 | 3.26  | 21.67 | 1.31 |
| 3.500 | 1.41 | 9.583  | 3.48 | 15.667 | 3.26  | 21.75 | 1.31 |
| 3.583 | 1.41 | 9.667  | 3.48 | 15.750 | 3.26  | 21.83 | 1.31 |
| 3.667 | 1.41 | 9.750  | 3.48 | 15.833 | 3.26  | 21.92 | 1.31 |
| 3.750 | 1.41 | 9.833  | 3.92 | 15.917 | 3.26  | 22.00 | 1.31 |
| 3.833 | 1.41 | 9.917  | 3.92 | 16.000 | 3.26  | 22.08 | 1.31 |
| 3.917 | 1.41 | 10.000 | 3.92 | 16.083 | 3.26  | 22.17 | 1.31 |
| 4.000 | 1.41 | 10.083 | 3.92 | 16.167 | 3.26  | 22.25 | 1.31 |
| 4.083 | 1.41 | 10.167 | 3.92 | 16.250 | 3.26  | 22.33 | 1.31 |
| 4.167 | 1.41 | 10.250 | 3.92 | 16.333 | 1.96  | 22.42 | 1.31 |
| 4.250 | 1.41 | 10.333 | 5.00 | 16.417 | 1.96  | 22.50 | 1.31 |
| 4.333 | 1.74 | 10.417 | 5.00 | 16.500 | 1.96  | 22.58 | 1.31 |
| 4.417 | 1.74 | 10.500 | 5.00 | 16.583 | 1.96  | 22.67 | 1.31 |
| 4.500 | 1.74 | 10.583 | 5.00 | 16.667 | 1.96  | 22.75 | 1.31 |
| 4.583 | 1.74 | 10.667 | 5.00 | 16.750 | 1.96  | 22.83 | 1.31 |

Unit Hyd Qpeak (cms)= 0.458

PEAK FLOW (cms)= 0.404 (i)

TIME TO PEAK (hrs)= 12.500

RUNOFF VOLUME (mm)= 54.235

TOTAL RAINFALL (mm)= 108.800

RUNOFF COEFFICIENT = 0.498

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0002) Area (ha)= 13.33 Curve Number (CN)= 74.0  
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00  
U.H. Tp(hrs)= 0.62

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

| TIME  | RAIN  | TIME  | RAIN  | TIME   | RAIN   | TIME  | RAIN  |
|-------|-------|-------|-------|--------|--------|-------|-------|
| hrs   | mm/hr | hrs   | mm/hr | hrs    | mm/hr  | hrs   | mm/hr |
| 0.083 | 0.00  | 6.167 | 1.74  | 12.250 | 133.17 | 18.33 | 1.96  |
| 0.167 | 0.00  | 6.250 | 1.74  | 12.333 | 15.68  | 18.42 | 1.96  |
| 0.250 | 0.00  | 6.333 | 1.96  | 12.417 | 15.67  | 18.50 | 1.96  |
| 0.333 | 1.20  | 6.417 | 1.96  | 12.500 | 15.67  | 18.58 | 1.96  |
| 0.417 | 1.20  | 6.500 | 1.96  | 12.583 | 15.67  | 18.67 | 1.96  |
| 0.500 | 1.20  | 6.583 | 1.96  | 12.667 | 15.67  | 18.75 | 1.96  |
| 0.583 | 1.20  | 6.667 | 1.96  | 12.750 | 15.67  | 18.83 | 1.96  |
| 0.667 | 1.20  | 6.750 | 1.96  | 12.833 | 8.05   | 18.92 | 1.96  |







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V V I SSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
W I SSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vojn.dat

Output filename:
C:\Users\kswin\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\38446
1d6-a562-4a70-a749-780bc480b325\scena

Summary filename:
C:\Users\kswin\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\38446
1d6-a562-4a70-a749-780bc480b325\scena

DATE: 08-23-2022 TIME: 11:47:07

USER:

COMMENTS: \_\_\_\_\_

\*\* SIMULATION : K. 50yr 24hr 15min SCS Type I \*\*

READ STORM Filename: C:\Users\kswin\AppData\Local\Temp\

Ptotal=120.77 mm Comments: K. 50yr 24hr 15min SCS Type II

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals and corresponding rainfall amounts in mm/hr.

CALIB NASHYD ( 0001) Area (ha)= 4.32 Curve Number (CN)= 74.0
Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.36

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data.

Large data table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show detailed simulation results.

Large data table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show detailed simulation results.

Unit Hyd Qpeak (cms)= 0.458
PEAK FLOW (cms)= 0.476 (i)
TIME TO PEAK (hrs)= 12.500
RUNOFF VOLUME (mm)= 63.745
TOTAL RAINFALL (mm)= 120.770
RUNOFF COEFFICIENT = 0.528

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0002) Area (ha)= 13.33 Curve Number (CN)= 74.0
Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.62

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data.





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V V I SSSS U U A L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\voim.dat

Output filename:
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437-9abb-43fb-8232-acddf4b89f00\scena

Summary filename:
C:\Users\kswin\AppData\Local\Civica\H5\5e7e8f9f-3516-4350-9cc9-216f91ea9b42\fe2af
437-9abb-43fb-8232-acddf4b89f00\scena

DATE: 08-23-2022 TIME: 11:47:07

USER:

COMMENTS: \_\_\_\_\_

\*\* SIMULATION : 1. 100yr 24hr 15min SCS Type \*\*

READ STORM Filename: C:\Users\kswin\AppData\Local\Temp\

Ptotal=132.74 mm Comments: 1. 100yr 24hr 15min SCS Type II

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals and corresponding rainfall amounts.

CALIB NASHYD ( 0001) Area (ha)= 4.32 Curve Number (CN)= 74.0
Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.36

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show detailed simulation data.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show detailed simulation data.

Unit Hyd Qpeak (cms)= 0.458
PEAK FLOW (cms)= 0.551 (i)
TIME TO PEAK (hrs)= 12.500
RUNOFF VOLUME (mm)= 73.530
TOTAL RAINFALL (mm)= 132.740
RUNOFF COEFFICIENT = 0.554

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0002) Area (ha)= 13.33 Curve Number (CN)= 74.0
Ia (mm)= 7.00 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.62

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed hyetograph data.

Table with 8 columns: 1. Rainfall Depth (mm) (e.g., 0.750, 0.833, 0.917, 1.000, 1.083, 1.167, 1.250, 1.333, 1.417, 1.500, 1.583, 1.667, 1.750, 1.833, 1.917, 2.000, 2.083, 2.167, 2.250, 2.333, 2.417, 2.500, 2.583, 2.667, 2.750, 2.833, 2.917, 3.000, 3.083, 3.167, 3.250, 3.333, 3.417, 3.500, 3.583, 3.667, 3.750, 3.833, 3.917, 4.000, 4.083, 4.167, 4.250, 4.333, 4.417, 4.500, 4.583, 4.667, 4.750, 4.833). 2. S1 (mm). 3. S2 (mm). 4. S3 (mm). 5. S4 (mm). 6. S5 (mm). 7. S6 (mm). 8. S7 (mm).

Table with 8 columns: 1. Rainfall Depth (mm) (e.g., 4.917, 5.000, 5.083, 5.167, 5.250, 5.333, 5.417, 5.500, 5.583, 5.667, 5.750, 5.833, 5.917, 6.000, 6.083). 2. S1 (mm). 3. S2 (mm). 4. S3 (mm). 5. S4 (mm). 6. S5 (mm). 7. S6 (mm). 8. S7 (mm).

Unit Hyd Qpeak (cms)= 0.821

PEAK FLOW (cms)= 1.146 (i)  
TIME TO PEAK (hrs)= 12.750  
RUNOFF VOLUME (mm)= 73.542  
TOTAL RAINFALL (mm)= 132.740  
RUNOFF COEFFICIENT = 0.554

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0003 ) Area (ha)= 3.05 Curve Number (CN)= 74.0  
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00  
U.H. Tp(hrs)= 0.22

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 10 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. It shows transformed hyetograph data for various time intervals and rainfall amounts.

Table with 8 columns: 1. Rainfall Depth (mm) (e.g., 1.000, 1.083, 1.167, 1.250, 1.333, 1.417, 1.500, 1.583, 1.667, 1.750, 1.833, 1.917, 2.000, 2.083, 2.167, 2.250, 2.333, 2.417, 2.500, 2.583, 2.667, 2.750, 2.833, 2.917, 3.000, 3.083, 3.167, 3.250, 3.333, 3.417, 3.500, 3.583, 3.667, 3.750, 3.833, 3.917, 4.000, 4.083, 4.167, 4.250, 4.333, 4.417, 4.500, 4.583, 4.667, 4.750, 4.833, 4.917, 5.000, 5.083). 2. S1 (mm). 3. S2 (mm). 4. S3 (mm). 5. S4 (mm). 6. S5 (mm). 7. S6 (mm). 8. S7 (mm).

Table with 8 columns: 1. Rainfall Depth (mm) (e.g., 5.167, 5.250, 5.333, 5.417, 5.500, 5.583, 5.667, 5.750, 5.833, 5.917, 6.000, 6.083). 2. S1 (mm). 3. S2 (mm). 4. S3 (mm). 5. S4 (mm). 6. S5 (mm). 7. S6 (mm). 8. S7 (mm).

Unit Hyd Qpeak (cms)= 0.530

PEAK FLOW (cms)= 0.554 (i)  
TIME TO PEAK (hrs)= 12.333  
RUNOFF VOLUME (mm)= 73.447  
TOTAL RAINFALL (mm)= 132.740  
RUNOFF COEFFICIENT = 0.553

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0004 ) Area (ha)= 3.00 Curve Number (CN)= 74.0  
ID= 1 DT= 5.0 min Ia (mm)= 7.00 # of Linear Res.(N)= 3.00  
U.H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 10 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. It shows transformed hyetograph data for various time intervals and rainfall amounts.

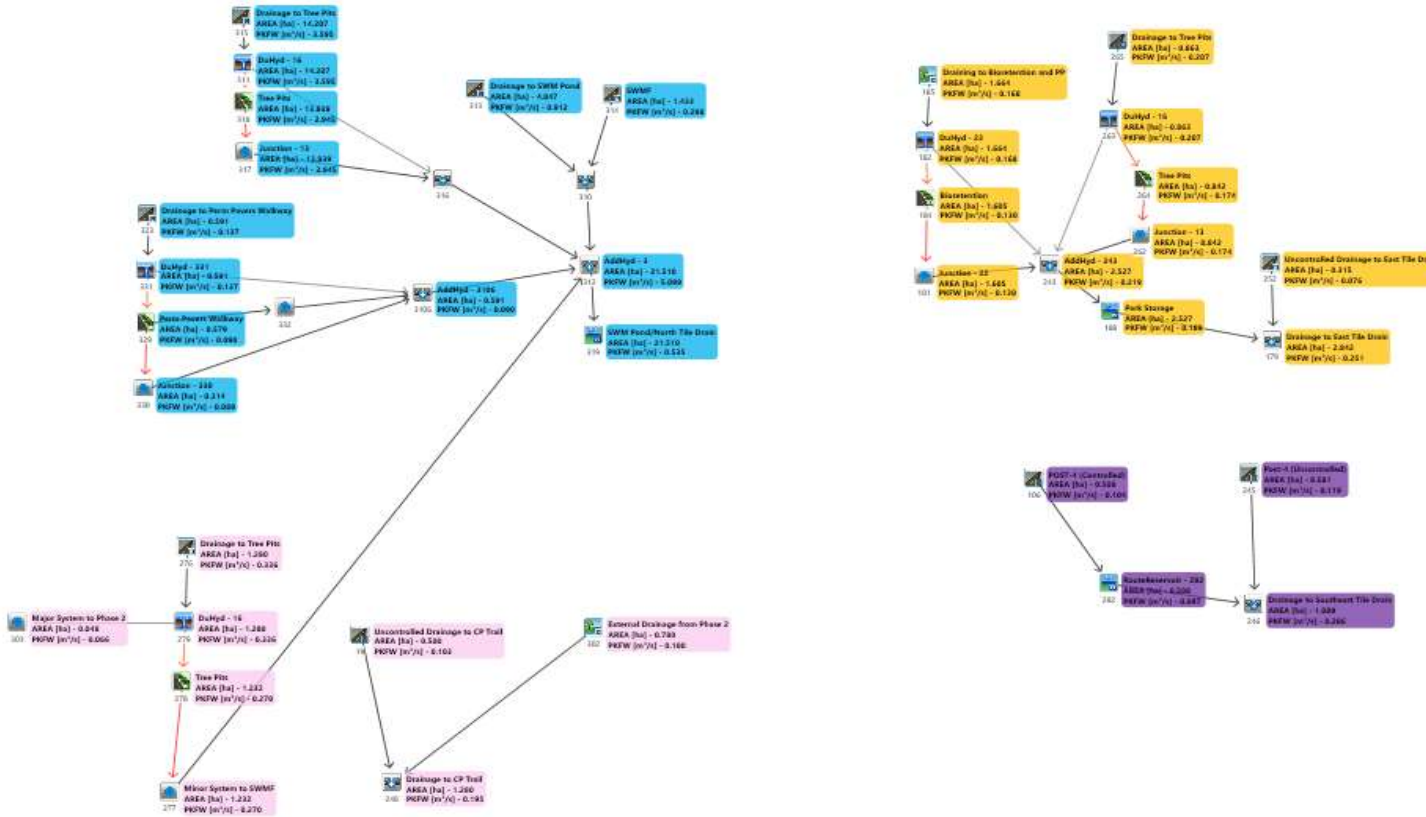




Project: Glenelg Expansion Lands  
Project No.: 1060-6220  
File: Model Schematic (Post-Development)  
Design by: K. Swain  
Date: 2022-08-23

## Visual OTTHYMO 6.0 Model Schematic

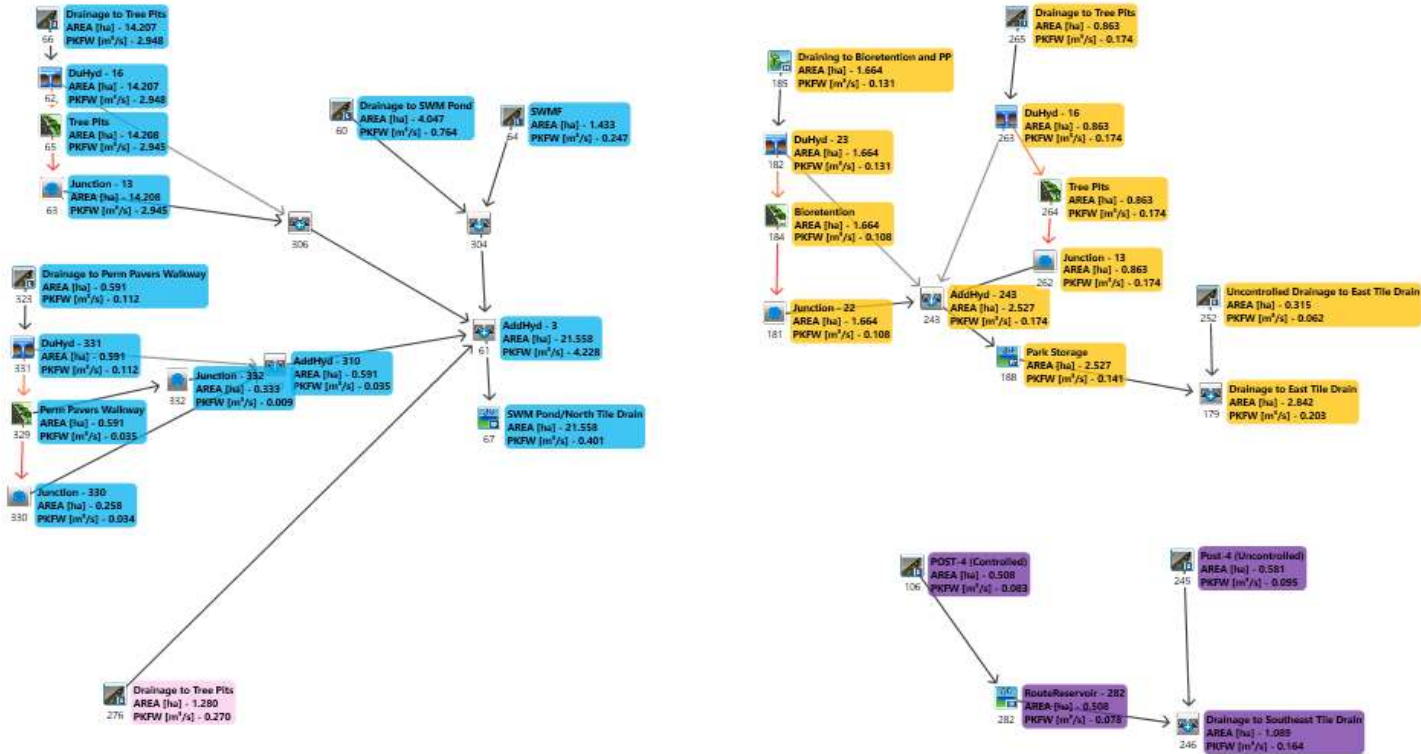
### Post-Development (Major System)





## Visual OTTHYMO 6.0 Model Schematic

### Post-Development (Minor System)



```

***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\VisualOTHMYO 6.2\VO2\vojn.dat
Output filename:
C:\Users\ksavin\AppData\Local\Civil\MS4\9a8a70-2b3a-4142-a551-3404f768702\3aac
6e-404d-42a2-bca9-3af8f377ffda\scena
Summary filename:
C:\Users\ksavin\AppData\Local\Civil\MS4\9a8a70-2b3a-4142-a551-3404f768702\3aac
6e-404d-42a2-bca9-3af8f377ffda\scena

DATE: 08-28-2022 TIME: 02:36:03
USER:

COMMENTS:

```

```

83976398-3436-4fc2-b953-6488413a757f6945257d
Plotal= 24.99 mm Comments: 25mm

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.00 1.36 1.00 6.91 2.00 4.18 3.00 3.86
0.08 1.45 1.08 11.02 2.08 3.77 3.08 3.78
0.17 1.55 1.17 26.16 2.17 3.43 3.17 3.71
0.25 1.67 1.25 76.07 2.25 3.16 3.25 3.64
0.33 1.81 1.33 33.71 2.33 2.92 3.33 3.58
0.42 1.99 1.42 18.64 2.42 2.72 3.42 3.52
0.50 2.20 1.50 12.61 2.50 2.55 3.50 3.47
0.58 2.47 1.58 9.46 2.58 2.39 3.58 3.43
0.67 2.82 1.67 7.55 2.67 2.26 3.67 3.38
0.75 3.29 1.75 6.28 2.75 2.14 3.75 3.34
0.83 3.97 1.83 5.38 2.83 2.04 3.83 3.30
0.92 5.03 1.92 4.70 2.92 1.94 3.92 3.26

CALIB
NASHVD ( 0185) Area (ha)= 1.66 Curve Number (CN)= 74.4
ID= 1 DT= 5.0 min Ia (mm)= 4.44 # of Linear Res.(N)= 3.00
Ua1.Tp(hrs)= 0.24

Unit Hyd Qpeak (cms)= 0.265
PEAK FLOW (cms)= 0.016 (1)
TIME TO PEAK (hrs)= 1.667
RUNOFF VOLUME (mm)= 3.908
TOTAL RAINFALL (mm)= 24.991
RUNOFF COEFFICIENT = 0.156

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

DUHYD ( 0182)
Inlet Cap.= 0.131
# of Inlets= 1
Total(cms)= 0.11 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
TOTAL HYD.(ID= 1): 1.66 0.02 1.67 3.91
MAJOR SYS.(ID= 2): 0.00 0.00 0.00 0.00
MINOR SYS.(ID= 3): 1.66 0.02 1.67 3.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

***** DETAILED OUTPUT *****

OUTFLOW: OFF, UNDERDRAIN: OFF
| RESERVOIR ( 0184) |
| DT= 5.0 min |
SURFACE PONDING LAYER:
Max. Ponding Storage(cu.m.)= 149.95

STAGE AREA STAGE AREA
(m) (m2) (m) (m2)
0.000 100.000 1.500 100.000
0.500 100.000 0.000 0.000
1.000 100.000 0.000 0.000

MULCH LAYER:
Depth (m)= 0.10 Porosity = 0.40
Maximum Mulch Layer Volume (cu.m.)= 21.60

ENGINEERED SOIL LAYER:
Soil moisture = 0.30 Depth (m)= 0.72
Width (m)= 40.00 Length (m)= 3.00
Porosity = 0.47 Infiltration (m/hr)= 0.0060
Maximum Engineered Soil Layer Volume(cu.m.)= 181.57

STORAGE LAYER:
Depth (m)= 0.50 Porosity = 0.40
Seepage (m/hr)= 0.0010
Maximum Storage Layer Volume(cu.m.)= 108.00

TOTAL AVAILABLE STORAGE:
TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil
+Storage Layer(cu.m.)= 461.13

NATIVE SOIL LAYER:
Infiltration (m/hr)= 0.0060

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW:ID= 2 1.66 0.016 1.67 3.91
OVERFLOW:ID= 3 0.00 0.000 0.00 0.00

Volume Reduction Rate(RVIN=RVOUT)/RVIN (%)= 100.00
Time to reach Max Ponding storage (Hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 116.76
Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 145.47
Maximum Engineered Soil Volume (cu.m.)= 9.21
**** After simulation, water volume is not zero.

```

```

***** DETAILED OUTPUT *****

JUNCTION COMMAND(0181)
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 3 ( 0184) 0.00 0.00 0.00 0.00
OUTFLOW: ID= 2 ( 0181) 0.00 0.00 0.00 0.00

CALIB
STANDHYD ( 0205) Area (ha)= 0.86
ID= 1 DT= 5.0 min Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00

IMPERVIOUS PERVIOUS (1)
Surface Area (ha)= 0.56 0.30
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 75.87 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 76.07 4.33
over (min) 5.00 30.00
Storage Coeff. (min)= 2.41 (11) 27.19 (11)
Unit Hyd. Tpeak (min)= 5.00 30.00
Unit Hyd. peak (cms)= 0.30 0.04

PEAK FLOW (cms)= 0.11 0.00 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.92 1.33
RUNOFF VOLUME (mm)= 23.99 3.66 16.85
TOTAL RAINFALL (mm)= 24.99 24.99 24.99
RUNOFF COEFFICIENT = 0.96 0.15 0.67

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

DUHYD ( 0263)
Inlet Cap.= 0.174
# of Inlets= 1
Total(cms)= 0.21 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW:ID= 2 ( 0243) 0.000 0.000 0.00 0.00
OUTFLOW:ID= 1 ( 0188) 0.000 0.000 0.00 NaN

PEAK FLOW REDUCTION (Qout/Qin)(%)= NaN
TIME SHIFT OF PEAK FLOW (min)= 0.00
MAXIMUM STORAGE USED (cu.m.)= 0.00000
MAXIMUM STORAGE USED (cu.m.)= 0.000000

**** WARNING : SELECTED ROUTING TIME STEP DENIED.
**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

CALIB
STANDHYD ( 0252) Area (ha)= 0.32
ID= 1 DT= 5.0 min Total Imp(%)= 69.00 Dir. Conn.(%)= 30.00

IMPERVIOUS PERVIOUS (1)
Surface Area (ha)= 0.22 0.10
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 45.84 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 76.07 32.49
over (min) 5.00 15.00
Storage Coeff. (min)= 1.78 (11) 12.85 (11)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.08

PEAK FLOW (cms)= 0.02 0.01 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.58 1.33
RUNOFF VOLUME (mm)= 23.99 8.33 13.00
TOTAL RAINFALL (mm)= 24.99 24.99 24.99
RUNOFF COEFFICIENT = 0.96 0.33 0.52

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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***** DETAILED OUTPUT *****

TOTAL HYD.(ID= 1): 0.86 0.11 1.33 16.85
MAJOR SYS.(ID= 2): 0.00 0.00 0.00 0.00
MINOR SYS.(ID= 3): 0.86 0.11 1.33 16.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| RESERVOIR ( 0264) |
| DT= 5.0 min |
SURFACE PONDING LAYER:
Max. Ponding Storage(cu.m.)= 149.95

STAGE AREA STAGE AREA
(m) (m2) (m) (m2)
0.000 100.000 1.500 100.000
0.500 100.000 0.000 0.000
1.000 100.000 0.000 0.000

MULCH LAYER:
Depth (m)= 0.10 Porosity = 0.40
Maximum Mulch Layer Volume (cu.m.)= 3.20

ENGINEERED SOIL LAYER:
Soil moisture = 0.30 Depth (m)= 0.67
Width (m)= 40.00 Length (m)= 2.00
Porosity = 0.47 Infiltration (m/hr)= 0.0060
Maximum Engineered Soil Layer Volume(cu.m.)= 25.03

STORAGE LAYER:
Depth (m)= 1.50 Porosity = 0.40
Seepage (m/hr)= 0.0010
Maximum Storage Layer Volume(cu.m.)= 48.00

TOTAL AVAILABLE STORAGE:
TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil
+Storage Layer(cu.m.)= 226.18

NATIVE SOIL LAYER:
Infiltration (m/hr)= 0.0060

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW:ID= 2 0.86 0.108 1.33 16.85
OVERFLOW:ID= 3 0.00 0.000 0.00 0.00

| JUNCTION COMMAND(0262) |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 3 ( 0264) 0.00 0.00 0.00 0.00
OUTFLOW: ID= 2 ( 0262) 0.00 0.00 0.00 0.00

| ADD HYD ( 0243) |
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
*** M A R N I N G : HYDROGRAPH 0182 ID= 3 IS DRY.
*** M A R N I N G : HYDROGRAPH 0182 ID= 2 IS DRY.
*** M A R N I N G : HYDROGRAPH 0243 ID= 3 IS ALSO DRY

| ADD HYD ( 0243) |
3 + 2 = 1 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
*** M A R N I N G : HYDROGRAPH 0243 ID= 3 IS DRY.
*** M A R N I N G : HYDROGRAPH 0262 ID= 2 IS DRY.
*** M A R N I N G : HYDROGRAPH 0243 ID= 1 IS ALSO DRY

| ADD HYD ( 0243) |
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
*** M A R N I N G : HYDROGRAPH 0243 ID= 3 IS DRY.
*** M A R N I N G : HYDROGRAPH 0263 ID= 3 IS DRY.
*** M A R N I N G : HYDROGRAPH 0243 ID= 3 IS ALSO DRY

| RESERVOIR ( 0181) |
| DT= 5.0 min |
SURFACE PONDING LAYER:
Max. Ponding Storage (cu.m.)= 149.95
OUTFLOW STORAGE (cu.m.)= 0.0000
OUTFLOW STORAGE (cu.m.)= 0.1000

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***** DETAILED OUTPUT *****

JUNCTION COMMAND(0262)
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 3 ( 0264) 0.00 0.00 0.00 0.00
OUTFLOW: ID= 2 ( 0262) 0.00 0.00 0.00 0.00

| ADD HYD ( 0243) |
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
*** M A R N I N G : HYDROGRAPH 0182 ID= 3 IS DRY.
*** M A R N I N G : HYDROGRAPH 0182 ID= 2 IS DRY.
*** M A R N I N G : HYDROGRAPH 0243 ID= 3 IS ALSO DRY

| ADD HYD ( 0243) |
3 + 2 = 1 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
*** M A R N I N G : HYDROGRAPH 0243 ID= 3 IS DRY.
*** M A R N I N G : HYDROGRAPH 0262 ID= 2 IS DRY.
*** M A R N I N G : HYDROGRAPH 0243 ID= 1 IS ALSO DRY

| ADD HYD ( 0243) |
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
*** M A R N I N G : HYDROGRAPH 0243 ID= 3 IS DRY.
*** M A R N I N G : HYDROGRAPH 0263 ID= 3 IS DRY.
*** M A R N I N G : HYDROGRAPH 0243 ID= 3 IS ALSO DRY

| RESERVOIR ( 0181) |
| DT= 5.0 min |
SURFACE PONDING LAYER:
Max. Ponding Storage (cu.m.)= 149.95
OUTFLOW STORAGE (cu.m.)= 0.0000
OUTFLOW STORAGE (cu.m.)= 0.1000

```

```

***** DETAILED OUTPUT *****

IMPERVIOUS PERVIOUS (1)
Surface Area (ha)= 0.0000 0.0000 0.4800 0.0190
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 45.84 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 76.07 32.49
over (min) 5.00 15.00
Storage Coeff. (min)= 1.78 (11) 12.85 (11)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.32 0.08

PEAK FLOW (cms)= 0.02 0.01 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.58 1.33
RUNOFF VOLUME (mm)= 23.99 8.33 13.00
TOTAL RAINFALL (mm)= 24.99 24.99 24.99
RUNOFF COEFFICIENT = 0.96 0.33 0.52

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| RESERVOIR ( 0202) |
OVERFLOW IS OFF

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***** DETAILED OUTPUT *****

| ADD HYD ( 0179) |
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
*** M A R N I N G : HYDROGRAPH 0188 ID= 1 IS DRY.
*** M A R N I N G : HYDROGRAPH 0179 = HYDROGRAPH 0252
ID= 1 ( 0188): 0.00 0.000 0.00 NaN
+ ID= 2 ( 0252): 0.32 0.021 1.33 13.00
ID= 3 ( 0179): 0.32 0.000 0.00 NaN

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD ( 0186) Area (ha)= 0.51
ID= 1 DT= 5.0 min Total Imp(%)= 55.00 Dir. Conn.(%)= 1.00

IMPERVIOUS PERVIOUS (1)
Surface Area (ha)= 0.28 0.23
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 58.22 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 76.07 30.92
over (min) 5.00 15.00
Storage Coeff. (min)= 2.06 (11) 13.35 (11)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.31 0.08

PEAK FLOW (cms)= 0.00 0.01 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.58 1.58
RUNOFF VOLUME (mm)= 23.99 8.16 8.30
TOTAL RAINFALL (mm)= 24.99 24.99 24.99
RUNOFF COEFFICIENT = 0.96 0.33 0.33

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
**** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| RESERVOIR ( 0202) |
OVERFLOW IS OFF

```

```

***** DETAILED OUTPUT *****

| IN= 2--- OUT= 1 |
| DT= 5.0 min |
SURFACE PONDING LAYER:
Max. Ponding Storage (cu.m.)= 149.95
OUTFLOW STORAGE (cu.m.)= 0.0000
OUTFLOW STORAGE (cu.m.)= 0.1000

| CALIB |
| STANDHYD ( 0245) |
| DT= 5.0 min |
Total Imp(%)= 55.00 Dir. Conn.(%)= 1.00

IMPERVIOUS PERVIOUS (1)
Surface Area (ha)= 0.32 0.26
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 62.24 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 76.07 30.92
over (min) 5.00 15.00
Storage Coeff. (min)= 2.14 (11) 13.43 (11)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.31 0.08

PEAK FLOW (cms)= 0.00 0.01 *TOTALS*
TIME TO PEAK (hrs)= 1.33 1.58 1.58
RUNOFF VOLUME (mm)= 23.99 8.16 8.30
TOTAL RAINFALL (mm)= 24.99 24.99 24.99
RUNOFF COEFFICIENT = 0.96 0.33 0.33

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
**** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----  
 ADD HYD ( 0246) | AREA QPEAK TPEAK R.V.  
 | 1 + 2 = 3 | (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0245): 0.59 0.014 1.58 8.30  
 + ID2= 2 ( 0242): 0.51 0.012 1.67 8.30  
 -----  
 ID = 3 ( 0246): 1.09 0.026 1.58 8.30  
 -----  
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 CALIB | STANDBY ( 0276) | Area (ha)= 1.20  
 | ID= 1 DT= 5.0 min | Total Imp(%)= 70.00 Dir. Conn.(%)= 70.00  
 -----  
 IMPERVIOUS PERVIOUS (1)  
 Surface Area (ha)= 0.90 0.38  
 Dep. Storage (mm)= 1.00 5.00  
 Average Slope (%)= 1.00 2.00  
 Length (m)= 92.38 40.00  
 Mannings n = 0.013 0.250  
 Max.Eff.Inten.(mm/hr)= 76.07 4.33  
 over (min) 5.00 30.00  
 Storage Coeff. (min)= 2.72 (1) 27.49 (11)  
 Unit Hyd. Tpeak (min)= 5.00 30.00  
 Unit Hyd. peak (cms)= 0.29 0.04  
 -----  
 PEAK FLOW (cms)= 0.37 0.00 \*TOTALS\*  
 TIME TO PEAK (hrs)= 1.33 1.92 1.33 (111)  
 RUNOFF VOLUME (mm)= 23.99 3.66 17.88  
 TOTAL RAINFALL (mm)= 24.99 24.99 24.99  
 RUNOFF COEFFICIENT = 0.96 0.15 0.72  
 -----  
 \*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 74.0 Ia = Dep. Storage (Above)  
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
 CALIB | STANDBY ( 0060) | Area (ha)= 4.05  
 | ID= 1 DT= 5.0 min | Total Imp(%)= 60.00 Dir. Conn.(%)= 60.00  
 -----  
 IMPERVIOUS PERVIOUS (1)  
 Surface Area (ha)= 3.09 0.96  
 Dep. Storage (mm)= 1.00 5.00  
 Average Slope (%)= 1.00 2.00  
 Length (m)= 307.76 40.00  
 Mannings n = 0.013 0.250  
 Max.Eff.Inten.(mm/hr)= 76.07 4.33  
 over (min) 5.00 30.00  
 Storage Coeff. (min)= 5.59 (1) 30.37 (11)  
 Unit Hyd. Tpeak (min)= 5.00 35.00  
 Unit Hyd. peak (cms)= 0.20 0.04  
 -----  
 PEAK FLOW (cms)= 1.48 0.02 1.488 (111)  
 TIME TO PEAK (hrs)= 2.10 1.33 1.33  
 RUNOFF VOLUME (mm)= 23.99 3.66 18.09  
 TOTAL RAINFALL (mm)= 24.99 24.99 24.99  
 RUNOFF COEFFICIENT = 0.96 0.15 0.72  
 -----  
 (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 74.0 Ia = Dep. Storage (Above)

-----  
 IMPERVIOUS PERVIOUS (1)  
 Surface Area (ha)= 2.43 1.62  
 Dep. Storage (mm)= 5.00 5.00  
 Average Slope (%)= 1.00 2.00  
 Length (m)= 164.25 40.00  
 Mannings n = 0.013 0.250  
 Max.Eff.Inten.(mm/hr)= 76.07 4.33  
 over (min) 5.00 30.00  
 Storage Coeff. (min)= 3.84 (1) 28.01 (11)  
 Unit Hyd. Tpeak (min)= 5.00 30.00  
 Unit Hyd. peak (cms)= 0.25 0.04  
 -----  
 PEAK FLOW (cms)= 0.39 0.01 \*TOTALS\*  
 TIME TO PEAK (hrs)= 1.33 1.92 1.33 (111)  
 RUNOFF VOLUME (mm)= 19.99 3.66 17.88  
 TOTAL RAINFALL (mm)= 24.99 24.99 24.99  
 RUNOFF COEFFICIENT = 0.80 0.15 0.54  
 -----  
 \*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 74.0 Ia = Dep. Storage (Above)  
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
 CALIB | STANDBY ( 0064) | Area (ha)= 1.43  
 | ID= 1 DT= 5.0 min | Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00  
 -----  
 IMPERVIOUS PERVIOUS (1)  
 Surface Area (ha)= 0.72 0.72  
 Dep. Storage (mm)= 2.00 5.00  
 Average Slope (%)= 0.50 2.00  
 Length (m)= 97.74 30.00  
 Mannings n = 0.013 0.250  
 Max.Eff.Inten.(mm/hr)= 76.07 4.84  
 over (min) 5.00 25.00  
 Storage Coeff. (min)= 4.66 (1) 33.45 (11)  
 Unit Hyd. Tpeak (min)= 5.00 25.00  
 Unit Hyd. peak (cms)= 0.26 0.05  
 -----  
 PEAK FLOW (cms)= 0.13 0.00 \*TOTALS\*  
 TIME TO PEAK (hrs)= 1.33 1.83 1.33  
 RUNOFF VOLUME (mm)= 22.99 3.66 13.32  
 TOTAL RAINFALL (mm)= 24.99 24.99 24.99  
 -----  
 (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 74.0 Ia = Dep. Storage (Above)

-----  
 CALIB | STANDBY ( 0064) | Area (ha)= 1.43  
 | ID= 1 DT= 5.0 min | Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00  
 -----  
 IMPERVIOUS PERVIOUS (1)  
 Surface Area (ha)= 0.72 0.72  
 Dep. Storage (mm)= 2.00 5.00  
 Average Slope (%)= 0.50 2.00  
 Length (m)= 97.74 30.00  
 Mannings n = 0.013 0.250  
 Max.Eff.Inten.(mm/hr)= 76.07 4.84  
 over (min) 5.00 25.00  
 Storage Coeff. (min)= 4.66 (1) 33.45 (11)  
 Unit Hyd. Tpeak (min)= 5.00 25.00  
 Unit Hyd. peak (cms)= 0.26 0.05  
 -----  
 PEAK FLOW (cms)= 0.13 0.00 \*TOTALS\*  
 TIME TO PEAK (hrs)= 1.33 1.83 1.33  
 RUNOFF VOLUME (mm)= 22.99 3.66 13.32  
 TOTAL RAINFALL (mm)= 24.99 24.99 24.99  
 -----  
 (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 74.0 Ia = Dep. Storage (Above)

-----  
 RUNOFF COEFFICIENT = 0.92 0.15 0.53  
 -----  
 \*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 74.0 Ia = Dep. Storage (Above)  
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
 ADD HYD ( 0304) | AREA QPEAK TPEAK R.V.  
 | 1 + 2 = 3 | (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0060): 4.05 0.392 1.33 13.45  
 + ID2= 2 ( 0064): 1.43 0.127 1.33 13.32  
 -----  
 ID = 3 ( 0304): 5.48 0.519 1.33 13.42  
 -----  
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 CALIB | STANDBY ( 0066) | Area (ha)= 14.21  
 | ID= 1 DT= 5.0 min | Total Imp(%)= 71.00 Dir. Conn.(%)= 71.00  
 -----  
 IMPERVIOUS PERVIOUS (1)  
 Surface Area (ha)= 10.09 4.12  
 Dep. Storage (mm)= 1.00 5.00  
 Average Slope (%)= 1.00 2.00  
 Length (m)= 307.76 40.00  
 Mannings n = 0.013 0.250  
 Max.Eff.Inten.(mm/hr)= 76.07 4.33  
 over (min) 5.00 35.00  
 Storage Coeff. (min)= 5.59 (1) 30.37 (11)  
 Unit Hyd. Tpeak (min)= 5.00 35.00  
 Unit Hyd. peak (cms)= 0.20 0.04  
 -----  
 PEAK FLOW (cms)= 1.48 0.02 1.488 (111)  
 TIME TO PEAK (hrs)= 2.10 1.33 1.33  
 RUNOFF VOLUME (mm)= 23.99 3.66 18.09  
 TOTAL RAINFALL (mm)= 24.99 24.99 24.99  
 RUNOFF COEFFICIENT = 0.96 0.15 0.72  
 -----  
 (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 74.0 Ia = Dep. Storage (Above)

-----  
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
 DUHYD ( 0062) | AREA QPEAK TPEAK R.V.  
 | Inlet Cap.= 2.948 | (ha) (cms) (hrs) (mm)  
 | # of Inlets= 1 | -----  
 | Total(cms)= 2.9 | (ha) (cms) (hrs) (mm)  
 -----  
 TOTAL HYD. (ID= 1): 14.21 1.48 1.33 18.09  
 -----  
 MAJOR SYS. (ID= 2): 0.00 0.00 0.00 0.00  
 MINOR SYS. (ID= 3): 14.21 1.48 1.33 18.09  
 -----  
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 [Retention( 0065) | OUTFLOW: OFF, UNDERDRAIN: OFF  
 | In= 2--> OUT= 3 |  
 | DT= 5.0 min |

-----  
 SURFACE PONDING LAYER:  
 Max. Ponding Storage(cu.m.)= 149.95

| STAGE (m) | AREA (m2) | STAGE (m) | AREA (m2) |
|-----------|-----------|-----------|-----------|
| 0.000     | 100.000   | 1.500     | 100.000   |
| 0.500     | 100.000   | 0.000     | 0.000     |
| 1.000     | 100.000   | 0.000     | 0.000     |

MULCH LAYER:  
 Depth (m)= 0.10 Porosity = 0.40  
 Maximum Mulch Layer Volume (cu.m.)= 62.40

ENGINEERED SOIL LAYER:  
 Soil moisture = 0.30 Depth (m)= 0.67  
 Width (m)= 780.00 Length (m)= 2.00  
 Porosity = 0.47 Infiltration (m/hr)= 0.0060  
 Maximum Engineered Soil Layer Volume(cu.m.)= 488.11

STORAGE LAYER:  
 Depth (m)= 1.50 Porosity = 0.40  
 Seepage (m/hr)= 0.0010  
 Maximum Storage Layer Volume(cu.m.)= 936.00

TOTAL AVAILABLE STORAGE:  
 TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil  
 + Storage Layer(cu.m.)= 680.04

-----  
 TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil  
 + Storage Layer(cu.m.)= 1636.46

MATIVE SOIL LAYER:  
 Infiltration (m/hr)= 0.0060

| AREA (ha)      | QPEAK (cms) | TPEAK (hrs) | R.V. (mm)  |
|----------------|-------------|-------------|------------|
| INFLOW:ID= 2   | 14.21       | 1.48        | 1.33 18.09 |
| OVERFLOW:ID= 3 | 14.21       | 1.477       | 1.33 16.29 |

Volume Reduction Rate(RvIn-RvOut)/RvIn (%)= 9.94  
 Time to reach Max Ponding Storage (Hr)= 0.00  
 Volume of water for drawdown in LID (cu.m.)= 391.94  
 Volume of Max. Water Storage (cu.m.)= 0.00  
 Maximum Surface Ponding And Mulch Vol(cu.m.)= 212.35  
 Maximum Engineered Soil Volume (cu.m.)= 191.16  
 \*\*\*\*\* After simulation, water volume is not zero.

-----  
 [ Junction Command(0063) |

| AREA (ha)             | QPEAK (cms) | TPEAK (hrs) | R.V. (mm)  |
|-----------------------|-------------|-------------|------------|
| INFLOW: ID= 3( 0065)  | 14.21       | 1.48        | 1.33 16.29 |
| OUTFLOW: ID= 2( 0063) | 14.21       | 1.48        | 1.33 16.29 |

-----  
 ADD HYD ( 0306) | AREA QPEAK TPEAK R.V.  
 | 1 + 2 = 3 | (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0062): 0.00 0.000 0.00 0.00  
 + ID2= 2 ( 0063): 14.21 1.477 1.33 16.29  
 -----  
 ID = 3 ( 0306): 14.21 1.477 1.33 16.29  
 -----  
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 CALIB | STANDBY ( 0323) | Area (ha)= 0.59  
 | ID= 1 DT= 5.0 min | Total Imp(%)= 66.00 Dir. Conn.(%)= 25.00  
 -----  
 IMPERVIOUS PERVIOUS (1)  
 Surface Area (ha)= 0.39 0.20

-----  
 Dep. Storage (mm)= 1.00 5.00  
 Average Slope (%)= 1.00 2.00  
 Length (m)= 62.78 40.00  
 Mannings n = 0.013 0.250  
 Max.Eff.Inten.(mm/hr)= 76.07 31.08  
 over (min) 5.00 15.00  
 Storage Coeff. (min)= 2.10 (1) 13.42 (11)  
 Unit Hyd. Tpeak (min)= 5.00 15.00  
 Unit Hyd. peak (cms)= 0.31 0.08  
 -----  
 PEAK FLOW (cms)= 0.03 0.01 0.032 (111)  
 TIME TO PEAK (hrs)= 1.33 1.58 1.33  
 RUNOFF VOLUME (mm)= 23.99 0.17 12.11  
 TOTAL RAINFALL (mm)= 24.99 24.99 24.99  
 RUNOFF COEFFICIENT = 0.96 0.33 0.48  
 -----  
 \*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 74.0 Ia = Dep. Storage (Above)  
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
 [Retention( 0329) | OUTFLOW: OFF UNDERDRAIN: ON  
 | In= 2--> OUT= 3 |  
 | DT= 5.0 min |

-----  
 SURFACE PONDING LAYER:  
 Max. Ponding Storage(cu.m.)= 149.95

| STAGE (m) | AREA (m2) | STAGE (m) | AREA (m2) |
|-----------|-----------|-----------|-----------|
| 0.000     | 100.000   | 1.500     | 100.000   |
| 0.500     | 100.000   | 0.000     | 0.000     |
| 1.000     | 100.000   | 0.000     | 0.000     |

MULCH LAYER:  
 Depth (m)= 0.00 Porosity = 0.40  
 Maximum Mulch Layer Volume (cu.m.)= 0.00

ENGINEERED SOIL LAYER:  
 Soil moisture = 0.30 Depth (m)= 0.72  
 Width (m)= 3.00 Length (m)= 240.00  
 Porosity = 0.47 Infiltration (m/hr)= 0.0060  
 Maximum Engineered Soil Layer Volume(cu.m.)= 242.09

STORAGE LAYER:  
 Depth (m)= 1.00 Porosity = 0.40  
 Seepage (m/hr)= 0.0010  
 Maximum Storage Layer Volume(cu.m.)= 288.00

TOTAL AVAILABLE STORAGE:  
 TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil  
 + Storage Layer(cu.m.)= 680.04

-----  
 0.000 100.000 | 1.500 100.000  
 0.500 100.000 | 0.000 0.000  
 1.000 100.000 | 0.000 0.000  
 -----  
 MULCH LAYER:  
 Depth (m)= 0.00 Porosity = 0.40  
 Maximum Mulch Layer Volume (cu.m.)= 0.00

ENGINEERED SOIL LAYER:  
 Soil moisture = 0.30 Depth (m)= 0.72  
 Width (m)= 3.00 Length (m)= 240.00  
 Porosity = 0.47 Infiltration (m/hr)= 0.0060  
 Maximum Engineered Soil Layer Volume(cu.m.)= 242.09

| DEPTH (m) | DISCHARGE (cms) | DEPTH (m) | DISCHARGE (cms) |
|-----------|-----------------|-----------|-----------------|
| 0.000     | 0.000           | 0.762     | 0.030           |
| 0.279     | 0.010           | 0.762     | 0.040           |
| 0.406     | 0.020           | 1.300     | 0.050           |

STORAGE LAYER:  
 Depth (m)= 1.00 Porosity = 0.40  
 Seepage (m/hr)= 0.0010  
 Maximum Storage Layer Volume(cu.m.)= 288.00

TOTAL AVAILABLE STORAGE:  
 TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil  
 + Storage Layer(cu.m.)= 680.04

MATIVE SOIL LAYER:  
 Infiltration (m/hr)= 0.0060

| AREA (ha)      | QPEAK (cms) | TPEAK (hrs) | R.V. (mm)  |
|----------------|-------------|-------------|------------|
| INFLOW:ID= 2   | 0.59        | 0.032       | 1.33 12.11 |
| OUTFLOW:ID= 1  | 0.59        | 0.009       | 0.08 12.13 |
| OVERFLOW:ID= 3 | 0.00        | 0.00        | 0.00 0.00  |

Volume Reduction Rate(RvIn-RvOut)/RvIn (%)= -0.13  
 Time to reach Max Ponding Storage (Hr)= 0.00  
 Volume of water for drawdown in LID (cu.m.)= 69.17  
 Volume of Max. Water Storage (cu.m.)= 0.00  
 Maximum Surface Ponding And Mulch Vol(cu.m.)= 56.93  
 Maximum Engineered Soil Volume (cu.m.)= 69.86  
 \*\*\*\*\* After simulation, water volume is not zero.

-----  
 [ Junction Command(0330) |

-----  
 INFLOW: ID= 3( 0329) 0.00 0.000 0.00 0.00  
 OUTFLOW: ID= 2( 0330) 0.00 0.00 0.00 0.00

-----  
 ADD HYD ( 0310) | AREA QPEAK TPEAK R.V.  
 | 1 + 2 = 3 | (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0310): 0.00 0.000 0.00 0.00  
 + ID2= 2 ( 0332): 0.59 0.009 0.08 12.13  
 -----  
 ID = 3 ( 0310): 0.59 0.009 0.08 12.13  
 -----  
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 CALIB | STANDBY ( 0061) | Area (ha)= 0.59  
 | ID= 1 DT= 5.0 min | Total Imp(%)= 66.00 Dir. Conn.(%)= 25.00  
 -----  
 IMPERVIOUS PERVIOUS (1)  
 Surface Area (ha)= 0.39 0.20

-----  
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 ADD HYD ( 0061) | AREA QPEAK TPEAK R.V.  
 | 1 + 2 = 3 | (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0061): 6.76 0.688 1.33 14.26  
 + ID2= 2 ( 0306): 14.21 1.477 1.33 16.29  
 -----  
 ID = 3 ( 0061): 20.97 2.165 1.33 15.64  
 -----  
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 ADD HYD ( 0061) | AREA QPEAK TPEAK R.V.  
 | 1 + 2 = 3 | (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0061): 20.97 2.165 1.33 15.64  
 + ID2= 2 ( 0310): 0.59 0.009 0.08 12.13  
 -----  
 ID = 3 ( 0061): 21.56 2.170 1.33 15.54  
 -----  
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 [RESERVOIR( 0067) | OUTFLOW IS OFF  
 | In= 2--> OUT= 1 |  
 | DT= 5.0 min |

| OUTFLOW STORAGE (cms) | THAM. (cu.m.) | OUTFLOW STORAGE (cms) | THAM. (cu.m.) |
|-----------------------|---------------|-----------------------|---------------|
| 0.0000                | 0.0000        | 0.6610                | 0.9850        |
| 0.0400                | 0.1140        | 1.0960                | 1.5200        |
| 0.0760                | 0.3000        | 1.4260                | 1.9500        |
| 0.2810                | 0.6450        | 0.0000                | 0.0000        |

-----  
 INFLOW: ID= 2 ( 0061) 21.558 2.170 1.33 15.54  
 OUTFLOW: ID= 1 ( 0067) 21.558 0.063 3.92 15.51

-----  
 PEAK FLOW REDUCTION [Qout/Qin](%)= 2.89  
 TIME SPLIT OF PEAK FLOW (min)=15.00  
 MAXIMUM STORAGE USED (ha.m.)= 0.2746

```

***** DETAILED OUTPUT *****
Input filename: C:\Program Files (x86)\VisualInfo\W2\02\voim.dat
Output filename:
C:\Users\ksain\AppData\Local\Civil\GIS\4\9\9a870-2b3a-4142-8551-3404df768702\fed2
602-eac-4b3-9c61-89b26ebd24\scena
Summary filename:
C:\Users\ksain\AppData\Local\Civil\GIS\4\9\9a870-2b3a-4142-8551-3404df768702\fed2
602-eac-4b3-9c61-89b26ebd24\scena
DATE: 08-28-2022 TIME: 02:36:03
USER:
COMMENTS:
***** SIMULATION : A. 2yr 3hr 10min Chicago *****
CHICAGO STORM ID# curve parameters: A= 404.147
POT(24) 31.23 mm Total Imp(%)= 65.000 Dir. Conn.(%)= 65.000
C= 0.699

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 1.66 0.028 1.25 6.66
OVERFLOW: ID= 3 0.00 0.000 0.000 0.00
Volume Reduction Rate((Rvin-Rvout)/Rvin) (%)= 100.00
Time to reach Max Ponding Storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 162.93
Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 181.58
Maximum Engineered Soil Volume (cu.m.)= 63.28
**** After simulation, water volume is not zero.
Junction Command(0181)
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 3 (0184) 0.00 0.00 0.00 0.00
OUTFLOW: ID= 2 (0181) 0.00 0.00 0.00 0.00
CALIB
STANDHYD ( 0205) Area (ha)= 0.86
ID# 1 DT= 5.0 min Total Imp(%)= 65.000 Dir. Conn.(%)= 65.000
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.56 0.30
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (ft)= 75.87 40.00
Mannings n = 0.013 0.250
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

```

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 74.0 ID= 1 Dep. Storage (above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
DUHDY ( 0263)
Inlet Cap.= 0.174
# of Inlets= 1
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
TOTAL HYD. (ID= 1): 0.86 0.13 1.00 22.44
MAJOR SYS. (ID= 2): 0.00 0.00 0.00 0.00
MINOR SYS. (ID= 3): 0.86 0.13 1.00 22.44
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
Junction Command(0262)
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 3 (0264) 0.86 0.01 1.83 4.49
OUTFLOW: ID= 2 (0262) 0.86 0.01 1.83 4.49
ADD HYD ( 0243)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 3 (0243) 0.86 0.01 1.83 4.49
OUTFLOW: ID= 1 (0188) 0.86 0.009 2.08 4.49
ID= 3 ( 0243): 0.86 0.011 1.83 4.49
ID= 2 ( 0262): 0.86 0.011 1.83 4.49
ID= 1 ( 0243): 0.86 0.011 1.83 4.49
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
RESERVOIR (0181) OVERFLOW IS OFF
ID# 2 -> OUT# 1
DT= 5.0 min
OUTFLOW STORAGE
(cms) (ha.m.)
0.0000 0.0000 0.4800 0.0190
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 ( 0243) 0.863 0.011 1.83 4.49
OUTFLOW: ID= 1 ( 0188) 0.863 0.009 2.08 4.49
PEAK FLOW REDUCTION (Qout/Qin)(%)= 83.17
TIME SPLIT OF PEAK FLOW (min)= 15.00
MAXIMUM STORAGE USED (ha.m.)= 0.0004

```

```

used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 3.76 | 0.833 13.95 | 1.583 6.78 | 2.33 4.23
0.167 3.76 | 0.917 80.82 | 1.667 6.78 | 2.42 3.90
0.250 4.42 | 1.000 80.82 | 1.750 5.83 | 2.50 3.90
0.333 4.42 | 1.083 17.11 | 1.833 5.83 | 2.58 3.63
0.417 5.48 | 1.167 17.11 | 1.917 5.15 | 2.67 3.63
0.500 5.48 | 1.250 10.79 | 2.000 5.15 | 2.75 3.39
0.583 7.50 | 1.333 10.79 | 2.083 4.63 | 2.83 3.39
0.667 7.50 | 1.417 8.23 | 2.167 4.63 | 2.92 3.20
0.750 13.95 | 1.500 8.23 | 2.250 4.23 | 3.00 3.20
CALIB
RESHYD ( 0185) Area (ha)= 1.66 Curve Number (CN)= 74.4
ID# 1 DT= 5.0 min Ia (mm)= 4.44 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.24
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

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***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 74.0 ID= 1 Dep. Storage (above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
DUHDY ( 0263)
Inlet Cap.= 0.174
# of Inlets= 1
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
TOTAL HYD. (ID= 1): 0.86 0.13 1.00 22.44
MAJOR SYS. (ID= 2): 0.00 0.00 0.00 0.00
MINOR SYS. (ID= 3): 0.86 0.13 1.00 22.44
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
Junction Command(0262)
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 3 (0264) 0.86 0.01 1.83 4.49
OUTFLOW: ID= 2 (0262) 0.86 0.01 1.83 4.49
ADD HYD ( 0243)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 3 (0243) 0.86 0.01 1.83 4.49
OUTFLOW: ID= 2 (0262) 0.86 0.01 1.83 4.49

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```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 74.0 ID= 1 Dep. Storage (above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
CALIB
STANDHYD ( 0252) Area (ha)= 0.32
ID# 1 DT= 5.0 min Total Imp(%)= 69.000 Dir. Conn.(%)= 30.000
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.22 0.10
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (ft)= 45.84 40.00
Mannings n = 0.013 0.250
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

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DUHDY ( 0182)
Inlet Cap.= 0.174
# of Inlets= 1
TOTAL Imp(%)= 0.11 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
TOTAL HYD. (ID= 1): 1.66 0.03 1.25 6.66
MAJOR SYS. (ID= 2): 0.00 0.00 0.00 0.00
MINOR SYS. (ID= 3): 1.66 0.03 1.25 6.66
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
RESERVOIR (0184) OUTFLOW: OFF, UNDERDRAIN: OFF
ID# 2 -> OUT# 3
DT= 5.0 min
SURFACE PONDING LAYER:
Max. Ponding Storage(cu.m.)= 149.95
STAGE AREA STAGE AREA
(m) (m2) (m) (m2)
0.000 100.000 | 1.500 100.000
0.500 100.000 | 0.000 0.000
1.000 100.000 | 0.000 0.000
MULCH LAYER:
Depth (m)= 0.10 Porosity = 0.40
Maximum Mulch Layer Volume (cu.m.)= 21.60
ENGINEERED SOIL LAYER:
Soil moisture = 0.30 Depth (m)= 0.72
Width (m)= 3.00 Length (m)= 0.0000
Porosity = 0.47 Infiltration (m/hr)= 0.0000
Maximum Engineered Soil Layer Volume(cu.m.)= 181.57
STORAGE LAYER:
Depth (m)= 0.50 Porosity = 0.40
Seepage (m/hr)= 0.0010
Maximum Storage Layer Volume(cu.m.)= 108.00
TOTAL AVAILABLE STORAGE:
TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil
+Storage Layer(cu.m.)= 461.12
NATIVE SOIL LAYER:
Infiltration (m/hr)= 0.0060

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```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 74.0 ID= 1 Dep. Storage (above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
DUHDY ( 0263)
Inlet Cap.= 0.174
# of Inlets= 1
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
TOTAL HYD. (ID= 1): 0.86 0.13 1.00 22.44
MAJOR SYS. (ID= 2): 0.00 0.00 0.00 0.00
MINOR SYS. (ID= 3): 0.86 0.13 1.00 22.44
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
Junction Command(0262)
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 3 (0264) 0.86 0.01 1.83 4.49
OUTFLOW: ID= 2 (0262) 0.86 0.01 1.83 4.49

```

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 74.0 ID= 1 Dep. Storage (above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
CALIB
STANDHYD ( 0252) Area (ha)= 0.32
ID# 1 DT= 5.0 min Total Imp(%)= 69.000 Dir. Conn.(%)= 30.000
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.22 0.10
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (ft)= 45.84 40.00
Mannings n = 0.013 0.250
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

```



Retention( 0329) | OUTFLOW: OFF UNDERDRAIN: ON  
 | DT= 5.0 min

SURFACE PONDING LAYER:

Max. Ponding Storage(cu.m.)= 149.95

| STAGE (m) | AREA (m2) | STAGE (m) | AREA (m2) |
|-----------|-----------|-----------|-----------|
| 0.000     | 100.000   | 1.500     | 100.000   |
| 0.500     | 100.000   | 0.000     | 0.000     |
| 1.000     | 100.000   | 0.000     | 0.000     |

MULCH LAYER:  
 Depth (m)= 0.00 Porosity = 0.40  
 Maximum Mulch Layer Volume (cu.m.)= 0.00

ENGINEERED SOIL LAYER:  
 Soil moisture = 0.30 Depth (m)= 0.72  
 Width (m)= 3.00 length (m)=240.00  
 Porosity = 0.47 Infiltration (m/hr)= 0.0060  
 Maximum Engineered Soil Layer Volume(cu.m.)= 242.09

| DEPTH (m) | DISCHARGE (cms) | DEPTH (m) | DISCHARGE (cms) |
|-----------|-----------------|-----------|-----------------|
| 0.000     | 0.000           | 0.559     | 0.030           |
| 0.279     | 0.010           | 0.762     | 0.040           |
| 0.406     | 0.020           | 1.300     | 0.050           |

STORAGE LAYER:  
 Depth (m)= 1.00 Porosity = 0.40  
 Seepage (m/hr)= 0.0010  
 Maximum Storage Layer Volume(cu.m.)= 288.00

TOTAL AVAILABLE STORAGE:

TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil + Storage Layer(cu.m.)= 680.04

NATIVE SOIL LAYER:  
 Infiltration (m/hr)= 0.0060

| INFLOW: ID= | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-------------|-----------|-------------|-------------|-----------|
| 2           | 0.59      | 0.043       | 1.00        | 17.29     |
| 1           | 0.59      | 0.009       | 0.08        | 11.13     |
| 3           | 0.00      | 0.000       | 0.00        | 0.00      |

Volume Reduction Rate[(RVin-RVout)/RVin] (%)= 35.61

Time to reach Max Ponding storage (Hr)= 0.00  
 Volume of water for drawdown in LID (cu.m.)= 106.32  
 Volume of Max. Water Storage (cu.m.)= 0.00  
 Maximum Surface Ponding And Mulch Vol(cu.m.)= 88.68  
 Maximum Engineered Soil Volume (cu.m.)= 69.86  
 \*\*\*\*\* After simulation, water volume is not zero.

Junction Command(0330) |

| INFLOW: ID= | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-------------|-----------|-------------|-------------|-----------|
| 3 ( 0329)   | 0.00      | 0.00        | 0.00        | 0.00      |
| 2 ( 0330)   | 0.00      | 0.00        | 0.00        | 0.00      |

Junction Command(0332) |

| INFLOW: ID= | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-------------|-----------|-------------|-------------|-----------|
| 1 ( 0329)   | 0.59      | 0.01        | 0.08        | 11.13     |
| 2 ( 0332)   | 0.59      | 0.01        | 0.08        | 11.13     |

ADD HYD ( 0310) |

| 1 + 2 = 3 | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-----------|-------------|-------------|-----------|
| 1 ( 0329) | 0.59      | 0.01        | 0.08        | 11.13     |
| 2 ( 0332) | 0.59      | 0.01        | 0.08        | 11.13     |

\*\*\* W A R N I N G : HYDROGRAPH 0310 <ID= 1> IS DRY.  
 \*\*\* W A R N I N G : HYDROGRAPH 0331 <ID= 2> IS DRY.  
 \*\*\* W A R N I N G : HYDROGRAPH 0310 <ID= 3> IS ALSO DRY

ADD HYD ( 0310) |

| 1 + 2 = 1 | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-----------|-------------|-------------|-----------|
| 3 ( 0310) | 0.00      | 0.000       | 0.00        | 0.00      |
| 2 ( 0332) | 0.59      | 0.009       | 0.08        | 11.13     |
| 1 ( 0310) | 0.59      | 0.009       | 0.08        | 11.13     |

\*\*\* W A R N I N G : HYDROGRAPH 0310 <ID= 3> IS DRY.  
 \*\*\* W A R N I N G : HYDROGRAPH 0310 = HYDROGRAPH 0332  
 ID1= 3 ( 0310): 0.00 0.000 0.00 0.00  
 + ID2= 2 ( 0332): 0.59 0.009 0.08 11.13  
 ID = 1 ( 0310): 0.59 0.009 0.08 11.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0061) |

| 1 + 2 = 3 | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-----------|-------------|-------------|-----------|
| 1 ( 0276) | 1.28      | 0.138       | 1.00        | 23.68     |
| 2 ( 0304) | 5.48      | 0.671       | 1.00        | 18.65     |
| 3 ( 0061) | 6.76      | 0.870       | 1.00        | 19.60     |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0061) |

| 3 + 2 = 1 | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-----------|-------------|-------------|-----------|
| 3 ( 0061) | 6.76      | 0.870       | 1.00        | 19.60     |
| 2 ( 0306) | 14.21     | 1.967       | 1.00        | 22.17     |
| 1 ( 0061) | 20.97     | 2.836       | 1.00        | 21.35     |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0061) |

| 1 + 2 = 3 | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----------|-----------|-------------|-------------|-----------|
| 1 ( 0061) | 20.97     | 2.836       | 1.00        | 21.35     |
| 2 ( 0310) | 0.59      | 0.009       | 0.08        | 11.13     |
| 3 ( 0061) | 21.56     | 2.842       | 1.00        | 21.07     |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR ( 0067) | OVERFLOW IS OFF

| DT= 5.0 min | OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (ha.m.) | STORAGE (ha.m.) |
|-------------|---------------|-----------------|-----------------|-----------------|
|             | 0.0000        | 0.0000          | 0.0000          | 0.9850          |
|             | 0.0400        | 0.1140          | 1.0960          | 1.3320          |
|             | 0.0760        | 0.3690          | 1.4260          | 1.5860          |
|             | 0.2810        | 0.6450          | 0.0000          | 0.0000          |

| INFLOW: ID= | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-------------|-----------|-------------|-------------|-----------|
| 2 ( 0061)   | 21.558    | 2.842       | 1.00        | 21.07     |

OUTFLOW: ID= 1 ( 0067) 21.558 0.092 3.08 21.03

PEAK FLOW REDUCTION [(Qout/Qin)(%)] = 3.25  
 TIME SHIFT OF PEAK FLOW (min)=125.00  
 MAXIMUM STORAGE USED (ha.m.)= 0.3914

FINISH







NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Orientation( 0329) | OUTFLOW: OFF UNDERDRAIN: ON  
 |He 2-> OUT= 3 |  
 | DT= 5.0 min |

SURFACE PONDING LAYER:  
 Max. Ponding Storage(cu.m.)= 149.95

| STAGE<br>(m) | AREA<br>(m2) | STAGE<br>(m) | AREA<br>(m2) |
|--------------|--------------|--------------|--------------|
| 0.000        | 100.000      | 1.500        | 100.000      |
| 0.500        | 100.000      | 0.000        | 0.000        |
| 1.000        | 100.000      | 0.000        | 0.000        |

MULCH LAYER:  
 Depth (m)= 0.00 Porosity = 0.40  
 Maximum Mulch Layer Volume (cu.m.)= 0.00

ENGINEERED SOIL LAYER:  
 Soil moisture = 0.30 Depth (m)= 0.72  
 WELD (m)= 3.00 Length (m)=240.00  
 Porosity = 0.47 Infiltration (m/hr)= 0.0060  
 Maximum Engineered Soil Layer Volume(cu.m.)= 242.09

| DEPTH<br>(m) | DISCHARGE<br>(cms) | DEPTH<br>(m) | DISCHARGE<br>(cms) |
|--------------|--------------------|--------------|--------------------|
| 0.000        | 0.000              | 0.550        | 0.030              |
| 0.279        | 0.010              | 0.762        | 0.040              |
| 0.406        | 0.020              | 1.300        | 0.050              |

STORAGE LAYER:  
 Depth (m)= 1.00 Porosity = 0.40  
 Seepage (m/hr)= 0.0010  
 Maximum Storage Layer Volume(cu.m.)= 288.00

TOTAL AVAILABLE STORAGE:

TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil  
 +Storage Layer(cu.m.)= 606.04

NATIVE SOIL LAYER:  
 Infiltration (m/hr)= 0.0060

| AREA<br>(ha)           | QPEAK<br>(cms) | TPEAK<br>(hrs) | R.V.<br>(mm) |       |
|------------------------|----------------|----------------|--------------|-------|
| INFLOW : ID= 2 ( 0061) | 21.558         | 3.995          | 1.00         | 29.45 |
| OUTFLOW: ID= 1 ( 0067) | 21.558         | 0.186          | 3.00         | 29.42 |

PEAK FLOW REDUCTION [Qout/Qin](%)= 4.65  
 TIME SHEET OF PEAK FLOW (min)=120.00  
 MAXIMUM STORAGE USED (ha.m.)= 0.5171

OUTFLOW:ID= 1 0.59 0.009 0.08 11.05  
 OVERFLOW:ID= 3 0.00 0.000 0.00 0.00

Volume Reduction Rate[(Rvin-Rvout)/Rvin] (%)= 56.60  
 Time to reach Max Ponding storage (hr)= 0.00  
 Volume of water for drawdown In IID (cu.m.)= 155.30  
 Volume of Max. Water Storage (cu.m.)= 0.00  
 Maximum Surface Ponding And Mulch Vol(cu.m.)= 136.90  
 Maximum Engineered Soil Volume (cu.m.)= 69.86  
 \*\*\*\*\* After simulation, water volume is not zero.

| Junction Command(0330) |

| AREA<br>(ha)          | QPEAK<br>(cms) | TPEAK<br>(hrs) | R.V.<br>(mm) |      |
|-----------------------|----------------|----------------|--------------|------|
| INFLOW : ID= 3( 0329) | 0.00           | 0.00           | 0.00         | 0.00 |
| OUTFLOW: ID= 2( 0330) | 0.00           | 0.00           | 0.00         | 0.00 |

| Junction Command(0332) |

| AREA<br>(ha)          | QPEAK<br>(cms) | TPEAK<br>(hrs) | R.V.<br>(mm) |       |
|-----------------------|----------------|----------------|--------------|-------|
| INFLOW : ID= 1( 0329) | 0.59           | 0.01           | 0.08         | 11.05 |
| OUTFLOW: ID= 2( 0332) | 0.59           | 0.01           | 0.08         | 11.05 |

| ADD HYD ( 0310) |

| 1 + 2 = 3   | AREA<br>(ha) | QPEAK<br>(cms) | TPEAK<br>(hrs) | R.V.<br>(mm) |
|---|--------------|----------------|----------------|--------------|
| *** W A R N I N G : HYDROGRAPH 0330 :ID= 1> IS DRY.     |              |                |                |              |
| *** W A R N I N G : HYDROGRAPH 0331 :ID= 2> IS DRY.     |              |                |                |              |
| *** W A R N I N G : HYDROGRAPH 0310 :ID= 3> IS ALSO DRY |              |                |                |              |

| ADD HYD ( 0310) |

| 3 + 2 = 1   | AREA<br>(ha) | QPEAK<br>(cms) | TPEAK<br>(hrs) | R.V.<br>(mm) |
|---|--------------|----------------|----------------|--------------|
| *** W A R N I N G : HYDROGRAPH 0310 :ID= 3> IS DRY.   |              |                |                |              |
| *** W A R N I N G : HYDROGRAPH 0310 = HYDROGRAPH 0332 |              |                |                |              |
| ID= 3 ( 0330): 0.00 0.000 0.00 0.00                   |              |                |                |              |
| + ID2= 2 ( 0332): 0.59 0.009 0.08 11.05               |              |                |                |              |

ID = 1 ( 0310): 0.59 0.009 0.08 11.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD ( 0061) |

| 1 + 2 = 3         | AREA<br>(ha) | QPEAK<br>(cms) | TPEAK<br>(hrs) | R.V.<br>(mm) |
|-------------------|--------------|----------------|----------------|--------------|
| ID1= 1 ( 0276):   | 1.28         | 0.276          | 1.00           | 32.43        |
| + ID2= 2 ( 0304): | 5.48         | 0.920          | 1.00           | 26.69        |
| ID = 3 ( 0061):   | 6.76         | 1.197          | 1.00           | 27.78        |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD ( 0061) |

| 3 + 2 = 1         | AREA<br>(ha) | QPEAK<br>(cms) | TPEAK<br>(hrs) | R.V.<br>(mm) |
|-------------------|--------------|----------------|----------------|--------------|
| ID1= 3 ( 0061):   | 6.76         | 1.197          | 1.00           | 27.78        |
| + ID2= 2 ( 0306): | 14.21        | 2.792          | 1.00           | 31.02        |
| ID = 1 ( 0061):   | 20.97        | 3.989          | 1.00           | 29.97        |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD ( 0061) |

| 1 + 2 = 3         | AREA<br>(ha) | QPEAK<br>(cms) | TPEAK<br>(hrs) | R.V.<br>(mm) |
|-------------------|--------------|----------------|----------------|--------------|
| ID1= 1 ( 0061):   | 20.97        | 3.989          | 1.00           | 29.97        |
| + ID2= 2 ( 0310): | 0.59         | 0.009          | 0.08           | 11.05        |
| ID = 3 ( 0061):   | 21.56        | 3.995          | 1.00           | 29.45        |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| RESERVOIR( 0067) |

| DT= 5.0 min    | OUTFLOW<br>(cms) | STORAGE<br>(ha.m.) | OUTFLOW<br>(cms) | STORAGE<br>(ha.m.) |
|----------------|------------------|--------------------|------------------|--------------------|
| IN 2->> OUT= 1 |                  |                    |                  |                    |
| 0.0000         | 0.0000           | 0.0000             | 0.6610           | 0.9850             |
| 0.0400         | 0.1140           | 1.0960             | 1.3320           | 1.3320             |
| 0.0760         | 0.3600           | 1.4760             | 1.5960           | 1.5960             |
| 0.2810         | 0.6450           | 0.0000             | 0.0000           | 0.0000             |

| AREA<br>(ha)           | QPEAK<br>(cms) | TPEAK<br>(hrs) | R.V.<br>(mm) |       |
|------------------------|----------------|----------------|--------------|-------|
| INFLOW : ID= 2 ( 0061) | 21.558         | 3.995          | 1.00         | 29.45 |
| OUTFLOW: ID= 1 ( 0067) | 21.558         | 0.186          | 3.00         | 29.42 |

PEAK FLOW REDUCTION [Qout/Qin](%)= 4.65  
 TIME SHEET OF PEAK FLOW (min)=120.00  
 MAXIMUM STORAGE USED (ha.m.)= 0.5171

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V V I SSSSS U U A L L (v 6.2.2008)
V V I SS U U A A L L
V V I SS U U A A A L L
W I SSSSS UUUU A A LLLLL

000 TITIT TITIT H H Y Y M M 000 TN
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\VisualOTHM6 6.2\vo\vo\in.dat

Output filename:
C:\Users\kskadin\AppData\Local\Civil3D\GIS\4c9a8b70-2b3a-4142-8551-3404df768702\c0fc6
c5b-16c8-8fbd-8648-c093f3e0baa\scena
Summary filename:
C:\Users\kskadin\AppData\Local\Civil3D\GIS\4c9a8b70-2b3a-4142-8551-3404df768702\c0fc6
c5b-16c8-8fbd-8648-c093f3e0baa\scena

DATE: 08-28-2022 TIME: 02:37:54

USER:

COMMENTS:

-----
CHICAGO STORM ID# curve parameters: A= 622.842
POTail= 49.52 mm Inlet Cap= 0.174
C= 0.699
-----

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used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.00 5.79 0.83 124.56 1.67 8.98 2.50 5.59
0.17 6.82 1.00 26.38 1.83 7.93 2.67 5.23
0.33 8.45 1.17 16.63 2.00 7.14 2.83 4.93
0.50 11.56 1.33 12.68 2.17 6.51
0.67 21.49 1.50 10.45 2.33 6.00

CALIB STAMHVD ( 0185) Area (ha)= 1.66 Curve Number (CN)= 74.4
ID# 1 Dt= 5.0 min Ia (mm)= 4.44 # of Linear Rvs.(N)= 3.00
U.H. Tp(hrs)= 0.24

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

-----
TRANSFORMED HYETOGRAPH
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.79 0.833 21.49 1.583 10.45 2.33 6.51
0.167 5.79 0.917 124.56 1.667 10.45 2.42 6.01
0.250 6.82 1.000 124.56 1.750 8.98 2.50 6.01
0.333 6.82 1.083 26.38 1.833 8.98 2.58 5.59
0.417 8.45 1.167 26.38 1.917 7.93 2.67 5.59
0.500 8.45 1.250 16.63 2.000 7.93 2.75 5.23
0.583 11.56 1.333 16.63 2.083 7.14 2.83 5.23
0.667 11.56 1.417 12.68 2.167 7.14 2.92 4.93
0.750 21.49 1.500 12.68 2.250 6.51 3.00 4.93

Unit Hyd Peak (cms)= 0.265
PEAK FLOW (cms)= 0.069 (1)
TIME TO PEAK (hrs)= 1.167
RUNOFF VOLUME (mm)= 15.325
TOTAL RAINFALL (mm)= 49.520
RUNOFF COEFFICIENT = 0.309

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
-----

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DUHDV ( 0182)
Inlet Cap.= 0.174
# of Inlets= 1
Total Imp(%)= 0.11 Area QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
TOTAL HVD(ID# 1): 1.66 0.07 1.17 15.33
MAJOR SYS.(ID# 2): 0.00 0.00 0.00 0.00
MINOR SYS.(ID# 3): 1.66 0.07 1.17 15.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----
|bioretention( 0184) | OUTFLOW: OFF, UNDERDRAIN: OFF
|In 2-> OUT 3 |
|Dt= 5.0 min |
-----
SURFACE PONDING LAYER:
Max. Ponding Storage(cu.m.)= 149.95

STAGE AREA STAGE AREA
(m) (m2) (m) (m2)
0.000 100.000 1.500 100.000
1.000 100.000 0.000 0.000

MULCH LAYER:
Depth (m)= 0.10 Porosity = 0.40
Maximum Mulch Layer Volume (cu.m.)= 21.60

ENGINEERED SOIL LAYER:
Soil moisture = 0.30 Depth (m)= 0.72
Width (m)= 100.00 Length (m)= 3.00
Porosity = 0.47 Infiltration (m/hr)= 0.0000
Maximum Engineered Soil Layer Volume(cu.m.)= 181.57

STORAGE LAYER:
Depth (m)= 0.50 Porosity = 0.40
Seepage (m/hr)= 0.0010
Maximum Storage Layer Volume(cu.m.)= 108.00

TOTAL AVAILABLE STORAGE:
TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil
+Storage Layer(cu.m.)= 461.12

NATIVE SOIL LAYER:
Infiltration (m/hr)= 0.0060
-----

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AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID# 2 1.66 0.069 1.17 15.33
OVERFLOW: ID# 3 1.66 0.021 2.08 4.42

Volume Reduction Rate[(RVin-RVout)/RVin] (%)= 71.18
Time to reach Max Ponding Storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 233.50
Volume of Max. water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 171.55
Maximum Engineered Soil Volume (cu.m.)= 64.12
**** After simulation, water volume is not zero.

Junction Command(0181)
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID# 3 ( 0184) 1.66 0.02 2.08 4.42
OUTFLOW: ID# 2 ( 0181) 1.66 0.02 2.08 4.42

CALIB STAMHVD ( 0205) Area (ha)= 0.80
ID# 1 Dt= 5.0 min Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00

IMPERVIOUS PERVIOUS (1)
Surface Area (ha)= 0.56 0.20
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 75.87 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

-----
TRANSFORMED HYETOGRAPH
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.79 0.833 21.49 1.583 10.45 2.33 6.51
0.167 5.79 0.917 124.56 1.667 10.45 2.42 6.01
0.250 6.82 1.000 124.56 1.750 8.98 2.58 5.59
0.333 6.82 1.083 26.38 1.833 8.98 2.58 5.59
0.417 8.45 1.167 26.38 1.917 7.93 2.67 5.59
0.500 8.45 1.250 16.63 2.000 7.93 2.75 5.23
0.583 11.56 1.333 16.63 2.083 7.14 2.83 5.23
0.667 11.56 1.417 12.68 2.167 7.14 2.92 4.93
0.750 21.49 1.500 12.68 2.250 6.51 3.00 4.93

Max.Eff.Inten.(mm/hr)= 124.56 24.35
over (min)= 5.00 15.00
Storage Coeff. (min)= 1.08 (11) 14.40 (11)
Unit Hyd. Peak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.31 0.08

*TOTALS*
PEAK FLOW (cms)= 0.19 0.01 0.199 (111)
TIME TO PEAK (hrs)= 1.00 1.17 1.00
RUNOFF VOLUME (mm)= 48.52 14.82 36.71
TOTAL RAINFALL (mm)= 49.52 49.52 49.52
RUNOFF COEFFICIENT = 0.98 0.30 0.74

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
-----
DUHDV ( 0263)
Inlet Cap.= 0.174
# of Inlets= 1
Total Imp(%)= 0.21 Area QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
TOTAL HVD.(ID# 1): 0.86 0.20 1.00 36.71
MAJOR SYS.(ID# 2): 0.03 0.02 1.00 36.71
MINOR SYS.(ID# 3): 0.84 0.17 0.92 36.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----
|bioretention( 0264) | OUTFLOW: OFF, UNDERDRAIN: OFF
|In 2-> OUT 3 |
|Dt= 5.0 min |
-----
SURFACE PONDING LAYER:
Max. Ponding Storage(cu.m.)= 149.95

STAGE AREA STAGE AREA
(m) (m2) (m) (m2)
0.000 100.000 1.500 100.000
1.000 100.000 0.000 0.000

INFLOW: ID# 3 ( 0264) 0.84 0.05 1.17 18.18
OUTFLOW: ID# 2 ( 0262) 0.84 0.05 1.17 18.18

Junction Command(0262)
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID# 3 ( 0264) 0.84 0.05 1.17 18.18
OUTFLOW: ID# 2 ( 0262) 0.84 0.05 1.17 18.18

ADD HVD ( 0243)
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
-----

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AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID# 3 1.66 0.069 1.17 15.33
OVERFLOW: ID# 3 1.66 0.021 2.08 4.42

Volume Reduction Rate[(RVin-RVout)/RVin] (%)= 71.18
Time to reach Max Ponding Storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 233.50
Volume of Max. water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 171.55
Maximum Engineered Soil Volume (cu.m.)= 64.12
**** After simulation, water volume is not zero.

Junction Command(0181)
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID# 3 ( 0184) 1.66 0.02 2.08 4.42
OUTFLOW: ID# 2 ( 0181) 1.66 0.02 2.08 4.42

CALIB STAMHVD ( 0205) Area (ha)= 0.80
ID# 1 Dt= 5.0 min Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00

IMPERVIOUS PERVIOUS (1)
Surface Area (ha)= 0.56 0.20
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 75.87 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

-----
TRANSFORMED HYETOGRAPH
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.79 0.833 21.49 1.583 10.45 2.33 6.51
0.167 5.79 0.917 124.56 1.667 10.45 2.42 6.01
0.250 6.82 1.000 124.56 1.750 8.98 2.58 5.59
0.333 6.82 1.083 26.38 1.833 8.98 2.58 5.59
0.417 8.45 1.167 26.38 1.917 7.93 2.67 5.59
0.500 8.45 1.250 16.63 2.000 7.93 2.75 5.23
0.583 11.56 1.333 16.63 2.083 7.14 2.83 5.23
0.667 11.56 1.417 12.68 2.167 7.14 2.92 4.93
0.750 21.49 1.500 12.68 2.250 6.51 3.00 4.93

Max.Eff.Inten.(mm/hr)= 124.56 24.35
over (min)= 5.00 15.00
Storage Coeff. (min)= 1.08 (11) 14.40 (11)
Unit Hyd. Peak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.31 0.08

*TOTALS*
PEAK FLOW (cms)= 0.19 0.01 0.199 (111)
TIME TO PEAK (hrs)= 1.00 1.17 1.00
RUNOFF VOLUME (mm)= 48.52 14.82 36.71
TOTAL RAINFALL (mm)= 49.52 49.52 49.52
RUNOFF COEFFICIENT = 0.98 0.30 0.74

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
-----
DUHDV ( 0263)
Inlet Cap.= 0.174
# of Inlets= 1
Total Imp(%)= 0.21 Area QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
TOTAL HVD.(ID# 1): 0.86 0.20 1.00 36.71
MAJOR SYS.(ID# 2): 0.03 0.02 1.00 36.71
MINOR SYS.(ID# 3): 0.84 0.17 0.92 36.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----
|bioretention( 0264) | OUTFLOW: OFF, UNDERDRAIN: OFF
|In 2-> OUT 3 |
|Dt= 5.0 min |
-----
SURFACE PONDING LAYER:
Max. Ponding Storage(cu.m.)= 149.95

STAGE AREA STAGE AREA
(m) (m2) (m) (m2)
0.000 100.000 1.500 100.000
1.000 100.000 0.000 0.000

INFLOW: ID# 3 ( 0264) 0.84 0.05 1.17 18.18
OUTFLOW: ID# 2 ( 0262) 0.84 0.05 1.17 18.18

Junction Command(0262)
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID# 3 ( 0264) 0.84 0.05 1.17 18.18
OUTFLOW: ID# 2 ( 0262) 0.84 0.05 1.17 18.18

ADD HVD ( 0243)
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
-----

```

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DUHDV ( 0182)
Inlet Cap.= 0.174
# of Inlets= 1
Total Imp(%)= 0.11 Area QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
TOTAL HVD(ID# 1): 1.66 0.07 1.17 15.33
MAJOR SYS.(ID# 2): 0.00 0.00 0.00 0.00
MINOR SYS.(ID# 3): 1.66 0.07 1.17 15.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----
|bioretention( 0184) | OUTFLOW: OFF, UNDERDRAIN: OFF
|In 2-> OUT 3 |
|Dt= 5.0 min |
-----
SURFACE PONDING LAYER:
Max. Ponding Storage(cu.m.)= 149.95

STAGE AREA STAGE AREA
(m) (m2) (m) (m2)
0.000 100.000 1.500 100.000
1.000 100.000 0.000 0.000

MULCH LAYER:
Depth (m)= 0.10 Porosity = 0.40
Maximum Mulch Layer Volume (cu.m.)= 21.60

ENGINEERED SOIL LAYER:
Soil moisture = 0.30 Depth (m)= 0.67
Width (m)= 40.00 Length (m)= 2.93
Porosity = 0.47 Infiltration (m/hr)= 0.0000
Maximum Engineered Soil Layer Volume(cu.m.)= 25.83

STORAGE LAYER:
Depth (m)= 1.50 Porosity = 0.40
Seepage (m/hr)= 0.0010
Maximum Storage Layer Volume(cu.m.)= 48.00

TOTAL AVAILABLE STORAGE:
TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil
+Storage Layer(cu.m.)= 226.18

NATIVE SOIL LAYER:
Infiltration (m/hr)= 0.0060
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(ha) (cms) (hrs) (mm)
*** WARNING: HYDROGRAPH 0182 <ID# 2> IS DRY.
*** WARNING: HYDROGRAPH 0003 <ID# 2> IS DRY 0001
ID# 1 ( 0183): 1.66 0.021 2.08 4.42
+ ID# 2 ( 0182): 0.00 0.000 0.00 0.00
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HVD ( 0243)
| 3 + 2 = 1 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID# 1 ( 0243): 2.50 0.055 1.17 9.02
+ ID# 2 ( 0263): 0.03 0.025 1.00 36.71
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR( 0188) OVERFLOW IS OFF
|Dt= 2-> OUT 1 |
|Dt= 5.0 min |
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 0.4800 0.0190

INFLOW: ID# 2 ( 0243) 2.50 0.055 1.17 9.02
INFLOW: ID# 1 ( 0188) 2.527 0.043 1.25 9.32

PEAK FLOW REDUCTION (Qout/Qin)(%)= 78.82
TIME SHIFT OF PEAK FLOW (min)= 5.00
MAXIMUM STORAGE USED (ha.m.)= 0.0018
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CALIB STAMHVD ( 0252) Area (ha)= 0.32
ID# 1 Dt= 5.0 min Total Imp(%)= 69.00 Dir. Conn.(%)= 30.00

IMPERVIOUS PERVIOUS (1)
Surface Area (ha)= 0.22 0.10
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 45.84 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

-----
TRANSFORMED HYETOGRAPH
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.79 0.833 21.49 1.583 10.45 2.33 6.51
0.167 5.79 0.917 124.56 1.667 10.45 2.42 6.01
0.250 6.82 1.000 124.56 1.750 8.98 2.58 6.01
0.333 6.82 1.083 26.38 1.833 8.98 2.58 5.59
0.417 8.45 1.167 26.38 1.917 7.93 2.67 5.59
0.500 8.45 1.250 16.63 2.000 7.93 2.75 5.23
0.583 11.56 1.333 16.63 2.083 7.14 2.83 5.23
0.667 11.56 1.417 12.68 2.167 7.14 2.92 4.93
0.750 21.49 1.500 12.68 2.250 6.51 3.00 4.93

Max.Eff.Inten.(mm/hr)= 124.56 139.90
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.17 (11) 7.64 (11)
Unit Hyd. Peak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.33 0.13

*TOTALS*
PEAK FLOW (cms)= 0.03 0.03 0.054 (111)
TIME TO PEAK (hrs)= 1.00 1.08 1.00
RUNOFF VOLUME (mm)= 48.52 25.77 32.58
TOTAL RAINFALL (mm)= 49.52 49.52 49.52
RUNOFF COEFFICIENT = 0.98 0.52 0.66

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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ADD HVD ( 0179)
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID# 1 ( 0188): 2.53 0.043 1.25 9.32
+ ID# 2 ( 0252): 0.32 0.054 1.00 32.58
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STAMHVD ( 0106) Area (ha)= 0.51
ID# 1 Dt= 5.0 min Total Imp(%)= 55.00 Dir. Conn.(%)= 1.00

IMPERVIOUS PERVIOUS (1)
Surface Area (ha)= 0.28 0.23
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 58.22 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

-----
TRANSFORMED HYETOGRAPH
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.79 0.833 21.49 1.583 10.45 2.33 6.51
0.167 5.79 0.917 124.56 1.667 10.45 2.42 6.01
0.250 6.82 1.000 124.56 1.750 8.98 2.58 6.01
0.333 6.82 1.083 26.38 1.833 8.98 2.58 5.59
0.417 8.45 1.167 26.38 1.917 7.93 2.67 5.59
0.500 8.45 1.250 16.63 2.000 7.93 2.75 5.23
0.583 11.56 1.333 16.63 2.083 7.14 2.83 5.23
0.667 11.56 1.417 12.68 2.167 7.14 2.92 4.93
0.750 21.49 1.500 12.68 2.250 6.51 3.00 4.93

Max.Eff.Inten.(mm/hr)= 124.56 134.01
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.69 (11) 7.97 (11)
Unit Hyd. Peak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.13

*TOTALS*
PEAK FLOW (cms)= 0.00 0.06 0.058 (111)
TIME TO PEAK (hrs)= 1.00 1.08 1.08
RUNOFF VOLUME (mm)= 48.52 25.42 25.64
TOTAL RAINFALL (mm)= 49.52 49.52 49.52
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RUNOFF COEFFICIENT = 0.98 0.51 0.52

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 74.0 Ia = Dep. Storage (Above)
(III) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: RESERVOIR (0282), OVERFLOW IS OFF, DF= 5.0 min, OUTFLOW STORAGE, INFLow: ID= 2 ( 0106), OUTFLOW: ID= 1 ( 0282)

PEAK FLOW REDUCTION (Qout/Qin)(%)= 86.58
TIME SHIFT OF PEAK FLOW (min)= 5.000
MAXIMUM STORAGE USED (ha.m.)= 0.0011

Table with columns: CALIB, STAMHVD ( 0245), Area (ha)= 0.58, IMPERVIOUS PERVIOUS (I), Surface Area, Dep. Storage, Average Slope, Length, Manning's n

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN

Table with columns: 0.250 6.82, 1.000 124.56, 1.750 5.00, 1.833 8.98, 2.50 6.01, 0.333 6.82, 1.083 26.38, 1.833 8.98, 2.50 5.59, 0.417 8.45, 1.167 26.38, 1.917 7.93, 2.67 5.59, 0.580 8.45, 1.250 16.63, 2.000 7.93, 2.75 5.23, 0.583 11.56, 1.333 16.63, 2.083 7.14, 2.83 5.23, 0.667 11.56, 1.417 12.68, 2.167 7.14, 2.92 4.93, 0.750 21.49, 1.500 12.68, 2.250 6.51, 3.00 4.93

Max.Eff.Inten.(mm/hr)= 124.56 134.01
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.76 (I) 8.04 (III)
Unit Hyd. Tpeak (mm)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.13
PEAK FLOW (cms)= 0.00 0.07
TIME TO PEAK (hrs)= 1.00 1.08
RUNDIFF VOLUME (mm)= 48.52 25.42 25.64
TOTAL RAINFALL (mm)= 49.52 49.52 49.52
RUNDIFF COEFFICIENT = 0.98 0.51 0.52

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 74.0 Ia = Dep. Storage (Above)
(III) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: CALIB, ADD HYD ( 0246), AREA QPEAK TPEAK R.V., ID= 1 DT= 5.0 min

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STAMHVD ( 0302), Area (ha)= 0.78, CURVE NUMBER (CN)= 78.5, ID= 1 DT= 5.0 min

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN

Unit Hyd qpeak (cms)= 0.349
PEAK FLOW (cms)= 0.043 (I)
TIME TO PEAK (hrs)= 1.167
RUNDIFF VOLUME (mm)= 17.716
TOTAL RAINFALL (mm)= 49.520
RUNDIFF COEFFICIENT = 0.528

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: CALIB, STAMHVD ( 0019), Area (ha)= 0.50, ID= 1 DT= 5.0 min

Table with columns: IMPERVIOUS PERVIOUS (I), Surface Area, Dep. Storage, Average Slope, Length, Manning's n

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN

Table with columns: 0.583 11.56, 1.333 16.63, 2.083 7.14, 2.83 5.23, 0.667 11.56, 1.417 12.68, 2.167 7.14, 2.92 4.93, 0.750 21.49, 1.500 12.68, 2.250 6.51, 3.00 4.93

Max.Eff.Inten.(mm/hr)= 124.56 134.01
over (min)= 1.68 (II) 7.96 (III)
Storage Coeff. (min)= 5.00 10.00
Unit Hyd. Tpeak (mm)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.13
PEAK FLOW (cms)= 0.00 0.06
TIME TO PEAK (hrs)= 1.00 1.08
RUNDIFF VOLUME (mm)= 48.52 25.42 25.64
TOTAL RAINFALL (mm)= 49.52 49.52 49.52
RUNDIFF COEFFICIENT = 0.98 0.51 0.52

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 74.0 Ia = Dep. Storage (Above)
(III) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD ( 0248), AREA QPEAK TPEAK R.V., ID= 1 DT= 5.0 min

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STAMHVD ( 0276), Area (ha)= 1.28, IMPERVIOUS PERVIOUS (I), Surface Area, Dep. Storage, Average Slope, Length, Manning's n

Volume Reduction Rate [(Rvin-Rvout)/Rvin] (%) = 33.42
Time to reach Max Ponding storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.) = 167.31
Volume of Max. Water Storage (cu.m.) = 0.00
Maximum Surface Ponding and Mulch Vol(cu.m.) = 154.59
Maximum Engineered Soil Volume (cu.m.) = 12.80

\*\*\*\*\* After simulation, water volume is not zero.

Junction Command(0277)

Table with columns: AREA QPEAK TPEAK R.V., INFLow: ID= 3 ( 0278), OUTFLOW: ID= 2 ( 0277)

Table with columns: CALIB, STAMHVD ( 0313), Area (ha)= 4.05, IMPERVIOUS PERVIOUS (I), Surface Area, Dep. Storage, Average Slope, Length, Manning's n

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Max.Eff.Inten.(mm/hr)= 124.56 134.01
over (min)= 2.23 (II) 6.04 (III)
Storage Coeff. (min)= 5.00 10.00
Unit Hyd. Tpeak (mm)= 5.00 10.00
Unit Hyd. peak (cms)= 0.30 0.14
PEAK FLOW (cms)= 0.31 0.02
TIME TO PEAK (hrs)= 1.00 1.08
RUNDIFF VOLUME (mm)= 48.52 14.82 38.41
TOTAL RAINFALL (mm)= 49.52 49.52 49.52
RUNDIFF COEFFICIENT = 0.98 0.30 0.78

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 74.0 Ia = Dep. Storage (Above)
(III) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: CALIB, DUHVD ( 0279), Inlet Cap.= 0.270, # of Inlets= 11, AREA QPEAK TPEAK R.V., ID= 1 DT= 5.0 min

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STAMHVD ( 0279), Area (ha)= 1.28, IMPERVIOUS PERVIOUS (I), Surface Area, Dep. Storage, Average Slope, Length, Manning's n

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Max. Ponding Storage(cu.m.)= 149.95
Depth (m)= 0.30 Porosity = 0.40
Maximum Mulch layer Volume (cu.m.)= 4.64
ENGINEERED SOIL LAYER:
Soil moisture = 50.00 Depth (m)= 0.67
Width (m)= 58.00 Length (m)= 2.00
Porosity = 0.47 Infiltration (m/hr)= 0.0000
Maximum Engineered soil layer Volume(cu.m.)= 36.30

Table with columns: STORAGE LAYER, STAGE AREA, STAGE AREA, STAGE AREA

MULCH LAYER:
Depth (m)= 0.10 Porosity = 0.40
Soil moisture = 50.00
Maximum Mulch layer Volume (cu.m.)= 4.64

ENGINEERED SOIL LAYER:
Soil moisture = 50.00 Depth (m)= 0.67
Width (m)= 58.00 Length (m)= 2.00
Porosity = 0.47 Infiltration (m/hr)= 0.0000
Maximum Engineered soil layer Volume(cu.m.)= 36.30

STORAGE LAYER:
Depth (m)= 1.50 Porosity = 0.40
Seepage (m/hr)= 0.0010
Maximum Storage layer Volume(cu.m.)= 69.60

TOTAL AVAILABLE STORAGE:
TOTAL STORAGE+Surface Ponding + Mulch Layer + Engineered soil
+Storage Layer(cu.m.)= 260.48

NATIVE SOIL LAYER:
Infiltration (m/hr)= 0.0060

Max.Eff.Inten.(mm/hr)= 124.56 24.35
over (min)= 5.00 20.00
Storage Coeff. (min)= 3.15 (II) 15.57 (II)
Unit Hyd. Tpeak (mm)= 5.00 20.00
Unit Hyd. peak (cms)= 0.27 0.07
PEAK FLOW (cms)= 0.81 0.06
TIME TO PEAK (hrs)= 1.00 1.25
RUNDIFF VOLUME (mm)= 47.52 14.82 31.16
TOTAL RAINFALL (mm)= 49.52 49.52 49.52
RUNDIFF COEFFICIENT = 0.96 0.30 0.63

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 74.0 Ia = Dep. Storage (Above)
(III) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: CALIB, STAMHVD ( 0314), Area (ha)= 1.43, IMPERVIOUS PERVIOUS (I), Surface Area, Dep. Storage, Average Slope, Length, Manning's n

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN

Max.Eff.Inten.(mm/hr)= 124.56 31.27
over (min)= 5.00 15.00
Storage Coeff. (min)= 2.84 (II) 12.30 (II)
Unit Hyd. Tpeak (mm)= 5.00 15.00
Unit Hyd. peak (cms)= 0.28 0.09
PEAK FLOW (cms)= 0.24 0.03
TIME TO PEAK (hrs)= 1.00 1.17
RUNDIFF VOLUME (mm)= 47.52 14.82 31.16
TOTAL RAINFALL (mm)= 49.52 49.52 49.52
RUNDIFF COEFFICIENT = 0.96 0.30 0.63

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 74.0 Ia = Dep. Storage (Above)
(III) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: CALIB, STAMHVD ( 0315), Area (ha)= 14.21, IMPERVIOUS PERVIOUS (I), Surface Area, Dep. Storage, Average Slope, Length, Manning's n

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN

```

hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.79 0.833 21.49 1.583 10.45 2.33 6.51
0.167 5.79 0.833 21.49 1.583 10.45 2.42 6.01
0.250 6.82 1.000 124.56 1.750 8.98 2.50 6.01
0.333 6.82 1.083 26.38 1.833 8.98 2.58 5.59
0.417 8.45 1.167 26.38 1.917 7.98 2.67 5.59
0.500 8.45 1.250 16.63 2.000 7.93 2.75 5.23
0.583 11.56 1.333 16.63 2.083 7.14 2.83 5.23
0.667 11.56 1.417 12.68 2.167 7.14 2.92 4.93
0.750 21.49 1.500 12.68 2.250 6.51 3.00 4.93

Max.Eff.Inten.(mm/hr)= 124.56 *****
over (min) 5.00 10.00
Storage Coeff. (min)= 4.59 (11) 9.11 (11)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.23 0.12

PEAK FLOW (cms)= 3.16 0.24 *TOTALS*
TIME TO PEAK (hrs)= 1.00 1.08 3.332 (111)
RUNOFF VOLUME (mm)= 48.52 14.82 38.75
TOTAL RAINFALL (mm)= 49.52 49.52 49.52
RUNOFF COEFFICIENT = 0.98 0.30 0.78

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
CN = 74.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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SURFACE PONDING LAYER:
Max. Ponding Storage(cu.m.)= 149.95

STAGE AREA STAGE AREA
(m) (m2) (m) (m2)
0.000 100.000 1.500 100.000
0.500 100.000 0.000 0.000
1.000 100.000 0.000 0.000

MULCH LAYER:
Depth (m)= 0.10 Porosity = 0.40
Maximum Mulch Layer Volume (cu.m.)= 62.40

ENGINEERED SOIL LAYER:
Soil moisture = 0.30 Depth (m)= 0.67
Width (m)= 780.00 Length (m)= 2.00
Porosity = 0.47 Infiltration (m/hr) = 0.0060
Maximum Engineered Soil Layer Volume(cu.m.)= 488.11

STORAGE LAYER:
Depth (m)= 1.50 Porosity = 0.40
Seepage (m/hr) = 0.0010
Maximum Storage Layer Volume(cu.m.)= 936.00

TOTAL AVAILABLE STORAGE:
TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil
+Storage Layer(cu.m.)= 1636.46

NATIVE SOIL LAYER:
Infiltration (m/hr) = 0.0060

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW:ID= 2 13.91 2.948 1.00 38.75
OVERFLOW:ID= 3 13.91 2.945 1.00 36.99

Volume Reduction Rate((Rv-in-Rvout)/RvIn) (%)= 4.53
Time to reach Max Ponding Storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 384.22
Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 212.35
Maximum Engineered Soil Volume (cu.m.)= 176.07
**** After simulation, water volume is not zero.

Junction Command(0317)

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AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 3( 0318) 13.91 2.945 1.00 36.99
OUTFLOW: ID= 2( 0317) 13.91 2.95 1.00 36.99

ADD HYD ( 0316)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 ( 0311): 0.30 0.384 1.00 38.75
+ ID2= 2 ( 0317): 13.91 2.945 1.00 36.99
ID = 3 ( 0316): 14.21 3.329 1.00 37.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD ( 0323) Area (ha)= 0.59
ID= 1 Dfs= 5.0 min Total Dfs(%)= 66.00 Dfs. Conn.(%)= 25.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.39 0.20
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 62.78 60.00
Manning's n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYDROGRAPH
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 5.79 0.833 21.49 1.583 10.45 2.33 6.51
0.167 5.79 0.917 124.56 1.667 10.45 2.42 6.01
0.250 6.82 1.000 124.56 1.750 8.98 2.50 6.01
0.333 6.82 1.083 26.38 1.833 8.98 2.58 5.59
0.417 8.45 1.167 26.38 1.917 7.93 2.67 5.59
0.500 8.45 1.250 16.63 2.000 7.93 2.75 5.23
0.583 11.56 1.333 16.63 2.083 7.14 2.83 5.23
0.667 11.56 1.417 12.68 2.167 7.14 2.92 4.93
0.750 21.49 1.500 12.68 2.250 6.51 3.00 4.93

Max.Eff.Inten.(mm/hr)= 124.56 134.60
over (min) 5.00 10.00
Storage Coeff. (min)= 1.77 (11) 0.84 (11)
Unit Hyd. Tpeak (min)= 5.00 10.00

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Unit Hyd. peak (cms)= 0.32 0.13 *TOTALS*
PEAK FLOW (cms)= 0.05 0.05 0.093 (111)
TIME TO PEAK (hrs)= 1.00 1.08 1.00
RUNOFF VOLUME (mm)= 48.52 25.46 31.21
TOTAL RAINFALL (mm)= 49.52 49.52 49.52
RUNOFF COEFFICIENT = 0.98 0.51 0.63

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
CN = 74.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

DUHYD ( 0331)
Inlet Cap.= 0.112
Ref Inlets= 1
Total(cms)= 0.11
TOTAL HYD.(ID= 1): 0.59 0.09 1.00 31.21
MAJOR SYS.(ID= 2): 0.00 0.00 0.00 0.00
MINOR SYS.(ID= 3): 0.59 0.09 1.00 31.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR( 0319) OUTFLOW: OFF, UNDERDRAIN: ON
ID= 2--> OUT= 3
Dfs= 5.0 min

SURFACE PONDING LAYER:
Max. Ponding Storage(cu.m.)= 149.95

STAGE AREA STAGE AREA
(m) (m2) (m) (m2)
0.000 100.000 1.500 100.000
0.500 100.000 0.000 0.000
1.000 100.000 0.000 0.000

MULCH LAYER:
Depth (m)= 0.00 Porosity = 0.40
Maximum Mulch Layer Volume (cu.m.)= 0.00

ENGINEERED SOIL LAYER:
Soil moisture = 0.30 Depth (m)= 0.72
Width (m)= 1.00 Length (m)=260.00
Porosity = 0.47 Infiltration (m/hr) = 0.0060
Maximum Engineered Soil Layer Volume(cu.m.)= 242.09

DEPTH DISCHARGE DEPTH DISCHARGE
(m) (cms) (m) (cms)
0.000 0.000 0.550 0.030
0.279 0.010 0.762 0.040
0.406 0.020 1.300 0.050

STORAGE LAYER:
Depth (m)= 1.00 Porosity = 0.40
Seepage (m/hr) = 0.0010
Maximum Storage Layer Volume(cu.m.)= 288.00

TOTAL AVAILABLE STORAGE:
TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil
+Storage Layer(cu.m.)= 680.04

NATIVE SOIL LAYER:
Infiltration (m/hr) = 0.0060

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW:ID= 2 0.59 0.093 1.00 31.21
OUTFLOW:ID= 1 0.45 0.089 0.08 14.47
OVERFLOW:ID= 3 0.14 0.008 2.33 14.47

Volume Reduction Rate((Rv-in-Rvout)/RvIn) (%)= 53.65
Time to reach Max Ponding Storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 169.06
Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 149.95
Maximum Engineered Soil Volume (cu.m.)= 69.06
**** After simulation, water volume is not zero.

Junction Command(0330)

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Junction Command(0322)
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 1( 0329) 0.45 0.01 0.08 14.47
OUTFLOW: ID= 2( 0332) 0.45 0.01 0.08 14.47

ADD HYD ( 3106)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 ( 0310): 0.14 0.008 2.33 14.47
+ ID2= 2 ( 0331): 0.00 0.000 0.00 0.00
ID = 3 ( 3106): 0.14 0.008 2.33 14.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 3106)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 ( 3106): 0.14 0.008 2.33 14.47
+ ID2= 2 ( 0332): 0.45 0.009 0.08 14.47
ID = 1 ( 3106): 0.59 0.012 2.33 14.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0312)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 ( 0277): 1.22 0.257 1.00 25.57
+ ID2= 2 ( 0310): 5.48 0.007 1.00 32.25
ID = 3 ( 0312): 6.70 1.344 1.00 31.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0312)

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3 + 2 = 1 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 3 ( 0312): 6.70 1.344 1.00 31.03
+ ID2= 2 ( 3106): 0.59 0.012 2.33 14.47
ID = 1 ( 0312): 7.29 1.350 1.00 29.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0312)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 ( 0312): 7.29 1.350 1.00 29.69
+ ID2= 2 ( 0316): 14.21 3.329 1.00 37.03
ID = 3 ( 0312): 21.50 4.680 1.00 34.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR( 0319) OUTFLOW IS OFF
ID= 2--> OUT= 1
Dfs= 5.0 min
OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 0.6610 0.9850
0.0400 0.1140 1.0900 1.3320
0.0760 0.3090 1.4760 1.5860
0.2810 0.6450 0.0000 0.0000

INFLOW : ID= 2 ( 0313) 21.500 4.680 1.00 34.54
OUTFLOW: ID= 1 ( 0319) 21.500 0.239 3.00 34.51

PEAK FLOW REDUCTION [out/In] (%) = 5.10
TIME SHIFT OF PEAK FLOW (min)=120.00
MAXIMUM STORAGE USED (ha.m.)= 0.5883

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3 + 2 = 1 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 3 ( 0312): 6.70 1.344 1.00 31.03
+ ID2= 2 ( 3106): 0.59 0.012 2.33 14.47
ID = 1 ( 0312): 7.29 1.350 1.00 29.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0312)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 ( 0312): 7.29 1.350 1.00 29.69
+ ID2= 2 ( 0316): 14.21 3.329 1.00 37.03
ID = 3 ( 0312): 21.50 4.680 1.00 34.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR( 0319) OUTFLOW IS OFF
ID= 2--> OUT= 1
Dfs= 5.0 min
OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 0.6610 0.9850
0.0400 0.1140 1.0900 1.3320
0.0760 0.3090 1.4760 1.5860
0.2810 0.6450 0.0000 0.0000

INFLOW : ID= 2 ( 0313) 21.500 4.680 1.00 34.54
OUTFLOW: ID= 1 ( 0319) 21.500 0.239 3.00 34.51

PEAK FLOW REDUCTION [out/In] (%) = 5.10
TIME SHIFT OF PEAK FLOW (min)=120.00
MAXIMUM STORAGE USED (ha.m.)= 0.5883

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SURFACE PONDING LAYER:
Max. Ponding Storage(cu.m.)= 149.95

STAGE AREA STAGE AREA
(m) (m2) (m) (m2)
0.000 100.000 1.500 100.000
0.500 100.000 0.000 0.000
1.000 100.000 0.000 0.000

MULCH LAYER:
Depth (m)= 0.00 Porosity = 0.40
Maximum Mulch Layer Volume (cu.m.)= 0.00

ENGINEERED SOIL LAYER:
Soil moisture = 0.30 Depth (m)= 0.72
Width (m)= 1.00 Length (m)=260.00
Porosity = 0.47 Infiltration (m/hr) = 0.0060
Maximum Engineered Soil Layer Volume(cu.m.)= 242.09

DEPTH DISCHARGE DEPTH DISCHARGE
(m) (cms) (m) (cms)
0.000 0.000 0.550 0.030
0.279 0.010 0.762 0.040
0.406 0.020 1.300 0.050

STORAGE LAYER:
Depth (m)= 1.00 Porosity = 0.40
Seepage (m/hr) = 0.0010
Maximum Storage Layer Volume(cu.m.)= 288.00

TOTAL AVAILABLE STORAGE:
TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil
+Storage Layer(cu.m.)= 680.04

NATIVE SOIL LAYER:
Infiltration (m/hr) = 0.0060

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW:ID= 2 0.59 0.093 1.00 31.21
OUTFLOW:ID= 1 0.45 0.089 0.08 14.47
OVERFLOW:ID= 3 0.14 0.008 2.33 14.47

Volume Reduction Rate((Rv-in-Rvout)/RvIn) (%)= 53.65
Time to reach Max Ponding Storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 169.06
Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 149.95
Maximum Engineered Soil Volume (cu.m.)= 69.06
**** After simulation, water volume is not zero.

Junction Command(0330)

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Junction Command(0322)
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 1( 0329) 0.45 0.01 0.08 14.47
OUTFLOW: ID= 2( 0332) 0.45 0.01 0.08 14.47

ADD HYD ( 3106)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 ( 0310): 0.14 0.008 2.33 14.47
+ ID2= 2 ( 0331): 0.00 0.000 0.00 0.00
ID = 3 ( 3106): 0.14 0.008 2.33 14.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 3106)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 ( 3106): 0.14 0.008 2.33 14.47
+ ID2= 2 ( 0332): 0.45 0.009 0.08 14.47
ID = 1 ( 3106): 0.59 0.012 2.33 14.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0312)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 ( 0277): 1.22 0.257 1.00 25.57
+ ID2= 2 ( 0310): 5.48 0.007 1.00 32.25
ID = 3 ( 0312): 6.70 1.344 1.00 31.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0312)

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\VisualInfo\6.2\vo2\vo1n.dat

Output filename: C:\Users\kskim\AppData\Local\Civil\GIS\4c9a807-2b3a-4142-8551-3404f768702\46c12-988-d14d-4c17-b46c-6482e30df544\scena

Summary filename: C:\Users\kskim\AppData\Local\Civil\GIS\4c9a807-2b3a-4142-8551-3404f768702\46c12-988-d14d-4c17-b46c-6482e30df544\scena

DATE: 08-28-2022 TIME: 02:37:55

USER:

COMMENTS:

CHICAGO STORM IDF curve parameters: A= 731.314  
POT(24) 58.14 mm I C= 0.699

used IN INTENSITY = A / (t + B)<sup>C</sup>

Duration of storm = 3.00 hrs

Storm time step = 10.00 min

Time to peak ratio = 0.33

| TIME | RAIN  | TIME | RAIN   | TIME | RAIN  | TIME | RAIN  |
|------|-------|------|--------|------|-------|------|-------|
| hrs  | mm/hr | hrs  | mm/hr  | hrs  | mm/hr | hrs  | mm/hr |
| 0.00 | 6.80  | 0.83 | 146.25 | 1.67 | 10.54 | 2.50 | 6.56  |
| 0.17 | 8.01  | 1.00 | 30.97  | 1.83 | 9.31  | 2.67 | 6.14  |
| 0.33 | 9.92  | 1.17 | 15.53  | 2.00 | 8.38  | 2.83 | 5.79  |
| 0.50 | 13.57 | 1.33 | 14.89  | 2.17 | 7.65  |      |       |
| 0.67 | 25.24 | 1.50 | 12.27  | 2.33 | 7.05  |      |       |

CALLB HSDVD ( 0185) Area (ha)= 1.66 Curve Number (CN)= 74.4  
ID= 1 Dt= 5.0 min Ia (mm)= 4.44 # of Linear Rvs.(N)= 3.00  
U.H. Tp(hrs)= 0.24

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 6.80  | 0.833 | 25.24  | 1.583 | 12.27 | 2.33 | 7.05  |
| 0.167 | 6.80  | 0.917 | 146.25 | 1.667 | 12.27 | 2.42 | 7.05  |
| 0.250 | 8.01  | 1.000 | 146.25 | 1.750 | 10.54 | 2.50 | 7.05  |
| 0.333 | 8.01  | 1.083 | 30.97  | 1.833 | 10.54 | 2.58 | 6.56  |
| 0.417 | 9.92  | 1.167 | 30.97  | 1.917 | 9.31  | 2.67 | 6.56  |
| 0.500 | 9.92  | 1.250 | 15.53  | 2.000 | 9.31  | 2.75 | 6.14  |
| 0.583 | 13.57 | 1.333 | 15.53  | 2.083 | 8.38  | 2.83 | 6.14  |
| 0.667 | 13.57 | 1.417 | 14.89  | 2.167 | 8.38  | 2.92 | 5.79  |
| 0.750 | 25.24 | 1.500 | 14.89  | 2.250 | 7.65  | 3.00 | 5.79  |

Unit Hyd Peak (cms) = 0.265

PEAK FLOW (cms) = 0.094 (1)

TIME TO PEAK (hrs) = 1.167

RUNOFF VOLUME (mm) = 20.420

TOTAL RAINFALL (mm) = 58.144

RUNOFF COEFFICIENT = 0.351

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

DUHDV ( 0182) Inlet Cap.= 1 Inlet= 1 Total(cms)= 0.11 Area QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

TOTAL INP.(ID= 1): 1.66 0.09 1.17 20.42

MAJOR SYS.(ID= 2): 0.00 0.00 0.00 0.00

MINOR SYS.(ID= 3): 1.66 0.09 1.17 20.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

bloretention( 0184) OUTFLOW: OFF, UNDERDRAIN: OFF

DT= 5.0 min

SURFACE PONDING LAYER:

| STAGE | AREA    | STAGE |         |
|-------|---------|-------|---------|
| (m)   | (m2)    | (m)   |         |
| 0.000 | 100.000 | 1.500 | 100.000 |
| 0.500 | 100.000 | 1.000 | 0.000   |
| 1.000 | 100.000 | 0.000 | 0.000   |

MULCH LAYER: Depth (m)= 0.10 Porosity = 0.40 Maximum Mulch layer Volume (cu.m.)= 21.60

ENGINEERED SOIL LAYER: Soil moisture = 0.30 Depth (m)= 0.72 Width (m)= 3.00 Length (m)= 3.00 Porosity = 0.47 Infiltration (m/hr) = 0.0000 Maximum Engineered Soil layer Volume(cu.m.)= 181.57

STORAGE LAYER: Depth (m)= 0.50 Porosity = 0.40 Seepage (m/hr) = 0.0010 Storage Layer Volume(cu.m.)= 108.00

TOTAL AVAILABLE STORAGE: TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered Soil +Storage Layer(cu.m.)= 461.12

NATIVE SOIL LAYER: Infiltration (m/hr) = 0.0060

AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

INFLOW:ID= 2 1.66 0.094 1.17 20.42

OVERFLOW:ID= 3 1.66 0.047 1.67 9.49

Volume Reduction Rate(RVIN-RVOUT)/RVIN (%)= 53.51

Time to reach Max Ponding storage (hr)= 0.00

Volume of water for drawdown in LID (cu.m.)= 233.91

Volume of Max. Water Storage (cu.m.)= 0.00

Maximum Surface Ponding And Mulch Vol(cu.m.)= 171.55

Maximum Engineered Soil Volume (cu.m.)= 64.32

\*\*\*\* After simulation, water volume is not zero.

Junction Command(0181)

AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

INFLOW: ID= 3( 0184) 1.66 0.05 1.67 9.49

OUTFLOW: ID= 2( 0181) 1.66 0.05 1.67 9.49

CALLB STAHMVD ( 0205) Area (ha)= 0.80 Inlet Imp(%)= 65.00 Dir. Conn.(%)= 65.00

IMPERVIOUS PERVIOUS (1) IMPERVIOUS PERVIOUS (1)

Surface Area (ha)= 0.56 0.30

Dep. Storage (mm)= 1.00 5.00

Average Slope (%)= 1.00 2.00

Length (m)= 75.87 40.00

Manning's n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 6.80  | 0.833 | 25.24  | 1.583 | 12.27 | 2.33 | 7.05  |
| 0.167 | 6.80  | 0.917 | 146.25 | 1.667 | 12.27 | 2.42 | 7.05  |
| 0.250 | 8.01  | 1.000 | 146.25 | 1.750 | 10.54 | 2.50 | 7.05  |
| 0.333 | 8.01  | 1.083 | 30.97  | 1.833 | 10.54 | 2.58 | 6.56  |
| 0.417 | 9.92  | 1.167 | 30.97  | 1.917 | 9.31  | 2.67 | 6.56  |
| 0.500 | 9.92  | 1.250 | 15.53  | 2.000 | 9.31  | 2.75 | 6.14  |
| 0.583 | 13.57 | 1.333 | 15.53  | 2.083 | 8.38  | 2.83 | 6.14  |
| 0.667 | 13.57 | 1.417 | 14.89  | 2.167 | 8.38  | 2.92 | 5.79  |
| 0.750 | 25.24 | 1.500 | 14.89  | 2.250 | 7.65  | 3.00 | 5.79  |

Max.Eff.Inten.(mm/hr)= 146.25 \*\*\*\*\*

over (min)= 5.00 10.00

Storage Coeff. (min)= 1.86 (11) 6.59 (11)

Unit Hyd. Peak (min)= 5.00 10.00

Unit Hyd. peak (cms)= 0.32 0.14

PEAK FLOW (cms)= 0.23 0.03 \*TOTALS\*

TIME TO PEAK (hrs)= 1.00 1.00 0.200 (111)

RUNOFF VOLUME (mm)= 57.14 19.84 44.08

TOTAL RAINFALL (mm)= 58.14 58.14 58.14

RUNOFF COEFFICIENT = 0.98 0.34 0.76

\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 74.0 Ia = Dep. Storage (Above)

(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

DUHDV ( 0263) Inlet Cap.= 0.174 Inlet= 1 Total(cms)= 0.21 Area QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

TOTAL INP.(ID= 1): 0.86 0.25 1.00 44.08

MAJOR SYS.(ID= 2): 1.08 0.07 1.00 44.08

MINOR SYS.(ID= 3): 0.78 0.17 0.92 44.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

bloretention( 0264) OUTFLOW: OFF, UNDERDRAIN: OFF

DT= 5.0 min

SURFACE PONDING LAYER:

| STAGE | AREA    | STAGE |         |
|-------|---------|-------|---------|
| (m)   | (m2)    | (m)   |         |
| 0.000 | 100.000 | 1.500 | 100.000 |
| 0.500 | 100.000 | 1.000 | 0.000   |
| 1.000 | 100.000 | 0.000 | 0.000   |

Max. Ponding Storage(cu.m.)= 149.95

AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

INFLOW:ID= 2 0.78 0.174 0.92 44.08

OVERFLOW:ID= 3 0.78 0.088 1.00 24.25

Volume Reduction Rate(RVIN-RVOUT)/RVIN (%)= 44.59

Time to reach Max Ponding storage (hr)= 0.00

Volume of water for drawdown in LID (cu.m.)= 161.97

Volume of Max. water Storage (cu.m.)= 0.00

Maximum Surface Ponding And Mulch Vol(cu.m.)= 151.15

Maximum Engineered Soil Volume (cu.m.)= 8.87

\*\*\*\* After simulation, water volume is not zero.

Junction Command(0262)

AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

INFLOW:ID= 3( 0264) 0.78 0.09 1.08 24.25

OUTFLOW: ID= 2( 0262) 0.78 0.09 1.08 24.25

ADD HYD ( 0243) 1 + 2 = 3 Area QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

DUHDV ( 0263) Inlet Cap.= 0.174 Inlet= 1 Total(cms)= 0.21 Area QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

TOTAL INP.(ID= 1): 0.86 0.25 1.00 44.08

MAJOR SYS.(ID= 2): 1.08 0.07 1.00 44.08

MINOR SYS.(ID= 3): 0.78 0.17 0.92 44.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

bloretention( 0264) OUTFLOW: OFF, UNDERDRAIN: OFF

DT= 5.0 min

SURFACE PONDING LAYER:

| STAGE | AREA    | STAGE |         |
|-------|---------|-------|---------|
| (m)   | (m2)    | (m)   |         |
| 0.000 | 100.000 | 1.500 | 100.000 |
| 0.500 | 100.000 | 1.000 | 0.000   |
| 1.000 | 100.000 | 0.000 | 0.000   |

Max. Ponding Storage(cu.m.)= 149.95

AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

INFLOW:ID= 2 0.78 0.09 1.08 24.25

OUTFLOW: ID= 2( 0262) 0.78 0.09 1.08 24.25

ADD HYD ( 0243) 1 + 2 = 3 Area QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

HA R N I G : HYDROGRAPH 0182 <ID= 2> 15.DR. HYDROGRAPH 0003 = HYDROGRAPH 0001

ID= 1 ( 0182): 1.66 0.047 1.67 9.49

+ID= 2 ( 0182): 0.00 0.000 0.00 0.00

ID= 3 ( 0243): 1.66 0.047 1.67 9.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0243) 1 + 2 + 3 Area QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

ID= 3 ( 0243): 1.66 0.047 1.67 9.49

+ID= 2 ( 0262): 0.78 0.088 1.08 24.25

ID= 1 ( 0243): 2.44 0.088 1.08 14.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0243) 1 + 2 + 3 Area QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

ID= 1 ( 0243): 2.44 0.088 1.08 14.20

+ID= 2 ( 0262): 0.08 0.074 1.00 44.08

ID= 3 ( 0243): 2.53 0.088 1.00 15.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR( 0188) OVERFLOW IS OFF

DT= 2 -> OUT= 1

OUTFLOW STORAGE (cms) (ha.m.) (ha.m.)

0.0000 0.0000 0.4800 0.0190

AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

INFLOW: ID= 2 ( 0243) 2.527 0.088 1.00 15.19

OUTFLOW: ID= 1 ( 0188) 2.527 0.073 1.17 15.19

PEAK FLOW REDUCTION (Qout/Qin)(%)= 83.09

TIME SHIFT OF PEAK FLOW (min)= 10.00

MAXIMUM STORAGE USED (ha.m.)= 0.0030

CALLB STAHMVD ( 0252) Area (ha)= 0.32 Inlet Imp(%)= 69.00 Dir. Conn.(%)= 30.00

IMPERVIOUS PERVIOUS (1) IMPERVIOUS PERVIOUS (1)

Surface Area (ha)= 0.22 0.10

Dep. Storage (mm)= 1.00 5.00

Average Slope (%)= 1.00 2.00

Length (m)= 45.84 40.00

Manning's n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 6.80  | 0.833 | 25.24  | 1.583 | 12.27 | 2.33 | 7.05  |
| 0.167 | 6.80  | 0.917 | 146.25 | 1.667 | 12.27 | 2.42 | 7.05  |
| 0.250 | 8.01  | 1.000 | 146.25 | 1.750 | 10.54 | 2.50 | 7.05  |
| 0.333 | 8.01  | 1.083 | 30.97  | 1.833 | 10.54 | 2.58 | 6.56  |
| 0.417 | 9.92  | 1.167 | 30.97  | 1.917 | 9.31  | 2.67 | 6.56  |
| 0.500 | 9.92  | 1.250 | 15.53  | 2.000 | 9.31  | 2.75 | 6.14  |
| 0.583 | 13.57 | 1.333 | 15.53  | 2.083 | 8.38  | 2.83 | 6.14  |
| 0.667 | 13.57 | 1.417 | 14.89  | 2.167 | 8.38  | 2.92 | 5.79  |
| 0.750 | 25.24 | 1.500 | 14.89  | 2.250 | 7.65  | 3.00 | 5.79  |

Max.Eff.Inten.(mm/hr)= 146.25 181.22

over (min)= 5.00 10.00

Storage Coeff. (min)= 1.37 (11) 6.94 (11)

Unit Hyd. Peak (min)= 5.00 10.00

Unit Hyd. peak (cms)= 0.33 0.14

PEAK FLOW (cms)= 0.04 0.03 \*TOTALS\*

TIME TO PEAK (hrs)= 1.00 1.00 0.068 (111)

RUNOFF VOLUME (mm)= 57.14 32.77 6.94 (11)

TOTAL RAINFALL (mm)= 58.14 58.14 58.14

RUNOFF COEFFICIENT = 0.98 0.56 0.69

\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 74.0 Ia = Dep. Storage (Above)

(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

DUHDV ( 0270) Inlet Cap.= 1 Inlet= 1 Total(cms)= 0.25 Area QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

ID= 1 ( 0188): 2.53 0.073 1.17 15.19

+ID= 2 ( 0252): 0.32 0.068 1.00 40.07

ID= 3 ( 0179): 2.84 0.112 1.08 17.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALLB STAHMVD ( 0106) Area (ha)= 0.51 Inlet Imp(%)= 55.00 Dir. Conn.(%)= 1.00

IMPERVIOUS PERVIOUS (1) IMPERVIOUS PERVIOUS (1)

Surface Area (ha)= 0.28 0.23

Dep. Storage (mm)= 1.00 5.00

Average Slope (%)= 1.00 2.00

Length (m)= 58.22 40.00

Manning's n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 6.80  | 0.833 | 25.24  | 1.583 | 12.27 | 2.33 | 7.05  |
| 0.167 | 6.80  | 0.917 | 146.25 | 1.667 | 12.27 | 2.42 | 7.05  |
| 0.250 | 8.01  | 1.000 | 146.25 | 1.750 | 10.54 | 2.50 | 7.05  |
| 0.333 | 8.01  | 1.083 | 30.97  | 1.833 | 10.54 | 2.58 | 6.56  |
| 0.417 | 9.92  | 1.167 | 30.97  | 1.917 | 9.31  | 2.67 | 6.56  |
| 0.500 | 9.92  | 1.250 | 15.53  | 2.000 | 9.31  | 2.75 | 6.14  |
| 0.583 | 13.57 | 1.333 | 15.53  | 2.083 | 8.38  | 2.83 | 6.14  |
| 0.667 | 13.57 | 1.417 | 14.89  | 2.167 | 8.38  | 2.92 | 5.79  |
| 0.750 | 25.24 | 1.500 | 14.89  | 2.250 | 7.65  | 3.00 | 5.79  |

Max.Eff.Inten.(mm/hr)= 146.25 173.89

over (min)= 5.00 10.00

Storage Coeff. (min)= 1.59 (11) 7.24 (11)

Unit Hyd. Peak (min)= 5.00 10.00

Unit Hyd. peak (cms)= 0.33 0.14

PEAK FLOW (cms)= 0.00 0.08 \*TOTALS\*

TIME TO PEAK (hrs)= 1.00 1.08 0.078 (111)

RUNOFF VOLUME (mm)= 57.14 32.37 32.61

TOTAL RAINFALL (mm)= 58.14 58.14 58.14

ADD HYD ( 0179) 1 + 2 + 3 Area QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

ID= 1 ( 0188): 2.53 0.073 1.17 15.19

+ID= 2 ( 0252): 0.32 0.068 1.00 40.07

ID= 3 ( 0179): 2.84 0.112 1.08 17.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALLB STAHMVD ( 0106) Area (ha)= 0.51 Inlet Imp(%)= 55.00 Dir. Conn.(%)= 1.00

IMPERVIOUS PERVIOUS (1) IMPERVIOUS PERVIOUS (1)

Surface Area (ha)= 0.28 0.23

Dep. Storage (mm)= 1.00 5.00

Average Slope (%)= 1.00 2.00

Length (m)= 58.22 40.00

Manning's n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

| TIME  | RAIN  | TIME  | RAIN   | TIME  | RAIN  | TIME | RAIN  |
|-------|-------|-------|--------|-------|-------|------|-------|
| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
| 0.083 | 6.80  | 0.833 | 25.24  | 1.583 | 12.27 | 2.33 | 7.05  |
| 0.167 | 6.80  | 0.917 | 146.25 | 1.667 | 12.27 | 2.42 | 7.05  |
| 0.250 | 8.01  | 1.000 | 146.25 | 1.750 | 10.54 | 2.50 | 7.05  |
| 0.333 | 8.01  | 1.083 | 30.97  | 1.833 | 10.54 | 2.58 | 6.56  |
| 0.417 | 9.92  | 1.167 | 30.97  | 1.917 | 9.31  | 2.67 | 6.56  |
| 0.500 | 9.92  | 1.250 | 15.53  | 2.000 | 9.31  | 2.75 | 6.14  |
| 0.583 | 13.57 | 1.333 | 15.53  | 2.083 | 8.38  | 2.83 | 6.14  |
| 0.667 | 13.57 | 1.417 | 14.89  | 2.167 | 8.38  | 2.92 | 5.79  |
| 0.750 | 25.24 | 1.500 | 14.89  | 2.250 | 7.65  | 3.00 | 5.79  |

Max.Eff.Inten.(mm/hr)= 146.25 173.89

over (min)= 5.00 10.00

Storage Coeff. (min)= 1.59 (11) 7.24 (11)

Unit Hyd. Peak (min)= 5.00 10.00

Unit Hyd. peak (cms)= 0.33 0.14

PEAK FLOW (cms)= 0.00 0.08 \*TOTALS\*

TIME TO PEAK (hrs)= 1.00 1.08 0.078 (111)

RUNOFF VOLUME (mm)= 57.14 32.37 32.61

TOTAL RAINFALL (mm)= 58.14 58.14 58.14



| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
|-------|-------|-------|--------|-------|-------|------|-------|
| 0.083 | 6.80  | 0.833 | 25.24  | 1.583 | 12.27 | 2.33 | 7.65  |
| 0.167 | 6.80  | 0.917 | 146.25 | 1.667 | 12.27 | 2.42 | 7.45  |
| 0.250 | 8.81  | 1.000 | 146.25 | 1.750 | 10.54 | 2.50 | 7.05  |
| 0.333 | 8.81  | 1.083 | 30.97  | 1.833 | 10.54 | 2.58 | 6.56  |
| 0.417 | 9.92  | 1.167 | 30.97  | 1.917 | 9.31  | 2.67 | 6.56  |
| 0.500 | 9.92  | 1.250 | 19.53  | 2.000 | 9.31  | 2.75 | 6.14  |
| 0.583 | 13.57 | 1.333 | 19.53  | 2.083 | 8.38  | 2.83 | 6.14  |
| 0.667 | 13.57 | 1.417 | 14.89  | 2.167 | 8.38  | 2.92 | 5.79  |
| 0.750 | 25.24 | 1.500 | 14.89  | 2.250 | 7.65  | 3.00 | 5.79  |

Max.Eff.Inten.(mm/hr)= 146.25 over (min)= 5.00 (hr)= 10.00  
Storage Coeff. (min)= 4.31 (hr)= 8.54 (ii)  
Unit Hyd. Tpeak (min)= 5.00 (hr)= 10.00  
Unit Hyd. Peak (cms)= 0.23 0.12

PEAK FLOW (cms)= 3.76 0.33  
TIME TO PEAK (hrs)= 1.00 1.08  
RUNOFF VOLUME (mm)= 57.14 19.84  
TOTAL RAINFALL (mm)= 58.14 58.14  
RUNOFF COEFFICIENT = 0.96 0.80

\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN = 74.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| DUHD ( 0311)                             |  |  |  |  |
|--|--|--|--|--|
| Inlet Cap.= 2.948                        |  |  |  |  |
| #of Inlets= 1                            |  |  |  |  |
| Total(cms)= 2.01                         |  |  |  |  |
| Total Hyd.(ID= 1): 14.21 4.01 1.00 46.32 |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.79 1.07 1.00 46.32  |  |  |  |  |
| MINOR SYS.(ID= 3): 13.42 2.95 0.92 46.32 |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| Retention( 0318)                        |  |  |  |  |
|---|--|--|--|--|
| ID= 2 -> OUT= 3                         |  |  |  |  |
| Dt= 5.0 min                             |  |  |  |  |
| Outflow: OFF, UNDERDRAIN: OFF           |  |  |  |  |
| # of Inlets= 1                          |  |  |  |  |
| Total(cms)= 0.1                         |  |  |  |  |
| Total Hyd.(ID= 1): 0.59 0.12 1.00 38.59 |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.00 0.01 1.00 38.59 |  |  |  |  |
| MINOR SYS.(ID= 3): 0.59 0.11 1.00 38.59 |  |  |  |  |

\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN = 74.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| DUHD ( 0331)                            |  |  |  |  |
|---|--|--|--|--|
| Inlet Cap.= 0.112                       |  |  |  |  |
| #of Inlets= 1                           |  |  |  |  |
| Total(cms)= 0.1                         |  |  |  |  |
| Total Hyd.(ID= 1): 0.59 0.12 1.00 38.59 |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.00 0.01 1.00 38.59 |  |  |  |  |
| MINOR SYS.(ID= 3): 0.59 0.11 1.00 38.59 |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| Reservoir( 0319)                        |  |  |  |  |
|---|--|--|--|--|
| ID= 2 -> OUT= 1                         |  |  |  |  |
| Dt= 5.0 min                             |  |  |  |  |
| Outflow: OFF, UNDERDRAIN: ON            |  |  |  |  |
| # of Inlets= 1                          |  |  |  |  |
| Total(cms)= 0.1                         |  |  |  |  |
| Total Hyd.(ID= 1): 0.59 0.12 1.00 38.59 |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.00 0.01 1.00 38.59 |  |  |  |  |
| MINOR SYS.(ID= 3): 0.59 0.11 1.00 38.59 |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD ( 0312)                          |  |  |  |  |
|--|--|--|--|--|
| 1 + 2 = 3                                |  |  |  |  |
| Total(cms)= 2.01                         |  |  |  |  |
| Total Hyd.(ID= 1): 14.21 4.01 1.00 46.32 |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.79 1.07 1.00 46.32  |  |  |  |  |
| MINOR SYS.(ID= 3): 13.42 2.95 0.92 46.32 |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| RESERVOIR ( 0319)                       |  |  |  |  |
|---|--|--|--|--|
| ID= 2 -> OUT= 1                         |  |  |  |  |
| Dt= 5.0 min                             |  |  |  |  |
| Outflow: OFF, UNDERDRAIN: OFF           |  |  |  |  |
| # of Inlets= 1                          |  |  |  |  |
| Total(cms)= 0.1                         |  |  |  |  |
| Total Hyd.(ID= 1): 0.59 0.12 1.00 38.59 |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.00 0.01 1.00 38.59 |  |  |  |  |
| MINOR SYS.(ID= 3): 0.59 0.11 1.00 38.59 |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| SURFACE PONDING LAYER:              |                   |       |                   |       |       |       |
|-------------------------------------|-------------------|-------|-------------------|-------|-------|-------|
| Max. Ponding Storage(cu.m.)= 149.95 |                   |       |                   |       |       |       |
| STAGE                               | AREA              | STAGE | AREA              | R.V.  | TPEAK | R.V.  |
| (m)                                 | (m <sup>2</sup> ) | (m)   | (m <sup>2</sup> ) | (mm)  | (hrs) | (mm)  |
| 0.000                               | 100.000           | 1.500 | 100.000           | 13.42 | 2.95  | 46.50 |
| 0.500                               | 100.000           | 0.000 | 0.000             | 14.21 | 4.01  | 46.32 |
| 1.000                               | 100.000           | 0.000 | 0.000             | 14.21 | 4.01  | 46.32 |

| MULCH LAYER:   |     |          |     |              |        |              |
|--|-----|----------|-----|--------------|--------|--------------|
| Depth  | (m) | Porosity | (%) | Infiltration | (m/hr) | Volume       |
| Maximum Mulch Layer Volume (cu.m.)= 62.40            |     |          |     |              |        |              |
| ENGINEERED SOIL LAYER:                               |     |          |     |              |        |              |
| Soil moisture  | (%) | Depth    | (m) | Length       | (m)    | Infiltration |
| Maximum Engineered Soil Layer Volume (cu.m.)= 488.11 |     |          |     |              |        |              |

| STORAGE LAYER:  |     |          |     |         |        |        |
|---|-----|----------|-----|---------|--------|--------|
| Depth   | (m) | Porosity | (%) | Seepage | (m/hr) | Volume |
| Maximum Storage Layer Volume(cu.m.)= 936.00                   |     |          |     |         |        |        |
| TOTAL AVAILABLE STORAGE:                                      |     |          |     |         |        |        |
| TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil |     |          |     |         |        |        |
| +Storage Layer(cu.m.)= 1636.46                                |     |          |     |         |        |        |

| NATIVE SOIL LAYER:          |       |       |       |       |        |          |
|-----------------------------|-------|-------|-------|-------|--------|----------|
| Infiltration (m/hr)= 0.0060 |       |       |       |       |        |          |
| AREA                        | QPEAK | TPEAK | R.V.  | INFLW | OUTFLW | OVERFLOW |
| (ha)                        | (cms) | (hrs) | (mm)  | ID= 2 | ID= 3  | ID= 3    |
| 13.42                       | 2.948 | 0.92  | 46.32 | 0.30  | 0.01   | 0.08     |
| 13.42                       | 2.945 | 0.92  | 44.50 | 0.30  | 0.01   | 0.08     |

Volume Reduction Rate(R(in-R(out))/R(in))= 3.94  
Time to reach Max Ponding Storage (hr)= 0.00  
Volume of water for drawdown in IID (cu.m.)= 385.89  
Volume of Max. Water Storage (cu.m.)= 0.00  
Maximum Surface Ponding and Mulch Vol(cu.m.)= 212.35  
Maximum Engineered Soil Volume (cu.m.)= 176.20  
\*\*\*\* After simulation, water volume is not zero.

| Junction Command(0317)                   |  |  |  |  |  |  |
|--|--|--|--|--|--|--|
| DUHD ( 0311)                             |  |  |  |  |  |  |
| Inlet Cap.= 2.948                        |  |  |  |  |  |  |
| #of Inlets= 1                            |  |  |  |  |  |  |
| Total(cms)= 2.01                         |  |  |  |  |  |  |
| Total Hyd.(ID= 1): 14.21 4.01 1.00 46.32 |  |  |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.79 1.07 1.00 46.32  |  |  |  |  |  |  |
| MINOR SYS.(ID= 3): 13.42 2.95 0.92 46.32 |  |  |  |  |  |  |

| Soil moisture = 0.30 Depth (m)= 0.72                |           |       |           |       |       |       |       |
|---|-----------|-------|-----------|-------|-------|-------|-------|
| Width (m)= 1.00 Length (m)=240.00                   |           |       |           |       |       |       |       |
| Porosity = 0.47 Infiltration (m/hr)= 0.0060         |           |       |           |       |       |       |       |
| Maximum Engineered Soil Layer Volume(cu.m.)= 242.09 |           |       |           |       |       |       |       |
| DEPTH   | DISCHARGE | DEPTH | DISCHARGE | (m)   | (cms) | (m)   | (cms) |
| 0.000   | 0.000     | 0.550 | 0.030     | 0.279 | 0.010 | 0.762 | 0.040 |
| 0.406   | 0.020     | 1.300 | 0.050     |       |       |       |       |

| STORAGE LAYER:  |     |          |     |         |        |        |
|---|-----|----------|-----|---------|--------|--------|
| Depth   | (m) | Porosity | (%) | Seepage | (m/hr) | Volume |
| Maximum Storage Layer Volume(cu.m.)= 288.00                   |     |          |     |         |        |        |
| TOTAL AVAILABLE STORAGE:                                      |     |          |     |         |        |        |
| TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil |     |          |     |         |        |        |
| +Storage Layer(cu.m.)= 680.04                                 |     |          |     |         |        |        |

| NATIVE SOIL LAYER:          |       |       |       |       |        |          |
|-----------------------------|-------|-------|-------|-------|--------|----------|
| Infiltration (m/hr)= 0.0060 |       |       |       |       |        |          |
| AREA                        | QPEAK | TPEAK | R.V.  | INFLW | OUTFLW | OVERFLOW |
| (ha)                        | (cms) | (hrs) | (mm)  | ID= 2 | ID= 1  | ID= 1    |
| 0.59                        | 0.112 | 1.00  | 38.59 | 0.30  | 0.009  | 0.08     |
| 0.59                        | 0.11  | 1.00  | 38.59 | 0.29  | 0.019  | 1.58     |

| Junction Command(0330)                  |  |  |  |  |  |  |
|---|--|--|--|--|--|--|
| DUHD ( 0331)                            |  |  |  |  |  |  |
| Inlet Cap.= 0.112                       |  |  |  |  |  |  |
| #of Inlets= 1                           |  |  |  |  |  |  |
| Total(cms)= 0.1                         |  |  |  |  |  |  |
| Total Hyd.(ID= 1): 0.59 0.12 1.00 38.59 |  |  |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.00 0.01 1.00 38.59 |  |  |  |  |  |  |
| MINOR SYS.(ID= 3): 0.59 0.11 1.00 38.59 |  |  |  |  |  |  |

| INFLW : ID= 3( 0329) 0.30 0.01 0.08 21.71            |  |  |  |  |  |  |
|--|--|--|--|--|--|--|
| OUTFLW: ID= 2( 0332) 0.30 0.01 0.08 21.71            |  |  |  |  |  |  |
| OVERFLOW: ID= 3 0.29 0.019 1.58 21.71                |  |  |  |  |  |  |
| Volume Reduction Rate(R(in-R(out))/R(in))= 43.73     |  |  |  |  |  |  |
| Time to reach Max Ponding Storage (hr)= 0.00         |  |  |  |  |  |  |
| Volume of water for drawdown in IID (cu.m.)= 169.10  |  |  |  |  |  |  |
| Volume of Max. Water Storage (cu.m.)= 0.00           |  |  |  |  |  |  |
| Maximum Surface Ponding and Mulch Vol(cu.m.)= 149.95 |  |  |  |  |  |  |
| Maximum Engineered Soil Volume (cu.m.)= 69.66        |  |  |  |  |  |  |
| **** After simulation, water volume is not zero.     |  |  |  |  |  |  |

| ADD HYD ( 0312)                          |  |  |  |  |  |  |
|--|--|--|--|--|--|--|
| 1 + 2 = 3                                |  |  |  |  |  |  |
| Total(cms)= 2.01                         |  |  |  |  |  |  |
| Total Hyd.(ID= 1): 14.21 4.01 1.00 46.32 |  |  |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.79 1.07 1.00 46.32  |  |  |  |  |  |  |
| MINOR SYS.(ID= 3): 13.42 2.95 0.92 46.32 |  |  |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD ( 0316)                          |  |  |  |  |  |  |
|--|--|--|--|--|--|--|
| 1 + 2 = 3                                |  |  |  |  |  |  |
| Total(cms)= 2.01                         |  |  |  |  |  |  |
| Total Hyd.(ID= 1): 14.21 4.01 1.00 46.32 |  |  |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.79 1.07 1.00 46.32  |  |  |  |  |  |  |
| MINOR SYS.(ID= 3): 13.42 2.95 0.92 46.50 |  |  |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB ( 0327)                            |      |      |      |              |      |               |     |
|--|------|------|------|--------------|------|---------------|-----|
| STARDHYD ( 0327)                         |      |      |      |              |      |               |     |
| ID= 1 Dt= 5.0 min                        |      |      |      |              |      |               |     |
| Total Hyd(%)= 66.00 Dir. Conn.(%)= 25.00 |      |      |      |              |      |               |     |
| IMPERVIOUS PERVIOUS (i)                  |      |      |      |              |      |               |     |
| Surface Area                             | (ha) | Area | (ha) | Dep. Storage | (mm) | Average Slope | (%) |
|  |      |      |      |              |      |               |     |

| Time RAIN   TIME RAIN   TIME RAIN   TIME RAIN       |  |  |  |  |  |  |
|---|--|--|--|--|--|--|
| hrs mm/hr   hrs mm/hr   hrs mm/hr   hrs mm/hr       |  |  |  |  |  |  |
| 0.083 6.80   0.833 25.24   1.583 12.27   2.33 7.65  |  |  |  |  |  |  |
| 0.167 6.80   0.917 146.25   1.667 12.27   2.42 7.05 |  |  |  |  |  |  |
| 0.250 8.81   1.000 146.25   1.750 10.54   2.50 7.05 |  |  |  |  |  |  |
| 0.333 8.81   1.083 30.97   1.833 10.54   2.58 6.56  |  |  |  |  |  |  |
| 0.417 9.92   1.167 30.97   1.917 9.31   2.67 6.56   |  |  |  |  |  |  |
| 0.500 9.92   1.250 19.53   2.000 9.31   2.75 6.14   |  |  |  |  |  |  |
| 0.583 13.57   1.333 19.53   2.083 8.38   2.83 6.14  |  |  |  |  |  |  |
| 0.667 13.57   1.417 14.89   2.167 8.38   2.92 5.79  |  |  |  |  |  |  |
| 0.750 25.24   1.500 14.89   2.250 7.65   3.00 5.79  |  |  |  |  |  |  |

Max.Eff.Inten.(mm/hr)= 146.25 over (min)= 5.00 (hr)= 10.00  
Storage Coeff. (min)= 1.66 (ii)= 7.31 (iii)  
Unit Hyd. Tpeak (min)= 5.00 (hr)= 10.00

| Junction Command(0322)                  |  |  |  |  |  |  |
|---|--|--|--|--|--|--|
| DUHD ( 0329)                            |  |  |  |  |  |  |
| Inlet Cap.= 0.112                       |  |  |  |  |  |  |
| #of Inlets= 1                           |  |  |  |  |  |  |
| Total(cms)= 0.1                         |  |  |  |  |  |  |
| Total Hyd.(ID= 1): 0.59 0.12 1.00 38.59 |  |  |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.00 0.01 1.00 38.59 |  |  |  |  |  |  |
| MINOR SYS.(ID= 3): 0.59 0.11 1.00 38.59 |  |  |  |  |  |  |

| ADD HYD ( 3106)                          |  |  |  |  |  |  |
|--|--|--|--|--|--|--|
| 1 + 2 = 3                                |  |  |  |  |  |  |
| Total(cms)= 2.01                         |  |  |  |  |  |  |
| Total Hyd.(ID= 1): 14.21 4.01 1.00 46.32 |  |  |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.79 1.07 1.00 46.32  |  |  |  |  |  |  |
| MINOR SYS.(ID= 3): 13.42 2.95 0.92 46.32 |  |  |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD ( 3106)                          |  |  |  |  |  |  |
|--|--|--|--|--|--|--|
| 3 + 2 = 1                                |  |  |  |  |  |  |
| Total(cms)= 2.01                         |  |  |  |  |  |  |
| Total Hyd.(ID= 1): 14.21 4.01 1.00 46.32 |  |  |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.79 1.07 1.00 46.32  |  |  |  |  |  |  |
| MINOR SYS.(ID= 3): 13.42 2.95 0.92 46.32 |  |  |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD ( 0312)                          |  |  |  |  |  |  |
|--|--|--|--|--|--|--|
| 1 + 2 = 3                                |  |  |  |  |  |  |
| Total(cms)= 2.01                         |  |  |  |  |  |  |
| Total Hyd.(ID= 1): 14.21 4.01 1.00 46.32 |  |  |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.79 1.07 1.00 46.32  |  |  |  |  |  |  |
| MINOR SYS.(ID= 3): 13.42 2.95 0.92 46.32 |  |  |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD ( 0312)                          |  |  |  |  |  |  |
|--|--|--|--|--|--|--|
| 1 + 2 = 3                                |  |  |  |  |  |  |
| Total(cms)= 2.01                         |  |  |  |  |  |  |
| Total Hyd.(ID= 1): 14.21 4.01 1.00 46.32 |  |  |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.79 1.07 1.00 46.32  |  |  |  |  |  |  |
| MINOR SYS.(ID= 3): 13.42 2.95 0.92 46.32 |  |  |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD ( 0312)                          |  |  |  |  |  |  |
|--|--|--|--|--|--|--|
| 1 + 2 = 3                                |  |  |  |  |  |  |
| Total(cms)= 2.01                         |  |  |  |  |  |  |
| Total Hyd.(ID= 1): 14.21 4.01 1.00 46.32 |  |  |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.79 1.07 1.00 46.32  |  |  |  |  |  |  |
| MINOR SYS.(ID= 3): 13.42 2.95 0.92 46.32 |  |  |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ID1= 3 ( 0312): 6.63 1.583 1.00 38.12   |  |  |  |  |  |  |
|---|--|--|--|--|--|--|
| + ID2= 2 ( 3106): 0.59 0.024 1.58 21.83 |  |  |  |  |  |  |
| ID = 1 ( 0312): 7.22 1.595 1.00 36.79   |  |  |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD ( 0312)                          |  |  |  |  |  |  |
|--|--|--|--|--|--|--|
| 1 + 2 = 3                                |  |  |  |  |  |  |
| Total(cms)= 2.01                         |  |  |  |  |  |  |
| Total Hyd.(ID= 1): 14.21 4.01 1.00 46.32 |  |  |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.79 1.07 1.00 46.32  |  |  |  |  |  |  |
| MINOR SYS.(ID= 3): 13.42 2.95 0.92 46.32 |  |  |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| RESERVOIR ( 0319)                       |  |  |  |  |  |  |
|---|--|--|--|--|--|--|
| ID= 2 -> OUT= 1                         |  |  |  |  |  |  |
| Dt= 5.0 min                             |  |  |  |  |  |  |
| Outflow: OFF, UNDERDRAIN: OFF           |  |  |  |  |  |  |
| # of Inlets= 1                          |  |  |  |  |  |  |
| Total(cms)= 0.1                         |  |  |  |  |  |  |
| Total Hyd.(ID= 1): 0.59 0.12 1.00 38.59 |  |  |  |  |  |  |
| MAJOR SYS.(ID= 2): 0.00 0.01 1.00 38.59 |  |  |  |  |  |  |
| MINOR SYS.(ID= 3): 0.59 0.11 1.00 38.59 |  |  |  |  |  |  |

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

\*\*\*\*\*  
V V I SSSSS U U A L (v 6.2.2008)  
V V I SS U U A A L  
V V I SS U U A A L L  
W I SSSSS UUUU A A LLLL  
000 TITTT TITTT H H Y Y M M 000 TN  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000  
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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\VisualOTHMIO 6.2\VO2\vojn.dat  
Output filename:  
C:\Users\ksain\AppData\Local\Civil\GIS\4c9a8b70-2b3a-4142-a551-3404df768702\ead0  
518-c681-421-9ef6-f4d29d5ef4scna  
Summary filename:  
C:\Users\ksain\AppData\Local\Civil\GIS\4c9a8b70-2b3a-4142-a551-3404df768702\ead0  
518-c681-421-9ef6-f4d29d5ef4scna

DATE: 08-28-2022 TIME: 02:37:55  
USER:

COMMENTS:

CHICAGO STORM IDF curve parameters: A= 811.794  
POT(24) 64.54 mm C= 0.699

AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
INFLOW:ID= 2 1.66 0.115 1.17 24.47  
OVERFLOW:ID= 3 1.66 0.076 1.50 13.53  
Volume Reduction Rate(RVIN-RVout)/RVIN (%)= 44.70  
Time to reach Max Ponding Storage (hr)= 0.00  
Volume of water for drawdown in LID (cu.m.)= 234.08  
Volume of Max. Water Storage (cu.m.)= 0.00  
Maximum Surface Ponding And Mulch Vol(cu.m.)= 171.55  
Maximum Engineered Soil Volume (cu.m.)= 64.41  
\*\*\*\* After simulation, water volume is not zero.

Junction Command(0181)  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
INFLOW: ID= 3 ( 0184) 1.66 0.08 1.50 13.53  
OUTFLOW: ID= 2 ( 0181) 1.66 0.08 1.50 13.53

CALIB STAMHVD ( 0205) Area (ha)= 0.00  
Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00  
IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.00  
Dep. Storage (mm)= 1.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 75.87 40.00  
Mannings n = 0.013 0.250  
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---  
TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
0.083 7.55 0.833 28.01 1.583 13.62 2.42 7.83  
0.167 7.55 0.917 162.35 1.667 13.62 2.42 7.83  
0.250 8.89 1.000 162.35 1.750 11.70 2.58 7.28  
0.333 8.89 1.083 34.38 1.833 11.70 2.58 7.28  
0.417 11.01 1.167 34.38 1.917 10.34 2.67 7.28  
0.500 11.01 1.250 21.68 2.000 10.34 2.75 6.82  
0.583 15.06 1.333 21.68 2.083 9.30 2.83 6.82  
0.667 15.06 1.417 16.53 2.167 9.30 2.92 6.42  
0.750 28.01 1.500 16.53 2.250 8.49 3.00 6.42

(ha) (cms) (hrs) (mm)  
\*\*\* WARNING: HYDROGRAPH 0182 <ID= 2> IS DRY.  
\*\*\* WARNING: HYDROGRAPH 0003 <HYDROGRAPH 0001  
ID= 1 ( 0183): 1.66 0.076 1.50 13.53  
+ ID= 2 ( 0182): 0.00 0.000 0.00 0.00  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0243) 1 + 2 = 3  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID= 1 ( 0243): 2.42 0.111 1.50 18.40  
ID= 3 ( 0243): 1.66 0.076 1.50 13.53  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0243) 1 + 2 + 3  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID= 1 ( 0243): 2.42 0.111 1.50 18.40  
ID= 2 ( 0262): 0.11 0.105 1.00 49.64  
ID= 3 ( 0243): 2.53 0.142 1.00 19.74  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR ( 0188) OVERFLOW IS OFF  
ID= 2 -> OUT - 1  
DT= 5.0 min  
OUTFLOW STORAGE OUTFLOW STORAGE  
(cms) (ha.m.) (cms) (ha.m.)  
0.0000 0.0000 0.4800 0.0190  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
INFLOW: ID= 2 ( 0243) 2.527 0.142 1.00 19.74  
OUTFLOW: ID= 1 ( 0188) 2.527 0.097 1.08 19.74  
PEAK FLOW REDUCTION (Qout/Qin)(%)= 68.62  
TIME SHIFT OF PEAK FLOW (min)= 5.00  
MAXIMUM STORAGE USED (ha.m.)= 0.0039

used INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33  
TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
0.083 7.55 0.833 162.35 1.67 11.70 2.58 7.28  
0.167 8.89 1.00 34.38 1.83 10.34 2.67 6.82  
0.33 11.01 1.17 21.68 2.00 9.30 2.83 6.42  
0.50 15.06 1.33 16.53 2.17 8.49  
0.67 28.01 1.50 13.62 2.33 7.83

CALIB HSDVD ( 0185) Area (ha)= 1.66 Curve Number (CN)= 74.4  
ID= 1 DT= 5.0 min Ia (mm)= 4.44 # of Linear Res.(N)= 3.00  
U.H. Tp(hrs)= 0.24  
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---  
TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
0.083 7.55 0.833 28.01 1.583 13.62 2.33 8.49  
0.167 7.55 0.917 162.35 1.667 13.62 2.42 7.83  
0.250 8.89 1.000 162.35 1.750 11.70 2.58 7.28  
0.333 8.89 1.083 34.38 1.833 11.70 2.58 7.28  
0.417 11.01 1.167 34.38 1.917 10.34 2.67 7.28  
0.500 11.01 1.250 21.68 2.000 10.34 2.75 6.82  
0.583 15.06 1.333 21.68 2.083 9.30 2.83 6.82  
0.667 15.06 1.417 16.53 2.167 9.30 2.92 6.42  
0.750 28.01 1.500 16.53 2.250 8.49 3.00 6.42

Unit Hyd. Peak (cms)= 0.265  
PEAK FLOW (cms)= 0.115 (i)  
TIME TO PEAK (hrs)= 1.67  
RUNOFF VOLUME (mm)= 24.467  
TOTAL RAINFALL (mm)= 64.542  
RUNOFF COEFFICIENT = 0.379  
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM IDF curve parameters: A= 811.794  
POT(24) 64.54 mm C= 0.699

Max. Eff. Inten. (mm/hr)= 162.35 \*\*\*\*\*  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.78 (ii) 6.32 (iii)  
Unit Hyd. Peak (min)= 5.00 10.00  
Unit Hyd. Peak (cms)= 0.32 0.15  
PEAK FLOW (cms)= 0.25 0.03 \*TOTALS\*  
TIME TO PEAK (hrs)= 1.00 1.08 1.00  
RUNOFF VOLUME (mm)= 63.54 23.83 49.64  
TOTAL RAINFALL (mm)= 64.54 64.54 64.54  
RUNOFF COEFFICIENT = 0.98 0.37 0.77  
\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PEROUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
TO THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Junction Command(0263)  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
INFLOW:ID= 3 ( 0264) 0.75 0.174 0.92 49.64  
OVERFLOW:ID= 3 ( 0264) 0.75 0.174 0.92 49.64  
Volume Reduction Rate(RVIN-RVout)/RVIN (%)= 41.32  
Time to reach Max Ponding Storage (hr)= 0.00  
Volume of water for drawdown in LID (cu.m.)= 161.97  
Volume of Max. Water Storage (cu.m.)= 0.00  
Maximum Surface Ponding And Mulch Vol(cu.m.)= 151.15  
Maximum Engineered Soil Volume (cu.m.)= 8.87  
\*\*\*\* After simulation, water volume is not zero.

Junction Command(0264)  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
INFLOW:ID= 3 ( 0264) 0.75 0.10 1.08 29.13  
OUTFLOW:ID= 2 ( 0262) 0.75 0.10 1.08 29.13  
CALIB STAMHVD ( 0243) 1 + 2 = 3  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID= 1 ( 0243): 2.42 0.111 1.50 18.40  
ID= 2 ( 0262): 0.11 0.105 1.00 49.64  
ID= 3 ( 0243): 2.53 0.142 1.00 19.74  
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

CALIB STAMHVD ( 0252) Area (ha)= 0.32  
Total Imp(%)= 69.00 Dir. Conn.(%)= 30.00  
IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.22 0.10  
Dep. Storage (mm)= 1.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 45.84 40.00  
Mannings n = 0.013 0.250  
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---  
TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
0.083 7.55 0.833 28.01 1.583 13.62 2.33 8.49  
0.167 7.55 0.917 162.35 1.667 13.62 2.42 7.83  
0.250 8.89 1.000 162.35 1.750 11.70 2.58 7.28  
0.333 8.89 1.083 34.38 1.833 11.70 2.58 7.28  
0.417 11.01 1.167 34.38 1.917 10.34 2.67 7.28  
0.500 11.01 1.250 21.68 2.000 10.34 2.75 6.82  
0.583 15.06 1.333 21.68 2.083 9.30 2.83 6.82  
0.667 15.06 1.417 16.53 2.167 9.30 2.92 6.42  
0.750 28.01 1.500 16.53 2.250 8.49 3.00 6.42

Max. Eff. Inten. (mm/hr)= 162.35 213.26  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.32 (ii) 6.53 (iii)  
Unit Hyd. Peak (min)= 5.00 10.00  
Unit Hyd. Peak (cms)= 0.33 0.14 \*TOTALS\*  
PEAK FLOW (cms)= 0.04 0.04 0.079 (iii)  
TIME TO PEAK (hrs)= 1.00 1.08 1.00  
RUNOFF VOLUME (mm)= 63.54 38.14 45.75  
TOTAL RAINFALL (mm)= 64.54 64.54 64.54  
RUNOFF COEFFICIENT = 0.98 0.59 0.71  
\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PEROUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
TO THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0188) OVERFLOW IS OFF  
ID= 2 -> OUT - 1  
DT= 5.0 min  
OUTFLOW STORAGE OUTFLOW STORAGE  
(cms) (ha.m.) (cms) (ha.m.)  
0.0000 0.0000 0.4800 0.0190  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
INFLOW: ID= 2 ( 0243) 2.527 0.142 1.00 19.74  
OUTFLOW: ID= 1 ( 0188) 2.527 0.097 1.08 19.74  
PEAK FLOW REDUCTION (Qout/Qin)(%)= 68.62  
TIME SHIFT OF PEAK FLOW (min)= 5.00  
MAXIMUM STORAGE USED (ha.m.)= 0.0039

DUHDV ( 0182) Inlet Cap.= 11  
# of Inlets= 1  
Total Imp(%)= 0.11 Area QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
TOTAL HYD.(ID= 1): 1.66 0.11 1.17 24.47  
MAJOR SYS.(ID= 2): 0.00 0.00 0.00 0.00  
MINOR SYS.(ID= 3): 1.66 0.11 1.17 24.47  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Outflow: OFF, UNDERDRAIN: OFF  
ID= 2 -> OUT - 3  
DT= 5.0 min

SURFACE PONDING LAYER:  
Max. Ponding Storage(cu.m.)= 149.95  
STAGE AREA STAGE AREA  
(m) (m2) (m) (m2)  
0.000 100.000 1.500 100.000  
1.000 100.000 0.000 0.000

MULCH LAYER: Depth (m)= 0.10 Porosity = 0.40  
Maximum Mulch Layer Volume (cu.m.)= 21.60  
ENGINEERED SOIL LAYER: Soil moisture = 0.30 Depth (m)= 0.72  
Width (m)= 100.00 Length (m)= 3.00  
Porosity = 0.47 Infiltration (m/hr)= 0.0000  
Maximum Engineered Soil Layer Volume(cu.m.)= 181.57

STORAGE LAYER: Depth (m)= 0.50 Porosity = 0.40  
Seepage (m/hr)= 0.0010  
Storage Layer Volume(cu.m.)= 108.00

TOTAL AVAILABLE STORAGE:  
TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil  
+Storage Layer(cu.m.)= 481.12

NATIVE SOIL LAYER: Infiltration (m/hr)= 0.0060

MULCH LAYER: Depth (m)= 0.10 Porosity = 0.40  
Maximum Mulch Layer Volume (cu.m.)= 3.20  
ENGINEERED SOIL LAYER: Soil moisture = 0.30 Depth (m)= 0.67  
Width (m)= 40.00 Length (m)= 6.00  
Porosity = 0.47 Infiltration (m/hr)= 0.0000  
Maximum Engineered Soil Layer Volume(cu.m.)= 25.83

STORAGE LAYER: Depth (m)= 1.50 Porosity = 0.40  
Seepage (m/hr)= 0.0010  
Maximum Storage Layer Volume(cu.m.)= 48.00

TOTAL AVAILABLE STORAGE:  
TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil  
+Storage Layer(cu.m.)= 226.18

NATIVE SOIL LAYER: Infiltration (m/hr)= 0.0060  
Junction Command(0262) 1 + 2 = 3  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
INFLOW:ID= 2 ( 0264) 0.75 0.10 1.08 29.13  
OUTFLOW:ID= 2 ( 0262) 0.75 0.10 1.08 29.13  
Volume Reduction Rate(RVIN-RVout)/RVIN (%)= 41.32  
Time to reach Max Ponding Storage (hr)= 0.00  
Volume of water for drawdown in LID (cu.m.)= 161.97  
Volume of Max. Water Storage (cu.m.)= 0.00  
Maximum Surface Ponding And Mulch Vol(cu.m.)= 151.15  
Maximum Engineered Soil Volume (cu.m.)= 8.87  
\*\*\*\* After simulation, water volume is not zero.

Junction Command(0262) 1 + 2 = 3  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
INFLOW:ID= 3 ( 0264) 0.75 0.10 1.08 29.13  
OUTFLOW:ID= 2 ( 0262) 0.75 0.10 1.08 29.13  
ADD HYD ( 0243) 1 + 2 = 3  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID= 1 ( 0243): 2.42 0.111 1.50 18.40  
ID= 2 ( 0262): 0.11 0.105 1.00 49.64  
ID= 3 ( 0243): 2.53 0.142 1.00 19.74  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0179) 1 + 2 = 3  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID= 1 ( 0188): 2.53 0.097 1.08 19.74  
+ ID= 2 ( 0252): 0.32 0.079 1.00 45.75  
ID= 3 ( 0179): 2.84 0.149 1.08 22.63  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STAMHVD ( 0106) Area (ha)= 0.51  
ID= 1 DT= 5.0 min Total Imp(%)= 55.00 Dir. Conn.(%)= 1.00  
IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.28 0.23  
Dep. Storage (mm)= 1.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 58.22 40.00  
Mannings n = 0.013 0.250  
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---  
TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
0.083 7.55 0.833 28.01 1.583 13.62 2.33 8.49  
0.167 7.55 0.917 162.35 1.667 13.62 2.42 7.83  
0.250 8.89 1.000 162.35 1.750 11.70 2.58 7.28  
0.333 8.89 1.083 34.38 1.833 11.70 2.58 7.28  
0.417 11.01 1.167 34.38 1.917 10.34 2.67 7.28  
0.500 11.01 1.250 21.68 2.000 10.34 2.75 6.82  
0.583 15.06 1.333 21.68 2.083 9.30 2.83 6.82  
0.667 15.06 1.417 16.53 2.167 9.30 2.92 6.42  
0.750 28.01 1.500 16.53 2.250 8.49 3.00 6.42

Max. Eff. Inten. (mm/hr)= 162.35 204.85  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.52 (ii) 6.82 (iii)  
Unit Hyd. Peak (min)= 5.00 10.00  
Unit Hyd. Peak (cms)= 0.33 0.14 \*TOTALS\*  
PEAK FLOW (cms)= 0.00 0.00 0.093 (iii)  
TIME TO PEAK (hrs)= 1.00 1.08 1.00  
RUNOFF VOLUME (mm)= 63.54 37.74 37.96  
TOTAL RAINFALL (mm)= 64.54 64.54 64.54



RUNOFF COEFFICIENT = 0.98 0.58 0.59

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| RESERVOIR (0282) | OVERFLOW IS OFF |
|------------------|-----------------|
| DT= 5.0 min      |                 |
| Area (ha)        | 0.58            |
| Dir. Conn. (%)   | 1.00            |

INFLW: ID= 2 ( 0106) 0.508 0.093 1.08 37.96  
OUTFLOW: ID= 1 ( 0282) 0.508 0.083 1.08 37.95

PEAK FLOW REDUCTION (Qout/Qin)(%)= 88.42  
TIME SHIFT OF PEAK FLOW (min)= 0.000  
MAXIMUM STORAGE USED (ha.m.)= 0.0018

| IMPERVIOUS        | PERVIOUS (1) |
|-------------------|--------------|
| Surface Area (ha) | 0.26         |
| Dep. Storage (mm) | 5.00         |
| Average Slope (%) | 2.00         |
| Length (m)        | 40.00        |
| Mannings n        | 0.250        |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

| TIME  | RAIN | TIME  | RAIN   | TIME  | RAIN  |
|-------|------|-------|--------|-------|-------|
| 0.167 | 7.55 | 0.917 | 162.35 | 1.667 | 13.62 |
| 2.42  | 7.83 |       |        |       |       |

| 0.250 | 8.89  | 1.000 | 162.35 | 1.750 | 11.70 | 2.50 | 7.83 |
|-------|-------|-------|--------|-------|-------|------|------|
| 0.333 | 8.89  | 1.083 | 34.38  | 1.833 | 11.70 | 2.58 | 7.28 |
| 0.417 | 11.01 | 1.167 | 34.38  | 1.917 | 10.34 | 2.67 | 7.28 |
| 0.500 | 11.01 | 1.250 | 21.68  | 2.000 | 10.34 | 2.75 | 6.82 |
| 0.583 | 15.06 | 1.333 | 21.68  | 2.083 | 9.30  | 2.83 | 6.82 |
| 0.667 | 15.06 | 1.417 | 16.53  | 2.167 | 9.30  | 2.92 | 6.42 |
| 0.750 | 28.01 | 1.500 | 16.53  | 2.250 | 8.49  | 3.00 | 6.42 |

Max.Eff.Inten.(mm/hr)= 162.35 204.05  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.58 (11) 6.88 (11)  
Unit Hyd. Peak (mm)= 5.00 10.00  
Unit Hyd. Peak (cms)= 0.33 0.14

PEAK FLOW (cms)= 0.00 0.11 \*TOTALS\*  
TIME TO PEAK (hrs)= 1.00 1.08 0.106 (111)  
RUNOFF VOLUME (mm)= 63.54 37.71 37.96  
TOTAL RAINFALL (mm)= 64.54 64.54 64.54  
RUNOFF COEFFICIENT = 0.98 0.98 0.59

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| STANDHVD ( 0245) | Area (ha) | 0.58                |
|------------------|-----------|---------------------|
| Total Imp(%)     | 55.00     | Dir. Conn.(%)= 1.00 |

IMPERVIOUS PERVIOUS (1)  
Surface Area (ha)= 0.26  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 62.24 40.00  
Length (m)= 0.13 0.250

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| TIME  | RAIN | TIME  | RAIN   | TIME  | RAIN  |
|-------|------|-------|--------|-------|-------|
| 0.167 | 7.55 | 0.917 | 162.35 | 1.667 | 13.62 |
| 2.42  | 7.83 |       |        |       |       |

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| STANDHVD ( 0019) | Area (ha) | 0.50                |
|------------------|-----------|---------------------|
| Total Imp(%)     | 55.00     | Dir. Conn.(%)= 1.00 |

IMPERVIOUS PERVIOUS (1)  
Surface Area (ha)= 0.28  
Dep. Storage (mm)= 1.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 57.74 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

| TIME  | RAIN | TIME  | RAIN   | TIME  | RAIN  |
|-------|------|-------|--------|-------|-------|
| 0.167 | 7.55 | 0.917 | 162.35 | 1.667 | 13.62 |
| 2.42  | 7.83 |       |        |       |       |

| 0.583 | 15.06 | 1.333 | 21.68 | 2.083 | 9.30 | 2.83 | 6.82 |
|-------|-------|-------|-------|-------|------|------|------|
| 0.667 | 15.06 | 1.417 | 16.53 | 2.167 | 9.30 | 2.92 | 6.42 |
| 0.750 | 28.01 | 1.500 | 16.53 | 2.250 | 8.49 | 3.00 | 6.42 |

Max.Eff.Inten.(mm/hr)= 162.35 204.05  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.51 (11) 6.81 (11)  
Unit Hyd. Peak (mm)= 5.00 10.00  
Unit Hyd. Peak (cms)= 0.33 0.14

PEAK FLOW (cms)= 0.00 0.09 0.092 (111)  
TIME TO PEAK (hrs)= 1.00 1.08  
RUNOFF VOLUME (mm)= 63.54 37.71 37.96  
TOTAL RAINFALL (mm)= 64.54 64.54 64.54  
RUNOFF COEFFICIENT = 0.98 0.98 0.59

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HVD ( 0248) | Area (ha) | 0.78                |
|-----------------|-----------|---------------------|
| Total Imp(%)    | 70.00     | Dir. Conn.(%)= 1.00 |

IMPERVIOUS PERVIOUS (1)  
Surface Area (ha)= 0.50 0.38  
Dep. Storage (mm)= 1.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 92.38 40.00  
Mannings n = 0.013 0.250

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| TIME  | RAIN | TIME  | RAIN   | TIME  | RAIN  |
|-------|------|-------|--------|-------|-------|
| 0.167 | 7.55 | 0.917 | 162.35 | 1.667 | 13.62 |
| 2.42  | 7.83 |       |        |       |       |

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

| TIME  | RAIN | TIME  | RAIN   | TIME  | RAIN  |
|-------|------|-------|--------|-------|-------|
| 0.167 | 7.55 | 0.917 | 162.35 | 1.667 | 13.62 |
| 2.42  | 7.83 |       |        |       |       |

Max.Eff.Inten.(mm/hr)= 162.35 204.05  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 2.01 (11) 6.15 (11)  
Unit Hyd. Peak (mm)= 5.00 10.00  
Unit Hyd. Peak (cms)= 0.31 0.15

PEAK FLOW (cms)= 0.40 0.04 0.436 (111)  
TIME TO PEAK (hrs)= 1.00 1.08 1.00  
RUNOFF VOLUME (mm)= 63.54 23.83 51.63  
TOTAL RAINFALL (mm)= 64.54 64.54 64.54  
RUNOFF COEFFICIENT = 0.98 0.37 0.80

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| STANDHVD ( 0279) | Area (ha) | 1.28                 |
|------------------|-----------|----------------------|
| Total Imp(%)     | 70.00     | Dir. Conn.(%)= 70.00 |

IMPERVIOUS PERVIOUS (1)  
Surface Area (ha)= 0.90 0.38  
Dep. Storage (mm)= 1.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 92.38 40.00  
Mannings n = 0.013 0.250

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| TIME  | RAIN | TIME  | RAIN   | TIME  | RAIN  |
|-------|------|-------|--------|-------|-------|
| 0.167 | 7.55 | 0.917 | 162.35 | 1.667 | 13.62 |
| 2.42  | 7.83 |       |        |       |       |

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| STANDHVD ( 0279) | Area (ha) | 1.28                 |
|------------------|-----------|----------------------|
| Total Imp(%)     | 70.00     | Dir. Conn.(%)= 70.00 |

IMPERVIOUS PERVIOUS (1)  
Surface Area (ha)= 0.90 0.38  
Dep. Storage (mm)= 1.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 92.38 40.00  
Mannings n = 0.013 0.250

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| TIME  | RAIN | TIME  | RAIN   | TIME  | RAIN  |
|-------|------|-------|--------|-------|-------|
| 0.167 | 7.55 | 0.917 | 162.35 | 1.667 | 13.62 |
| 2.42  | 7.83 |       |        |       |       |

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| STANDHVD ( 0313) | Area (ha) | 4.05                 |
|------------------|-----------|----------------------|
| Total Imp(%)     | 60.00     | Dir. Conn.(%)= 60.00 |

IMPERVIOUS PERVIOUS (1)  
Surface Area (ha)= 2.43 1.62  
Dep. Storage (mm)= 5.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 164.25 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

| TIME  | RAIN | TIME  | RAIN   | TIME  | RAIN  |
|-------|------|-------|--------|-------|-------|
| 0.167 | 7.55 | 0.917 | 162.35 | 1.667 | 13.62 |
| 2.42  | 7.83 |       |        |       |       |

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| STANDHVD ( 0313) | Area (ha) | 4.05                 |
|------------------|-----------|----------------------|
| Total Imp(%)     | 60.00     | Dir. Conn.(%)= 60.00 |

IMPERVIOUS PERVIOUS (1)  
Surface Area (ha)= 2.43 1.62  
Dep. Storage (mm)= 5.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 164.25 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

| TIME  | RAIN | TIME  | RAIN   | TIME  | RAIN  |
|-------|------|-------|--------|-------|-------|
| 0.167 | 7.55 | 0.917 | 162.35 | 1.667 | 13.62 |
| 2.42  | 7.83 |       |        |       |       |

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| STANDHVD ( 0315) | Area (ha) | 14.21                |
|------------------|-----------|----------------------|
| Total Imp(%)     | 71.00     | Dir. Conn.(%)= 71.00 |

IMPERVIOUS PERVIOUS (1)  
Surface Area (ha)= 10.09 4.12  
Dep. Storage (mm)= 1.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 307.76 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

| TIME  | RAIN | TIME  | RAIN   | TIME  | RAIN  |
|-------|------|-------|--------|-------|-------|
| 0.167 | 7.55 | 0.917 | 162.35 | 1.667 | 13.62 |
| 2.42  | 7.83 |       |        |       |       |

| hrs   | mm/hr | hrs   | mm/hr  | hrs   | mm/hr | hrs  | mm/hr |
|-------|-------|-------|--------|-------|-------|------|-------|
| 0.083 | 7.55  | 0.833 | 28.01  | 1.583 | 13.62 | 2.33 | 8.49  |
| 0.167 | 7.55  | 0.917 | 162.35 | 1.667 | 13.62 | 2.42 | 7.83  |
| 0.250 | 8.89  | 1.000 | 162.35 | 1.750 | 11.70 | 2.50 | 7.83  |
| 0.333 | 8.89  | 1.083 | 34.38  | 1.833 | 11.70 | 2.58 | 7.28  |
| 0.417 | 11.01 | 1.167 | 34.38  | 1.917 | 10.34 | 2.67 | 7.28  |
| 0.500 | 11.01 | 1.250 | 21.68  | 2.000 | 10.34 | 2.75 | 6.82  |
| 0.583 | 15.06 | 1.333 | 21.68  | 2.083 | 9.30  | 2.83 | 6.82  |
| 0.667 | 15.06 | 1.417 | 16.53  | 2.167 | 9.30  | 2.92 | 6.42  |
| 0.750 | 28.01 | 1.500 | 16.53  | 2.250 | 8.49  | 3.00 | 6.42  |

Max. Eff. Inten. (mm/hr) = 162.35  
 over (min) = 5.00  
 Storage Coeff. (min) = 4.13 (11)  
 Unit Hyd. Tpeak (min) = 5.00  
 Unit Hyd. peak (cms) = 0.24

PEAK FLOW (cms) = 4.21  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 63.54  
 TOTAL RAINFALL (mm) = 64.54  
 RUNOFF COEFFICIENT = 0.98

\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 CN = 74.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- SURFACE PONDING LAYER: -----

Max. Ponding Storage(cu.m.)= 149.95

| STAGE | AREA    | STAGE | AREA    |
|-------|---------|-------|---------|
| (m)   | (m2)    | (m)   | (m2)    |
| 0.000 | 100.000 | 1.500 | 100.000 |
| 0.500 | 100.000 | 0.000 | 0.000   |
| 1.000 | 100.000 | 0.000 | 0.000   |

MULCH LAYER:  
 Depth (m) = 0.10 Porosity = 0.40  
 Maximum Mulch Layer Volume (cu.m.) = 62.40

ENGINEERED SOIL LAYER:  
 Soil moisture = 0.30 Depth (m) = 0.67  
 Width (m) = 780.00 Length (m) = 2.00  
 Porosity = 0.47 Infiltration (m/hr) = 0.0060  
 Maximum Engineered Soil Layer Volume(cu.m.) = 488.11

STORAGE LAYER:  
 Depth (m) = 1.50 Porosity = 0.40  
 Seepage (m/hr) = 0.0010  
 Maximum Storage Layer Volume(cu.m.) = 936.00

TOTAL AVAILABLE STORAGE:  
 TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil  
 +Storage Layer(cu.m.)= 1636.46

NATIVE SOIL LAYER:  
 Infiltration (m/hr) = 0.0060

| AREA           | QPEAK | TPEAK | R.V. |       |
|----------------|-------|-------|------|-------|
| (ha)           | (cms) | (hrs) | (mm) |       |
| INFLW:ID= 2    | 12.97 | 2.948 | 0.92 | 52.02 |
| OVERFLOW:ID= 3 | 12.97 | 2.945 | 0.92 | 50.14 |

Volume Reduction Rate((RvIn-RvOut)/RvIn) (%) = 3.63  
 Time to reach Max Ponding Storage (hr) = 0.00  
 Volume of water for drawdown in LID (cu.m.) = 385.88  
 Volume of Max. Water Storage (cu.m.) = 0.00  
 Maximum Surface Ponding And Mulch Vol(cu.m.) = 212.35  
 Maximum Engineered Soil Volume (cu.m.) = 176.20  
 \*\*\*\* After simulation, water volume is not zero.

----- JUNCTION COMMAND(0317) -----

| AREA                  | QPEAK | TPEAK | R.V. |       |
|-----------------------|-------|-------|------|-------|
| (ha)                  | (cms) | (hrs) | (mm) |       |
| INFLW : ID= 3( 0318)  | 12.97 | 2.95  | 0.92 | 50.14 |
| OUTFLOW: ID= 2( 0317) | 12.97 | 2.95  | 0.92 | 50.14 |

----- ADD HYD ( 0316) -----  
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0311): 1.24 1.583 1.00 52.02  
 + ID2= 2 ( 0317): 12.97 2.945 0.92 50.14  
 ID = 3 ( 0316): 14.21 4.528 1.00 50.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB ( 0323) -----  
 STAMHWD ( 0323) | Area (ha)= 0.50  
 ID= 1 Dfs: 5.0 min | Total Dfs(%)= 66.00 Dfs. Conn.(%)= 25.00

| Surface Area | Dep. Storage | Average Slope | Length | Mannings n |
|--------------|--------------|---------------|--------|------------|
| (ha)         | (mm)         | (%)           | (m)    |            |
| 1.00         | 5.00         | 1.00          | 2.00   |            |
| 0.013        | 0.250        |               |        |            |

IMPERVIOUS PERVIOUS (i)  
 NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYDROGRAPH -----

| TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN |       |       |      |      |
|-----------|-----------|-----------|-----------|-------|-------|------|------|
| (hrs)     | (mm/hr)   | (hrs)     | (mm/hr)   |       |       |      |      |
| 0.083     | 7.55      | 0.833     | 28.01     | 1.583 | 13.62 | 2.33 | 8.49 |
| 0.167     | 7.55      | 0.917     | 162.35    | 1.667 | 13.62 | 2.42 | 7.83 |
| 0.250     | 8.89      | 1.000     | 162.35    | 1.750 | 11.70 | 2.50 | 7.83 |
| 0.333     | 8.89      | 1.083     | 34.38     | 1.833 | 11.70 | 2.58 | 7.28 |
| 0.417     | 11.01     | 1.167     | 34.38     | 1.917 | 10.34 | 2.67 | 7.28 |
| 0.500     | 11.01     | 1.250     | 21.68     | 2.000 | 10.34 | 2.75 | 6.82 |
| 0.583     | 15.06     | 1.333     | 21.68     | 2.083 | 9.30  | 2.83 | 6.82 |
| 0.667     | 15.06     | 1.417     | 16.53     | 2.167 | 9.30  | 2.92 | 6.42 |
| 0.750     | 28.01     | 1.500     | 16.53     | 2.250 | 8.49  | 3.00 | 6.42 |

Max. Eff. Inten. (mm/hr) = 162.35  
 over (min) = 5.00  
 Storage Coeff. (min) = 1.59 (11)  
 Unit Hyd. Tpeak (min) = 5.00

| Unit Hyd. peak | PEAK FLOW | TIME TO PEAK | RUNOFF VOLUME | TOTAL RAINFALL | RUNOFF COEFFICIENT |
|----------------|-----------|--------------|---------------|----------------|--------------------|
| (cms)          | (cms)     | (hrs)        | (mm)          | (mm)           |                    |
| 0.33           | 0.07      | 0.08         | 1.00          | 63.54          | 0.98               |
| 0.14           | 0.07      | 0.08         | 1.00          | 64.54          | 0.58               |

\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 CN = 74.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- ADD HYD ( 0331) -----  
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0329): 0.24 0.01 0.08 26.94  
 + ID2= 2 ( 0332): 0.24 0.01 0.08 26.94  
 ID = 3 ( 0306): 0.35 0.030 1.42 27.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD ( 0316) -----  
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0330): 0.24 0.009 0.08 26.94  
 + ID2= 2 ( 0332): 0.24 0.009 0.08 26.94  
 ID = 3 ( 0306): 0.59 0.035 1.42 27.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD ( 0316) -----  
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0327): 1.12 0.270 1.00 37.54  
 + ID2= 2 ( 0310): 5.48 1.574 1.00 44.71  
 ID = 3 ( 0312): 6.59 1.843 1.00 43.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD ( 0312) -----  
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0312): 6.59 1.843 1.00 43.50  
 + ID2= 2 ( 0310): 0.59 0.035 1.42 27.43  
 ID = 1 ( 0313): 7.19 1.874 1.00 42.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Soil moisture = 0.30 Depth (m) = 0.72  
 Width (m) = 1.00 Length (m) = 242.09  
 Porosity = 0.47 Infiltration (m/hr) = 0.0060  
 Maximum Engineered Soil Layer Volume(cu.m.) = 242.09

| DEPTH | DISCHARGE | DEPTH | DISCHARGE |
|-------|-----------|-------|-----------|
| (m)   | (cms)     | (m)   | (cms)     |
| 0.000 | 0.000     | 0.550 | 0.030     |
| 0.279 | 0.010     | 0.762 | 0.040     |
| 0.406 | 0.020     | 1.300 | 0.050     |

STORAGE LAYER:  
 Depth (m) = 1.00 Porosity = 0.40  
 Seepage (m/hr) = 0.0010  
 Maximum Storage Layer Volume(cu.m.) = 288.00

TOTAL AVAILABLE STORAGE:  
 TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil  
 +Storage Layer(cu.m.)= 680.04

NATIVE SOIL LAYER:  
 Infiltration (m/hr) = 0.0060

| AREA           | QPEAK | TPEAK | R.V. |       |
|----------------|-------|-------|------|-------|
| (ha)           | (cms) | (hrs) | (mm) |       |
| INFLW:ID= 2    | 0.57  | 0.112 | 1.00 | 44.19 |
| OUTFLOW:ID= 1  | 0.24  | 0.009 | 0.08 | 26.94 |
| OVERFLOW:ID= 3 | 0.33  | 0.030 | 1.42 | 26.94 |

Volume Reduction Rate((RvIn-RvOut)/RvIn) (%) = 39.03  
 Time to reach Max Ponding Storage (hr) = 0.00  
 Volume of water for drawdown in LID (cu.m.) = 169.12  
 Volume of Max. Water Storage (cu.m.) = 0.00  
 Maximum Surface Ponding And Mulch Vol(cu.m.) = 149.95  
 Maximum Engineered Soil Volume (cu.m.) = 69.06  
 \*\*\*\* After simulation, water volume is not zero.

----- JUNCTION COMMAND(0330) -----

| AREA                  | QPEAK | TPEAK | R.V. |       |
|-----------------------|-------|-------|------|-------|
| (ha)                  | (cms) | (hrs) | (mm) |       |
| INFLW : ID= 3( 0329)  | 0.33  | 0.03  | 1.42 | 26.94 |
| OUTFLOW: ID= 2( 0330) | 0.33  | 0.03  | 1.42 | 26.94 |

----- JUNCTION COMMAND(0322) -----

| AREA                  | QPEAK | TPEAK | R.V. |       |
|-----------------------|-------|-------|------|-------|
| (ha)                  | (cms) | (hrs) | (mm) |       |
| INFLW : ID= 1( 0329)  | 0.24  | 0.01  | 0.08 | 26.94 |
| OUTFLOW: ID= 2( 0332) | 0.24  | 0.01  | 0.08 | 26.94 |

----- ADD HYD ( 0316) -----  
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0330): 0.24 0.009 0.08 26.94  
 + ID2= 2 ( 0332): 0.24 0.009 0.08 26.94  
 ID = 3 ( 0306): 0.35 0.030 1.42 27.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD ( 0316) -----  
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0330): 0.24 0.009 0.08 26.94  
 + ID2= 2 ( 0332): 0.24 0.009 0.08 26.94  
 ID = 3 ( 0306): 0.59 0.035 1.42 27.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD ( 0316) -----  
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0327): 1.12 0.270 1.00 37.54  
 + ID2= 2 ( 0310): 5.48 1.574 1.00 44.71  
 ID = 3 ( 0312): 6.59 1.843 1.00 43.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD ( 0312) -----  
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0312): 6.59 1.843 1.00 43.50  
 + ID2= 2 ( 0310): 0.59 0.035 1.42 27.43  
 ID = 1 ( 0313): 7.19 1.874 1.00 42.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR( 0319) | OVERFLOW IS OFF  
 ID= 2--> OUT= 1  
 Dfs: 5.0 min

| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
|---------|---------|---------|---------|
| (cms)   | (ha.m.) | (cms)   | (ha.m.) |
| 0.0000  | 0.0000  | 0.6510  | 0.9850  |
| 0.0400  | 0.1140  | 1.0960  | 1.3320  |
| 0.0760  | 0.3690  | 1.4260  | 1.5860  |
| 0.2810  | 0.6450  | 0.0000  | 0.0000  |

| INFLW                  | QPEAK  | TPEAK | R.V. |       |
|------------------------|--------|-------|------|-------|
| (ha)                   | (cms)  | (hrs) | (mm) |       |
| ID= 2 ( 0312)          | 21.393 | 6.402 | 1.00 | 47.54 |
| OUTFLOW: ID= 1 ( 0319) | 21.393 | 6.404 | 2.58 | 47.54 |

PEAK FLOW REDUCTION [(Qout/Qin)(%)] = 6.30  
 TIME SHIFT OF PEAK FLOW (min) = 95.00  
 MAXIMUM STORAGE USED (ha.m.) = 0.7548

FINISH

RESERVOIR( 0319) | OVERFLOW IS OFF  
 ID= 2--> OUT= 1  
 Dfs: 5.0 min

| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
|---------|---------|---------|---------|
| (cms)   | (ha.m.) | (cms)   | (ha.m.) |
| 0.0000  | 0.0000  | 0.6510  | 0.9850  |
| 0.0400  | 0.1140  | 1.0960  | 1.3320  |
| 0.0760  | 0.3690  | 1.4260  | 1.5860  |
| 0.2810  | 0.6450  | 0.0000  | 0.0000  |

| INFLW                  | QPEAK  | TPEAK | R.V. |       |
|------------------------|--------|-------|------|-------|
| (ha)                   | (cms)  | (hrs) | (mm) |       |
| ID= 2 ( 0312)          | 21.393 | 6.402 | 1.00 | 47.54 |
| OUTFLOW: ID= 1 ( 0319) | 21.393 | 6.404 | 2.58 | 47.54 |

PEAK FLOW REDUCTION [(Qout/Qin)(%)] = 6.30  
 TIME SHIFT OF PEAK FLOW (min) = 95.00  
 MAXIMUM STORAGE USED (ha.m.) = 0.7548

FINISH

RESERVOIR( 0319) | OVERFLOW IS OFF  
 ID= 2--> OUT= 1  
 Dfs: 5.0 min

| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
|---------|---------|---------|---------|
| (cms)   | (ha.m.) | (cms)   | (ha.m.) |
| 0.0000  | 0.0000  | 0.6510  | 0.9850  |
| 0.0400  | 0.1140  | 1.0960  | 1.3320  |
| 0.0760  | 0.3690  | 1.4260  | 1.5860  |
| 0.2810  | 0.6450  | 0.0000  | 0.0000  |

| INFLW                  | QPEAK  | TPEAK | R.V. |       |
|------------------------|--------|-------|------|-------|
| (ha)                   | (cms)  | (hrs) | (mm) |       |
| ID= 2 ( 0312)          | 21.393 | 6.402 | 1.00 | 47.54 |
| OUTFLOW: ID= 1 ( 0319) | 21.393 | 6.404 | 2.58 | 47.54 |

PEAK FLOW REDUCTION [(Qout/Qin)(%)] = 6.30  
 TIME SHIFT OF PEAK FLOW (min) = 95.00  
 MAXIMUM STORAGE USED (ha.m.) = 0.7548

FINISH

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=====
V V I SSSSS U U A L L (v 6.2.2008)
V V I SS U U A A L
V V I SS U U A A L L
W I SSSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TN
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual OTTHYMO 6.2\VO2\vojn.dat

Output filename:
C:\Users\ksain\AppData\Local\Civical\WHS\4\9a8a70-2b3a-4142-a551-3404df768702\7ba2
950-4f5d-4e3a-9b11-662ac82cc65\scna
Summary filename:
C:\Users\ksain\AppData\Local\Civical\WHS\4\9a8a70-2b3a-4142-a551-3404df768702\7ba2
950-4f5d-4e3a-9b11-662ac82cc65\scna

DATE: 08-28-2022 TIME: 02:37:55

USER:

COMMENTS:
-----
[ CHICAGO STORM ] IDF curve parameters: Ar= 892.273
[ POTD= 70.94 mm ] B= 0.00000
C= 0.699

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-----
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 1.66 0.131 1.17 28.71
OVERFLOW: ID= 3 1.66 0.104 1.42 17.70

Volume Reduction Rate[(Rvin-Rvout)/Rvin] (%)= 38.33
Time to reach Max Ponding Storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 234.26
Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 171.55
Maximum Engineered Soil Volume (cu.m.)= 64.77
**** After simulation, water volume is not zero.

[ Junction Command(0181) ]
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 3 ( 0184) 1.66 0.10 1.42 17.70
OUTFLOW: ID= 2 ( 0181) 1.66 0.10 1.42 17.70

-----
[ CALIB ]
[ STANDHYD ( 0205) ] Area (ha)= 0.80
ID= 1 DT= 5.0 min Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00

Surface Area (ha)= 0.80
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 75.87 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 8.30 0.833 30.79 1.583 14.97 2.33 9.33
0.167 8.30 0.917 178.44 1.667 14.97 2.42 8.61
0.250 9.77 1.000 178.44 1.750 12.86 2.50 8.63
0.333 9.77 1.083 37.79 1.833 12.86 2.58 8.00
0.417 12.10 1.167 37.79 1.917 11.36 2.67 8.00
0.500 12.10 1.250 23.83 2.000 11.36 2.75 7.50
0.583 16.55 1.333 23.83 2.083 10.23 2.83 7.50
0.667 16.55 1.417 18.17 2.167 10.23 2.92 7.06
0.750 30.79 1.500 18.17 2.250 9.33 3.00 7.06

Max.Eff.Inten.(mm/hr)= 178.44 *****
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.72 (ii) 6.09 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.28 0.04 *TOTALS*
TIME TO PEAK (hrs)= 1.00 1.08 1.00 0.310 (iii)
RUNOFF VOLUME (mm)= 69.94 28.02 55.27
TOTAL RAINFALL (mm)= 70.94 70.94 70.94
RUNOFF COEFFICIENT = 0.99 0.39 0.78

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
[ CHICAGO STORM ] IDF curve parameters: Ar= 892.273
[ POTD= 70.94 mm ] B= 0.00000
C= 0.699

-----
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 1.66 0.131 1.17 28.71
OVERFLOW: ID= 3 1.66 0.104 1.42 17.70

Volume Reduction Rate[(Rvin-Rvout)/Rvin] (%)= 38.33
Time to reach Max Ponding Storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 234.26
Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 171.55
Maximum Engineered Soil Volume (cu.m.)= 64.77
**** After simulation, water volume is not zero.

[ Junction Command(0181) ]
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 3 ( 0184) 1.66 0.10 1.42 17.70
OUTFLOW: ID= 2 ( 0181) 1.66 0.10 1.42 17.70

-----
[ CALIB ]
[ STANDHYD ( 0205) ] Area (ha)= 0.80
ID= 1 DT= 5.0 min Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00

Surface Area (ha)= 0.80
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 75.87 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 8.30 0.833 30.79 1.583 14.97 2.33 9.33
0.167 8.30 0.917 178.44 1.667 14.97 2.42 8.61
0.250 9.77 1.000 178.44 1.750 12.86 2.50 8.63
0.333 9.77 1.083 37.79 1.833 12.86 2.58 8.00
0.417 12.10 1.167 37.79 1.917 11.36 2.67 8.00
0.500 12.10 1.250 23.83 2.000 11.36 2.75 7.50
0.583 16.55 1.333 23.83 2.083 10.23 2.83 7.50
0.667 16.55 1.417 18.17 2.167 10.23 2.92 7.06
0.750 30.79 1.500 18.17 2.250 9.33 3.00 7.06

Max.Eff.Inten.(mm/hr)= 178.44 *****
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.72 (ii) 6.09 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.28 0.04 *TOTALS*
TIME TO PEAK (hrs)= 1.00 1.08 1.00 0.310 (iii)
RUNOFF VOLUME (mm)= 69.94 28.02 55.27
TOTAL RAINFALL (mm)= 70.94 70.94 70.94
RUNOFF COEFFICIENT = 0.99 0.39 0.78

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 8.30 0.833 30.79 1.583 14.97 2.33 9.33
0.167 8.30 0.917 178.44 1.667 14.97 2.42 8.61
0.250 9.77 1.000 178.44 1.750 12.86 2.50 8.63
0.333 9.77 1.083 37.79 1.833 12.86 2.58 8.00
0.417 12.10 1.167 37.79 1.917 11.36 2.67 8.00
0.500 12.10 1.250 23.83 2.000 11.36 2.75 7.50
0.583 16.55 1.333 23.83 2.083 10.23 2.83 7.50
0.667 16.55 1.417 18.17 2.167 10.23 2.92 7.06
0.750 30.79 1.500 18.17 2.250 9.33 3.00 7.06

CALIB
[ RESURVID ( 0185) ] Area (ha)= 1.66 Curve Number (CN)= 74.4
ID= 1 DT= 5.0 min Ia (mm)= 4.44 # of Linear Res.(N)= 3.00
U.H. Tp(hrs)= 0.24

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 8.30 0.833 30.79 1.583 14.97 2.33 9.33
0.167 8.30 0.917 178.44 1.667 14.97 2.42 8.61
0.250 9.77 1.000 178.44 1.750 12.86 2.50 8.63
0.333 9.77 1.083 37.79 1.833 12.86 2.58 8.00
0.417 12.10 1.167 37.79 1.917 11.36 2.67 8.00
0.500 12.10 1.250 23.83 2.000 11.36 2.75 7.50
0.583 16.55 1.333 23.83 2.083 10.23 2.83 7.50
0.667 16.55 1.417 18.17 2.167 10.23 2.92 7.06
0.750 30.79 1.500 18.17 2.250 9.33 3.00 7.06

Unit Hyd Tpeak (cms)= 0.265
PEAK FLOW (cms)= 0.136 (i)
TIME TO PEAK (hrs)= 1.167
RUNOFF VOLUME (mm)= 28.709
TOTAL RAINFALL (mm)= 70.941
RUNOFF COEFFICIENT = 0.405

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 1.66 0.131 1.17 28.71
OVERFLOW: ID= 3 1.66 0.104 1.42 17.70

Volume Reduction Rate[(Rvin-Rvout)/Rvin] (%)= 38.33
Time to reach Max Ponding Storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 234.26
Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 171.55
Maximum Engineered Soil Volume (cu.m.)= 64.77
**** After simulation, water volume is not zero.

[ Junction Command(0181) ]
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 3 ( 0184) 1.66 0.10 1.42 17.70
OUTFLOW: ID= 2 ( 0181) 1.66 0.10 1.42 17.70

-----
[ CALIB ]
[ STANDHYD ( 0205) ] Area (ha)= 0.80
ID= 1 DT= 5.0 min Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00

Surface Area (ha)= 0.80
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 75.87 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 8.30 0.833 30.79 1.583 14.97 2.33 9.33
0.167 8.30 0.917 178.44 1.667 14.97 2.42 8.61
0.250 9.77 1.000 178.44 1.750 12.86 2.50 8.63
0.333 9.77 1.083 37.79 1.833 12.86 2.58 8.00
0.417 12.10 1.167 37.79 1.917 11.36 2.67 8.00
0.500 12.10 1.250 23.83 2.000 11.36 2.75 7.50
0.583 16.55 1.333 23.83 2.083 10.23 2.83 7.50
0.667 16.55 1.417 18.17 2.167 10.23 2.92 7.06
0.750 30.79 1.500 18.17 2.250 9.33 3.00 7.06

Max.Eff.Inten.(mm/hr)= 178.44 *****
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.72 (ii) 6.09 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.28 0.04 *TOTALS*
TIME TO PEAK (hrs)= 1.00 1.08 1.00 0.310 (iii)
RUNOFF VOLUME (mm)= 69.94 28.02 55.27
TOTAL RAINFALL (mm)= 70.94 70.94 70.94
RUNOFF COEFFICIENT = 0.99 0.39 0.78

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
[ CHICAGO STORM ] IDF curve parameters: Ar= 892.273
[ POTD= 70.94 mm ] B= 0.00000
C= 0.699

-----
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 1.66 0.131 1.17 28.71
OVERFLOW: ID= 3 1.66 0.104 1.42 17.70

Volume Reduction Rate[(Rvin-Rvout)/Rvin] (%)= 38.33
Time to reach Max Ponding Storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 234.26
Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 171.55
Maximum Engineered Soil Volume (cu.m.)= 64.77
**** After simulation, water volume is not zero.

[ Junction Command(0181) ]
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 3 ( 0184) 1.66 0.10 1.42 17.70
OUTFLOW: ID= 2 ( 0181) 1.66 0.10 1.42 17.70

-----
[ CALIB ]
[ STANDHYD ( 0205) ] Area (ha)= 0.80
ID= 1 DT= 5.0 min Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00

Surface Area (ha)= 0.80
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 75.87 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 8.30 0.833 30.79 1.583 14.97 2.33 9.33
0.167 8.30 0.917 178.44 1.667 14.97 2.42 8.61
0.250 9.77 1.000 178.44 1.750 12.86 2.50 8.63
0.333 9.77 1.083 37.79 1.833 12.86 2.58 8.00
0.417 12.10 1.167 37.79 1.917 11.36 2.67 8.00
0.500 12.10 1.250 23.83 2.000 11.36 2.75 7.50
0.583 16.55 1.333 23.83 2.083 10.23 2.83 7.50
0.667 16.55 1.417 18.17 2.167 10.23 2.92 7.06
0.750 30.79 1.500 18.17 2.250 9.33 3.00 7.06

Max.Eff.Inten.(mm/hr)= 178.44 *****
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.72 (ii) 6.09 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.28 0.04 *TOTALS*
TIME TO PEAK (hrs)= 1.00 1.08 1.00 0.310 (iii)
RUNOFF VOLUME (mm)= 69.94 28.02 55.27
TOTAL RAINFALL (mm)= 70.94 70.94 70.94
RUNOFF COEFFICIENT = 0.99 0.39 0.78

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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[ DUHYD ( 0182) ]
[ Inlet Cap.= 0.131 ]
[ Ref Inlets= 1 ]
[ Total(cms)= 0.11 ] AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
TOTAL HYD.(ID= 1): 1.66 0.14 1.17 28.71
MAJOR SYS.(ID= 2): 0.01 0.01 1.17 28.71
MINOR SYS.(ID= 3): 1.66 0.13 1.17 28.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----
[ SLOPE CONTROL ( 0184) ] OUTFLOW: OFF, UNDERDRAIN: OFF
[ Ew 2 -> OUT= 3 ]
[ DT= 5.0 min ]

SURFACE PONDING LAYER:
Max. Ponding Storage(cu.m.)= 149.95

STAGE AREA STAGE AREA
(m) (m2) (m) (m2)
0.000 100.000 | 1.500 100.000
0.500 100.000 | 0.000 0.000
1.000 100.000 | 0.000 0.000

MULCH LAYER:
Depth (m)= 0.10 Porosity = 0.40
Maximum Mulch Layer Volume (cu.m.)= 21.60

ENGINEERED SOIL LAYER:
Soil moisture = 0.30 Depth (m)= 0.72
Width (m)= 100.00 Length (m)= 2.00
Porosity = 0.47 Infiltration (m/hr) = 0.0000
Maximum Engineered Soil Layer Volume(cu.m.)= 181.57

STORAGE LAYER:
Depth (m)= 0.50 Porosity = 0.40
Seepage (m/hr) = 0.0010
Maximum Storage Layer Volume(cu.m.)= 108.00

TOTAL AVAILABLE STORAGE:
TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil
+Storage Layer(cu.m.)= 461.32

NATIVE SOIL LAYER:
Infiltration (m/hr) = 0.0060

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AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 1.66 0.131 1.17 28.71
OVERFLOW: ID= 3 1.66 0.104 1.42 17.70

Volume Reduction Rate[(Rvin-Rvout)/Rvin] (%)= 38.33
Time to reach Max Ponding Storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 234.26
Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 171.55
Maximum Engineered Soil Volume (cu.m.)= 64.77
**** After simulation, water volume is not zero.

[ Junction Command(0181) ]
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 3 ( 0184) 1.66 0.10 1.42 17.70
OUTFLOW: ID= 2 ( 0181) 1.66 0.10 1.42 17.70

-----
[ CALIB ]
[ STANDHYD ( 0205) ] Area (ha)= 0.80
ID= 1 DT= 5.0 min Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00

Surface Area (ha)= 0.80
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 75.87 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 8.30 0.833 30.79 1.583 14.97 2.33 9.33
0.167 8.30 0.917 178.44 1.667 14.97 2.42 8.61
0.250 9.77 1.000 178.44 1.750 12.86 2.50 8.63
0.333 9.77 1.083 37.79 1.833 12.86 2.58 8.00
0.417 12.10 1.167 37.79 1.917 11.36 2.67 8.00
0.500 12.10 1.250 23.83 2.000 11.36 2.75 7.50
0.583 16.55 1.333 23.83 2.083 10.23 2.83 7.50
0.667 16.55 1.417 18.17 2.167 10.23 2.92 7.06
0.750 30.79 1.500 18.17 2.250 9.33 3.00 7.06

Max.Eff.Inten.(mm/hr)= 178.44 *****
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.72 (ii) 6.09 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.28 0.04 *TOTALS*
TIME TO PEAK (hrs)= 1.00 1.08 1.00 0.310 (iii)
RUNOFF VOLUME (mm)= 69.94 28.02 55.27
TOTAL RAINFALL (mm)= 70.94 70.94 70.94
RUNOFF COEFFICIENT = 0.99 0.39 0.78

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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[ CHICAGO STORM ] IDF curve parameters: Ar= 892.273
[ POTD= 70.94 mm ] B= 0.00000
C= 0.699

-----
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 1.66 0.131 1.17 28.71
OVERFLOW: ID= 3 1.66 0.104 1.42 17.70

Volume Reduction Rate[(Rvin-Rvout)/Rvin] (%)= 38.33
Time to reach Max Ponding Storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 234.26
Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 171.55
Maximum Engineered Soil Volume (cu.m.)= 64.77
**** After simulation, water volume is not zero.

[ Junction Command(0181) ]
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 3 ( 0184) 1.66 0.10 1.42 17.70
OUTFLOW: ID= 2 ( 0181) 1.66 0.10 1.42 17.70

-----
[ CALIB ]
[ STANDHYD ( 0205) ] Area (ha)= 0.80
ID= 1 DT= 5.0 min Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00

Surface Area (ha)= 0.80
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 75.87 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.083 8.30 0.833 30.79 1.583 14.97 2.33 9.33
0.167 8.30 0.917 178.44 1.667 14.97 2.42 8.61
0.250 9.77 1.000 178.44 1.750 12.86 2.50 8.63
0.333 9.77 1.083 37.79 1.833 12.86 2.58 8.00
0.417 12.10 1.167 37.79 1.917 11.36 2.67 8.00
0.500 12.10 1.250 23.83 2.000 11.36 2.75 7.50
0.583 16.55 1.333 23.83 2.083 10.23 2.83 7.50
0.667 16.55 1.417 18.17 2.167 10.23 2.92 7.06
0.750 30.79 1.500 18.17 2.250 9.33 3.00 7.06

Max.Eff.Inten.(mm/hr)= 178.44 *****
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.72 (ii) 6.09 (iii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.28 0.04 *TOTALS*
TIME TO PEAK (hrs)= 1.00 1.08 1.00 0.310 (iii)
RUNOFF VOLUME (mm)= 69.94 28.02 55.27
TOTAL RAINFALL (mm)= 70.94 70.94 70.94
RUNOFF COEFFICIENT = 0.99 0.39 0.78

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0282 ) OVERFLOW IS OFF  
ID= 2 -> OUT= 1  
DT= 5.0 min  
OUTFLOW (cms) 0.0000  
STORAGE (ha.-m.) 0.0000  
OUTFLOW (cms) 0.1000  
STORAGE (ha.-m.) 0.0020

AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
INFLOW: ID= 2 ( 0106) 0.508 0.109 1.08 43.43  
OUTFLOW: ID= 1 ( 0282) 0.508 0.098 1.08 43.43

PEAK FLOW REDUCTION (Out/In)(%)= 89.18  
TIME SHIFT OF PEAK FLOW (min)= 0.000  
MAXIMUM STORAGE USED (ha.-m.)= 0.0021

CALIB  
STANDHYD ( 0245 ) Area (ha)= 0.58  
ID= 1 DT= 5.0 min Total Imp(S)= 55.00 Dir. Conn.(%)= 1.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.32 0.26  
Dep. Storage (mm)= 1.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 62.24 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

0.417 12.10 | 1.167 37.79 | 1.917 11.36 | 2.67 8.00  
0.500 12.10 | 1.250 23.83 | 2.000 11.36 | 2.75 7.50  
0.583 16.55 | 1.333 23.83 | 2.083 10.23 | 2.83 7.50  
0.667 16.55 | 1.417 18.17 | 2.167 10.23 | 2.92 7.06  
0.750 30.79 | 1.500 18.17 | 2.250 9.33 | 3.00 7.06

Max.Eff.Inten.(mm/hr)= 178.44 236.75  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.52 (ii) 6.52 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.33 0.14 \*TOTALS\*  
PEAK FLOW (cms)= 0.00 0.12 0.125 (iii)  
TIME TO PEAK (hrs)= 1.00 1.08 1.08  
RUNOFF VOLUME (mm)= 69.94 43.17 43.43  
TOTAL RAINFALL (mm)= 79.94 79.94 79.94  
RUNOFF COEFFICIENT = 0.99 0.61 0.61

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0246 )  
1 + 2 = 3  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID= 1 ( 0245): 0.58 0.125 1.08 43.43  
+ ID= 2 ( 0282): 0.51 0.098 1.08 43.43  
ID = 3 ( 0246): 1.09 0.222 1.08 43.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
NASHYD ( 0302 ) Area (ha)= 0.78 Curve Number (CN)= 78.5  
ID= 1 DT= 5.0 min IA (mm)= 4.00 # of Linear Res.(N)= 3.00  
U.I. Top(hrs)= 4.20

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH  
TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
0.083 8.30 | 0.833 30.79 | 1.583 14.97 | 2.33 9.33  
0.167 8.30 | 0.917 178.44 | 1.667 14.97 | 2.42 8.61  
0.250 9.77 | 1.000 178.44 | 1.750 12.86 | 2.58 8.00  
0.333 9.77 | 1.083 37.79 | 1.833 12.86 | 2.58 8.00  
0.417 12.10 | 1.167 37.79 | 1.917 11.36 | 2.67 8.00  
0.500 12.10 | 1.250 23.83 | 2.000 11.36 | 2.75 7.50  
0.583 16.55 | 1.333 23.83 | 2.083 10.23 | 2.83 7.50  
0.667 16.55 | 1.417 18.17 | 2.167 10.23 | 2.92 7.06  
0.750 30.79 | 1.500 18.17 | 2.250 9.33 | 3.00 7.06

Unit Hyd Qpeak (cms) = 0.149  
PEAK FLOW (cms) = 0.083 (i)  
TIME TO PEAK (hrs) = 1.167  
RUNOFF VOLUME (mm) = 32.468  
TOTAL RAINFALL (mm) = 79.941  
RUNOFF COEFFICIENT = 0.458

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB  
STANDHYD ( 0019 ) Area (ha)= 0.50  
ID= 1 DT= 5.0 min Total Imp(S)= 55.00 Dir. Conn.(%)= 1.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.28 0.22  
Dep. Storage (mm)= 1.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 57.74 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH  
TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
0.083 8.30 | 0.833 30.79 | 1.583 14.97 | 2.33 9.33  
0.167 8.30 | 0.917 178.44 | 1.667 14.97 | 2.42 8.61  
0.250 9.77 | 1.000 178.44 | 1.750 12.86 | 2.58 8.00  
0.333 9.77 | 1.083 37.79 | 1.833 12.86 | 2.58 8.00  
0.417 12.10 | 1.167 37.79 | 1.917 11.36 | 2.67 8.00  
0.500 12.10 | 1.250 23.83 | 2.000 11.36 | 2.75 7.50  
0.583 16.55 | 1.333 23.83 | 2.083 10.23 | 2.83 7.50  
0.667 16.55 | 1.417 18.17 | 2.167 10.23 | 2.92 7.06

0.750 30.79 | 1.500 18.17 | 2.250 9.33 | 3.00 7.06

Max.Eff.Inten.(mm/hr)= 178.44 236.75  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.46 (ii) 6.46 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.33 0.14 \*TOTALS\*  
PEAK FLOW (cms)= 0.00 0.11 0.108 (iii)  
TIME TO PEAK (hrs)= 1.00 1.08 1.08  
RUNOFF VOLUME (mm)= 69.94 43.17 43.43  
TOTAL RAINFALL (mm)= 79.94 79.94 79.94  
RUNOFF COEFFICIENT = 0.99 0.61 0.61

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0248 )  
1 + 2 + 3  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID= 1 ( 0019): 0.50 0.108 1.08 43.43  
+ ID= 2 ( 0302): 0.78 0.083 1.17 32.47  
ID = 3 ( 0248): 1.28 0.188 1.08 36.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

TRANSFORMED HYETOGRAPH  
TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
0.083 8.30 | 0.833 30.79 | 1.583 14.97 | 2.33 9.33  
0.167 8.30 | 0.917 178.44 | 1.667 14.97 | 2.42 8.61  
0.250 9.77 | 1.000 178.44 | 1.750 12.86 | 2.58 8.00  
0.333 9.77 | 1.083 37.79 | 1.833 12.86 | 2.58 8.00  
0.417 12.10 | 1.167 37.79 | 1.917 11.36 | 2.67 8.00  
0.500 12.10 | 1.250 23.83 | 2.000 11.36 | 2.75 7.50  
0.583 16.55 | 1.333 23.83 | 2.083 10.23 | 2.83 7.50  
0.667 16.55 | 1.417 18.17 | 2.167 10.23 | 2.92 7.06  
0.750 30.79 | 1.500 18.17 | 2.250 9.33 | 3.00 7.06

Max.Eff.Inten.(mm/hr)= 178.44 236.75  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.93 (ii) 5.92 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.31 0.15 \*TOTALS\*  
PEAK FLOW (cms)= 0.44 0.05 0.484 (iii)  
TIME TO PEAK (hrs)= 1.00 1.08 1.00  
RUNOFF VOLUME (mm)= 69.94 28.02 57.36  
TOTAL RAINFALL (mm)= 79.94 79.94 79.94  
RUNOFF COEFFICIENT = 0.99 0.39 0.81

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

DHYD ( 0279 )  
INLET Cap= 0.270  
# of Inlets= 1  
TOTAL (cms)= 0.3  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
TOTAL HYD.(ID= 1): 1.28 0.48 1.00 57.36  
MAJOR SYS.(ID= 2): 0.20 0.21 1.00 57.36  
MINOR SYS.(ID= 3): 1.08 0.27 0.92 57.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Junction Command(0303)  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
INFLOW: ID= 0 ( 0279) 0.20 0.21 1.00 57.36  
OUTFLOW: ID= 2 ( 0303) 0.20 0.21 1.00 57.36

OUTLET OFF, UNDERDRAIN: OFF  
ID= 2 -> OUT= 3  
DT= 5.0 min

SURFACE PONDING LAYER:

Max. Ponding Storage(cu.m.)= 149.95

STAGE AREA STAGE AREA  
(m) (m2) (m) (m2)  
0.000 100.000 | 1.500 100.000  
0.500 100.000 | 0.000 0.000  
1.000 100.000 | 0.000 0.000

MULCH LAYER:  
Depth (m)= 0.10 Porosity = 0.40  
Maximum Mulch Layer Volume (cu.m.)= 4.64

ENGINEERED SOIL LAYER:  
Soil moisture = 0.30 Depth (m)= 0.67  
Width (m)= 50.00 Length (m)= 2.00  
Porosity = 0.47 Infiltration (m/hr) = 0.0000  
Maximum Engineered Soil Layer Volume(cu.m.)= 36.30

STORAGE LAYER:  
Depth (m)= 1.50 Porosity = 0.40  
Seepage (m/hr)= 0.0010  
Maximum Storage Layer Volume(cu.m.)= 69.60

TOTAL AVAILABLE STORAGE:  
TOTAL STORAGE+Surface Ponding + Mulch Layer + Engineered soil  
+Storage Layer(cu.m.)= 260.48

NATIVE SOIL LAYER:  
Infiltration (m/hr) = 0.0060

AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)

INFLOW: ID= 2 1.08 0.270 0.92 57.36  
OVERFLOW: ID= 3 1.08 0.270 1.00 42.89

Volume Reduction Rate((RvIn-RvOut)/RvIn) (%)= 25.23  
Time to reach Max Ponding storage (Hr)= 0.00  
Volume of water for drawdown in LID (cu.m.)= 167.37  
Volume of Max. Water Storage (cu.m.)= 0.000  
Maximum Surface Ponding And Mulch Vol(cu.m.)= 154.59  
Maximum Engineered Soil Volume (cu.m.)= 12.86  
\*\*\*\*\* After simulation, water volume is not zero.

Junction Command(0277)  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
INFLOW: ID= 3 ( 0278) 1.08 0.27 1.00 42.89  
OUTFLOW: ID= 2 ( 0277) 1.08 0.27 1.00 42.89

CALIB  
STANDHYD ( 0313 ) Area (ha)= 4.05  
ID= 1 DT= 5.0 min Total Imp(S)= 60.00 Dir. Conn.(%)= 60.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 2.43 1.62  
Dep. Storage (mm)= 5.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 164.25 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

over (min)= 5.00 10.00  
Storage Coeff. (min)= 2.73 (ii) 7.49 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.29 0.13 \*TOTALS\*  
PEAK FLOW (cms)= 0.18 0.20 1.336 (iii)  
TIME TO PEAK (hrs)= 1.00 1.08 1.00  
RUNOFF VOLUME (mm)= 65.94 28.02 50.77  
TOTAL RAINFALL (mm)= 79.94 79.94 79.94  
RUNOFF COEFFICIENT = 0.93 0.39 0.72

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB  
STANDHYD ( 0314 ) Area (ha)= 1.43  
ID= 1 DT= 5.0 min Total Imp(S)= 50.00 Dir. Conn.(%)= 50.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.72 0.72  
Dep. Storage (mm)= 2.00 5.00  
Average Slope (%)= 0.50 2.00  
Length (m)= 97.74 30.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH  
TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
0.083 8.30 | 0.833 30.79 | 1.583 14.97 | 2.33 9.33  
0.167 8.30 | 0.917 178.44 | 1.667 14.97 | 2.42 8.61  
0.250 9.77 | 1.000 178.44 | 1.750 12.86 | 2.58 8.00  
0.333 9.77 | 1.083 37.79 | 1.833 12.86 | 2.58 8.00  
0.417 12.10 | 1.167 37.79 | 1.917 11.36 | 2.67 8.00  
0.500 12.10 | 1.250 23.83 | 2.000 11.36 | 2.75 7.50  
0.583 16.55 | 1.333 23.83 | 2.083 10.23 | 2.83 7.50  
0.667 16.55 | 1.417 18.17 | 2.167 10.23 | 2.92 7.06  
0.750 30.79 | 1.500 18.17 | 2.250 9.33 | 3.00 7.06

Max.Eff.Inten.(mm/hr)= 178.44 236.75  
over (min)= 5.00 10.00

Storage Coeff. (min)= 2.46 (ii) 7.17 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.30 0.14 \*TOTALS\*  
PEAK FLOW (cms)= 0.35 0.09 0.422 (iii)  
TIME TO PEAK (hrs)= 1.00 1.08 1.00  
RUNOFF VOLUME (mm)= 60.94 28.02 40.40  
TOTAL RAINFALL (mm)= 79.94 79.94 79.94  
RUNOFF COEFFICIENT = 0.97 0.39 0.68

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 74.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0310 )  
1 + 2 = 3  
AREA QPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID= 1 ( 0313): 4.05 1.336 1.00 50.77  
+ ID= 2 ( 0314): 1.43 0.422 1.00 40.48  
ID = 3 ( 0310): 5.48 1.758 1.00 50.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
STANDHYD ( 0315 ) Area (ha)= 14.21  
ID= 1 DT= 5.0 min Total Imp(S)= 75.00 Dir. Conn.(%)= 75.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 10.09 4.12  
Dep. Storage (mm)= 1.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 307.76 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH  
TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
0.083 8.30 | 0.833 30.79 | 1.583 14.97 | 2.33 9.33

Table with columns for flow rate, time to peak, runoff volume, and total rainfall. Includes a warning: STORAGE COEFF. IS SMALLER THAN TIME STEP!

Table detailing mulch layer and engineered soil layer properties, including porosity, infiltration, and storage capacity.

Table showing inflow and outflow data for various scenarios, including peak flows and storage coefficients.

Table with columns for peak flow, time to peak, runoff volume, and total rainfall. Includes a warning: STORAGE COEFF. IS SMALLER THAN TIME STEP!

Table detailing mulch layer and engineered soil layer properties, including porosity, infiltration, and storage capacity.

Table showing inflow and outflow data for various scenarios, including peak flows and storage coefficients.

Table with columns for peak flow, time to peak, runoff volume, and total rainfall. Includes a warning: STORAGE COEFF. IS SMALLER THAN TIME STEP!

Table detailing mulch layer and engineered soil layer properties, including porosity, infiltration, and storage capacity.

Table showing inflow and outflow data for various scenarios, including peak flows and storage coefficients.

83978398-3436-4fc2-b953-4d881a1757/71925000
Plotal= 60.13 mm
Comments: G. 2yr 24hr 15min SCS Type II
V V I SSSSS U U A A L (v 6.2-2088)
V V I SS U U A A L
V V I SS U U A A L
W I SSSSS UUUU A A LLLL
OOO TTTT TTTT H H Y Y M M OOO TN
O O T T H H Y Y M M M O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO
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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*
Input filename: C:\Program Files (x86)\VisualInfo\HMO 6.2\VOI\voim.dat
Output:
C:\Users\ksain\AppData\Local\Civil\GIS\4c9a8a70-2b3a-4142-8511-3404f768702\00429
2e0-ee2-47ba-95fc-82dca5d8331\scena
Summary filename:
C:\Users\ksain\AppData\Local\Civil\GIS\4c9a8a70-2b3a-4142-8511-3404f768702\00429
2e0-ee2-47ba-95fc-82dca5d8331\scena

DATE: 08-28-2022 TIME: 02:26:02
USER:
COMMENTS:

Table with 10 columns: HRS, RAIN, TIME, HRS, RAIN, TIME, HRS, RAIN, TIME, RAIN. Contains rainfall data for 6.00 hours.

\*\*\* SIMULATION : G. 2yr 24hr 15min SCS Type II \*\*\*
[ READ STORM ] filename: C:\Users\ksain\AppData\Local\Temp\
[ JUNCTION COMMAND(0185) ]

Unit Hyd. Peak (cms) = 0.265
PEAK FLOW (cms) = 0.080 (1)
TIME TO PEAK (hrs) = 12.333
RUNOFF VOLUME (mm) = 21.454
TOTAL RAINFALL (mm) = 60.130
RUNOFF COEFFICIENT = 0.360
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 10 columns: HRS, RAIN, TIME, HRS, RAIN, TIME, HRS, RAIN, TIME, RAIN. Contains rainfall data for 6.00 hours.

Max. Eff. Inten. (mm/hr)-over (mm) = 73.60 28.92
Storage Coeff. (min) = 2.45 (11) 14.04 (11)
Unit Hyd. Tpeak (min) = 5.00 15.00

Table with 10 columns: HRS, RAIN, TIME, HRS, RAIN, TIME, HRS, RAIN, TIME, RAIN. Contains rainfall data for 6.00 hours.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.
CALIB
NSHWID ( 0185) Area (ha) = 1.66 Curve Number (CN) = 74.4
ID: 1 Dfs 5.0 min Ia (mm) = 4.500 # of Linear Res. (N) = 3.00
U.H. Tp(hrs) = 0.24

Max. Ponding Storage(cu.m.) = 149.95
STAGE AREA (m) (m2) (m) (m2)
0.000 100.000 | 0.000 100.000
0.500 100.000 | 0.000 0.000
1.000 100.000 | 0.000 0.000

STORAGE LAYER:
Depth (m) = 0.50 Porosity = 0.40
Seepage (m/hr) = 0.0010
Maximum Storage Layer Volume(cu.m.) = 108.00

Table with 10 columns: HRS, RAIN, TIME, HRS, RAIN, TIME, HRS, RAIN, TIME, RAIN. Contains rainfall data for 6.00 hours.

Unit Hyd. peak (cms) = 0.30 0.08
PEAK FLOW (cms) = 0.11 0.01
TIME TO PEAK (hrs) = 12.25 22.33
RUNOFF VOLUME (mm) = 59.13 21.05
TOTAL RAINFALL (mm) = 60.13 60.13
RUNOFF COEFFICIENT = 0.98 0.35

Max. Ponding Storage(cu.m.) = 149.95
STAGE AREA (m) (m2) (m) (m2)
0.000 100.000 | 1.500 100.000
0.500 100.000 | 0.000 0.000
1.000 100.000 | 0.000 0.000

Table with 10 columns: HRS, RAIN, TIME, HRS, RAIN, TIME, HRS, RAIN, TIME, RAIN. Contains rainfall data for 6.00 hours.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.
CALIB
STANDHWD ( 0263) Area (ha) = 0.86
ID: 1 Dfs 5.0 min Total Imp(K%) = 65.00 Dir. Conn.(K%) = 65.00

Surface Area (ha) = 0.86
Impervious (i) = 0.86
Average Slope (%) = 1.00 2.00
Length (m) = 75.29 40.80
Numbering n = 0.013 0.250

STORAGE LAYER:
Depth (m) = 1.50 Porosity = 0.40
Seepage (m/hr) = 0.0010
Maximum Storage Layer Volume(cu.m.) = 48.00

Table with 10 columns: HRS, RAIN, TIME, HRS, RAIN, TIME, HRS, RAIN, TIME, RAIN. Contains rainfall data for 6.00 hours.

Unit Hyd. peak (cms) = 0.30 0.08
PEAK FLOW (cms) = 0.11 0.01
TIME TO PEAK (hrs) = 12.25 22.33
RUNOFF VOLUME (mm) = 59.13 21.05
TOTAL RAINFALL (mm) = 60.13 60.13
RUNOFF COEFFICIENT = 0.98 0.35

Max. Ponding Storage(cu.m.) = 149.95
STAGE AREA (m) (m2) (m) (m2)
0.000 100.000 | 1.500 100.000
0.500 100.000 | 0.000 0.000
1.000 100.000 | 0.000 0.000







TABLE WITH 4 COLUMNS: (ha), (cms), (hrs), (mm). Rows include TOTAL HYD., MAJOR SYS., and MINOR SYS. data.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Bioretention( 0065) | OUTFLOW: OFF, UNDERDRAIN: OFF

| Inlet 2 --> OUT= 3 |

SURFACE PONDING LAYER: Max. Ponding Storage( cu.m. ) = 149.95

MULCH LAYER: Depth (m) = 0.10 Porosity = 0.40

ENGINEERED SOIL LAYER: Soil moisture = 0.30 Depth (m) = 0.67

STORAGE LAYER: Depth (m) = 1.50 Porosity = 0.40

TOTAL AVAILABLE STORAGE: TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil

+Storage Layer( cu.m. ) = 1636.46

NATIVE SOIL LAYER: Infiltration ( m/hr ) = 0.0060

TABLE WITH 4 COLUMNS: AREA, QPEAK, TPEAK, R.V. Rows include INFLOW: ID= 2 and OVERFLOW: ID= 3.

Volume Reduction Rate (Rvin-Rvout)/Rvin (%) = 6.27

Time to reach Max Ponding Storage (hr) = 0.80

JUNCTION COMMAND(0063) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

INFLOW : ID= 3( 0065) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

OUTFLOW: ID= 2( 0063) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

\*\*\* W A R N I N G : HYDROGRAPH 0062 ID= 1> IS DRY.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB | AREA (ha) = 6.59

IMPERVIOUS PERVIOUS (%) | 62.78 40.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYDROGRAPH ---

TABLE WITH 4 COLUMNS: AREA, QPEAK, TPEAK, R.V. Rows include INFLOW: ID= 2 and OVERFLOW: ID= 3.

Max. Eff. Intem. (m/hr) = 73.60 110.60

Storage Coeff. (min) = 2.18 (11)

PEAK FLOW (cms) = 0.03 0.05

\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(3) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN = 74.0 Ia = Depg. Storage (Above)

(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0310) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0061) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0061) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0061) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

TOTAL HYD.(ID= 1): 0.59 0.08 12.25 40.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Bioretention( 0329) | OUTFLOW: OFF UNDERDRAIN: ON

| Inlet 2 --> OUT= 3 |

Max. Ponding Storage( cu.m. ) = 149.95

MULCH LAYER: Depth (m) = 0.10 Porosity = 0.40

ENGINEERED SOIL LAYER: Soil moisture = 0.30 Depth (m) = 0.72

STORAGE LAYER: Depth (m) = 1.90 Porosity = 0.40

TOTAL AVAILABLE STORAGE: TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil

+Storage Layer( cu.m. ) = 680.04

NATIVE SOIL LAYER: Infiltration ( m/hr ) = 0.0060

RESERVOIR( 0067) |

OUTFLOW STORAGE | OUTFLOW STORAGE (cms) (ha.m.) (cms) (ha.m.)

INFLOW : ID= 2 ( 0061) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

OUTFLOW: ID= 1 ( 0067) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

PEAK FLOW REDUCTION [out/(in)](%) = 7.20

TIME SHIFT OF PEAK FLOW (min) = 65.00

MAXIMUM STORAGE USED (ha.m.) = 0.5581

\*\*\* W A R N I N G : HYDROGRAPH 0063 HYDROGRAPH 0061

ID= 1 ( 0330): 0.04 0.001 16.25 23.55

+ ID= 2 ( 0331): 0.00 0.000 0.00 0.00

ID= 3 ( 0310): 0.04 0.001 16.25 23.55

TABLE WITH 4 COLUMNS: hrs, mm/hr, hrs, mm/hr. Rows include 0.083, 0.167, 0.250, etc.

Time to reach Max Ponding Storage (hr) = 0.80

JUNCTION COMMAND(0306) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

INFLOW : ID= 2( 0065) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

OUTFLOW: ID= 3( 0063) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

\*\*\* W A R N I N G : HYDROGRAPH 0306 HYDROGRAPH 0063

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB | AREA (ha) = 6.59

IMPERVIOUS PERVIOUS (%) | 62.78 40.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYDROGRAPH ---

TOTAL HYD.(ID= 1): 0.59 0.08 12.25 40.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Bioretention( 0329) | OUTFLOW: OFF UNDERDRAIN: ON

| Inlet 2 --> OUT= 3 |

Max. Ponding Storage( cu.m. ) = 149.95

MULCH LAYER: Depth (m) = 0.10 Porosity = 0.40

ENGINEERED SOIL LAYER: Soil moisture = 0.30 Depth (m) = 0.72

STORAGE LAYER: Depth (m) = 1.90 Porosity = 0.40

TOTAL AVAILABLE STORAGE: TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil

+Storage Layer( cu.m. ) = 680.04

NATIVE SOIL LAYER: Infiltration ( m/hr ) = 0.0060

RESERVOIR( 0067) |

OUTFLOW STORAGE | OUTFLOW STORAGE (cms) (ha.m.) (cms) (ha.m.)

INFLOW : ID= 2 ( 0061) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

OUTFLOW: ID= 1 ( 0067) | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

PEAK FLOW REDUCTION [out/(in)](%) = 7.20

TIME SHIFT OF PEAK FLOW (min) = 65.00

MAXIMUM STORAGE USED (ha.m.) = 0.5581

\*\*\* W A R N I N G : HYDROGRAPH 0331 ID= 2 IS DRY.

ID= 1 ( 0330): 0.04 0.001 16.25 23.55

+ ID= 2 ( 0331): 0.00 0.000 0.00 0.00

ID= 3 ( 0310): 0.04 0.001 16.25 23.55



NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: ADD HYD (0243), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID# 1, 2, 3 and a summary row.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: ADD HYD (0243), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID# 1, 2, 3 and a summary row.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: RESERVOIR (0188), OVERFLOW IS OFF, DT=2-->OUT=1, OUTFLOW STORAGE, OUTFLOW STORAGE.

Table with columns: INFLOW: ID# 2 (0243), ID# 1 (0188), AREA, QPEAK, TPEAK, R.V.

PEAK FLOW REDUCTION [Qout/Qin] (%) = 80.96, TIME SHIFT OF PEAK FLOW (min) = 0.00, MAXIMUM STORAGE USED (ha.m.) = 0.0061

Table with columns: CALIB, STANDHYD (0252), Area (ha), Total Imp(S) = 69.00, Dir. Conn.(%) = 30.00

IMPERVIOUS PERVIOUS (I)

Surface Area (ha) = 0.22, Dep. Storage (mm) = 1.00, Average Slope (%) = 1.00, Length (m) = 45.84, Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Large table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Contains multiple rows of rainfall data.

3.250 1.04 | 9.333 2.55 | 15.417 2.39 | 21.50 0.96

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Large table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Contains multiple rows of rainfall data.

Max.Eff.Inten.(mm/hr) = 97.49, over (min) = 5.00, Storage Coeff. (min) = 1.62 (ii), Unit Hyd. Tpeak (mm) = 5.00, Unit Hyd. peak (cms) = 0.32, 0.13

PEAK FLOW (cms) = 0.03, 0.04, TIME TO PEAK (hrs) = 12.25, 12.25, RUNOFF VOLUME (mm) = 78.65, 51.27, 59.47

TOTAL RAINFALL (mm) = 79.65, 79.65, 79.65, RUMOFF COEFFICIENT = 0.99, 0.64, 0.75

\*\*\*WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN = 74.0, Ia = Dep. Storage (Above)

(II) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD (0179), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID# 1, 2, 3 and a summary row.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANDHYD (0186), Area (ha), Total Imp(S) = 55.00, Dir. Conn.(%) = 1.00

IMPERVIOUS PERVIOUS (I)

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Mannings n

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Contains multiple rows of rainfall data.

3.167 0.88 | 9.250 1.43 | 13.333 4.30 | 19.42 1.43

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Large table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Contains multiple rows of rainfall data.

Max.Eff.Inten.(mm/hr) = 97.49, over (min) = 5.00, Storage Coeff. (min) = 1.62 (ii), Unit Hyd. Tpeak (mm) = 5.00, Unit Hyd. peak (cms) = 0.32, 0.13

PEAK FLOW (cms) = 0.00, 0.08, 0.083 (III), TIME TO PEAK (hrs) = 12.25, 12.25, 12.25, RUNOFF VOLUME (mm) = 78.65, 50.76, 51.03

TOTAL RAINFALL (mm) = 79.65, 79.65, 79.65, RUMOFF COEFFICIENT = 0.99, 0.64, 0.64

\*\*\*WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

\*\*\*WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN = 74.0, Ia = Dep. Storage (Above)

(II) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD (0179), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID# 1, 2, 3 and a summary row.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANDHYD (0186), Area (ha), Total Imp(S) = 55.00, Dir. Conn.(%) = 1.00

IMPERVIOUS PERVIOUS (I)

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Mannings n

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Contains multiple rows of rainfall data.

Max.Eff.Inten.(mm/hr) = 97.49, over (min) = 5.00, Storage Coeff. (min) = 1.62 (ii), Unit Hyd. Tpeak (mm) = 5.00, Unit Hyd. peak (cms) = 0.32, 0.13

PEAK FLOW (cms) = 0.00, 0.08, 0.083 (III), TIME TO PEAK (hrs) = 12.25, 12.25, 12.25, RUNOFF VOLUME (mm) = 78.65, 50.76, 51.03

TOTAL RAINFALL (mm) = 79.65, 79.65, 79.65, RUMOFF COEFFICIENT = 0.99, 0.64, 0.64

\*\*\*WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

\*\*\*WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN = 74.0, Ia = Dep. Storage (Above)

(II) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD (0243), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID# 1, 2, 3 and a summary row.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANDHYD (0252), Area (ha), Total Imp(S) = 69.00, Dir. Conn.(%) = 30.00

IMPERVIOUS PERVIOUS (I)

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Mannings n

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Contains multiple rows of rainfall data.

3.167 0.88 | 9.250 1.43 | 13.333 4.30 | 19.42 1.43

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Large table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Contains multiple rows of rainfall data.

Max.Eff.Inten.(mm/hr) = 97.49, over (min) = 5.00, Storage Coeff. (min) = 1.94 (ii), Unit Hyd. Tpeak (mm) = 1.00, Unit Hyd. peak (cms) = 0.31, 0.13

PEAK FLOW (cms) = 0.00, 0.00, 0.095 (III)

(I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN = 74.0, Ia = Dep. Storage (Above)

(II) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD (0243), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID# 1, 2, 3 and a summary row.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANDHYD (0270), Area (ha), Total Imp(S) = 70.00, Dir. Conn.(%) = 70.00

IMPERVIOUS PERVIOUS (I)

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Mannings n

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Contains multiple rows of rainfall data.

Max.Eff.Inten.(mm/hr) = 97.49, over (min) = 5.00, Storage Coeff. (min) = 1.94 (ii), Unit Hyd. Tpeak (mm) = 1.00, Unit Hyd. peak (cms) = 0.31, 0.13

PEAK FLOW (cms) = 0.00, 0.00, 0.095 (III)

Hydrograph data table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes summary statistics like Max. Eff. Inten., Storage Coeff., and various flow metrics.

Hydrograph data table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes summary statistics like Max. Eff. Inten., Storage Coeff., and various flow metrics.

Hydrograph data table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes summary statistics like Max. Eff. Inten., Storage Coeff., and various flow metrics.

Hydrograph data table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes summary statistics like Max. Eff. Inten., Storage Coeff., and various flow metrics.

Hydrograph data table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes summary statistics like Max. Eff. Inten., Storage Coeff., and various flow metrics.

Hydrograph data table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes summary statistics like Max. Eff. Inten., Storage Coeff., and various flow metrics.

Hydrograph data table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes summary statistics like Max. Eff. Inten., Storage Coeff., and various flow metrics.

Hydrograph data table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes summary statistics like Max. Eff. Inten., Storage Coeff., and various flow metrics.

Hydrograph data table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Includes summary statistics like Max. Eff. Inten., Storage Coeff., and various flow metrics.

MINOR SYS.(ID= 3): 14.21 2.95 12.25 65.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

DisRetention( 0065) | OUTFLOW: OFF, UNDERDRAIN: OFF
| I= 2 -> OUT= 3
| DT= 5.0 min

SURFACE PONDING LAYER:

Max. Ponding Storage(cu.m.)= 149.95

Table with columns: STAGE, AREA, STAGE, AREA. Values range from 0.00 to 1.00 and 0.000 to 100.000.

MULCH LAYER:
Depth (m)= 0.10 Porosity = 0.40
Maximum Mulch Layer Volume (cu.m.)= 62.40

ENGINEERED SOIL LAYER:
Soil moisture = 0.30 Depth (m)= 0.67
Width (m)= 780.00 Length (m)= 2.00
Porosity = 0.47 Infiltration (m/hr)= 0.0060
Maximum Engineered Soil Layer Volume(cu.m.)= 488.11

STORAGE LAYER:
Depth (m)= 1.50 Porosity = 0.40
Seepage (m/hr)= 0.0010
Maximum Storage Layer Volume(cu.m.)= 936.00

TOTAL AVAILABLE STORAGE:

\*Storage Layer(cu.m.)= 1636.46

NATIVE SOIL LAYER:
Infiltration (m/hr)= 0.0000

Table with columns: AREA, QPEAK, TPEAK, R.V., INFLOW: ID= 2, OVERFLOW: ID= 3. Values include 14.21, 2.948, 12.25, 65.70, 62.67.

Volume Reduction Rate([RvIn-RVout]/RvIn) (%) = 4.62
Time to reach Max Ponding storage (hr) = 0.00
Volume of water for drawdown in LID (cu.m.) = 538.12

Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 212.35
Maximum Engineered Soil Volume (cu.m.)= 330.25
\*\*\*\* After simulation, water volume is not zero.

[ Junction Command(0063) ]

Table with columns: INFLOW: ID= 3 (0065), OUTFLOW: ID= 2 (0063). Values include 14.21, 2.94, 12.25, 62.67.

[ ADD HYD ( 0306) ]
| 1 + 2 + 3

Table with columns: AREA, QPEAK, TPEAK, R.V., INFLOW: ID= 1 (0062), + ID2= 2 ( 0063), ID= 3 ( 0306).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ STANDBY ( 0323) ]

Inflow: ID= 1 DT= 5.0 min | Total Imp(%)= 66.00 Dir. Conn.(%)= 25.00

Table with columns: AREA, QPEAK, TPEAK, R.V., INFLOW: ID= 1, OVERFLOW: ID= 2.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Surface Area (ha)= 0.39 0.20
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 62.78 40.00
Manings n = 0.178 0.250

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN. Values include 0.003, 0.167, 6.260, 1.27, 12.333, 11.48, 18.42, 1.43, 0.250.

0.331 0.88 | 6.417 1.43 | 12.500 11.47 | 18.58 1.43
0.417 0.88 | 6.500 1.43 | 12.583 11.47 | 18.67 1.43
0.500 0.88 | 6.583 1.43 | 12.667 11.47 | 18.76 1.43
0.583 0.88 | 6.667 1.43 | 12.750 11.47 | 18.83 1.43
0.667 0.88 | 6.750 1.43 | 12.833 5.89 | 18.92 1.43
0.750 0.88 | 6.833 1.43 | 12.917 5.89 | 19.00 1.43
0.833 0.88 | 6.917 1.43 | 13.000 5.89 | 19.08 1.43
0.917 0.88 | 7.000 1.43 | 13.083 5.89 | 19.17 1.43
1.000 0.88 | 7.083 1.43 | 13.167 5.89 | 19.25 1.43
1.083 0.88 | 7.167 1.43 | 13.250 5.89 | 19.33 1.43
1.167 0.88 | 7.250 1.43 | 13.333 4.30 | 19.42 1.43
1.250 0.88 | 7.333 1.75 | 13.417 4.30 | 19.50 1.43
1.333 0.88 | 7.417 1.75 | 13.500 4.30 | 19.58 1.43
1.417 0.88 | 7.500 1.75 | 13.583 4.30 | 19.67 1.43
1.500 0.88 | 7.583 1.75 | 13.667 4.30 | 19.75 1.43
1.583 0.88 | 7.667 1.75 | 13.750 4.30 | 19.83 1.43
1.667 0.88 | 7.750 1.75 | 13.833 3.39 | 19.92 1.43
1.750 0.88 | 7.833 1.75 | 13.917 3.35 | 20.00 1.43
1.833 0.88 | 7.917 1.75 | 14.000 3.35 | 20.08 1.43
1.917 0.88 | 8.000 1.75 | 14.083 3.35 | 20.17 1.43
2.000 0.88 | 8.083 1.75 | 14.167 3.35 | 20.25 1.43
2.083 0.88 | 8.167 1.75 | 14.250 3.35 | 20.33 0.96
2.167 0.88 | 8.250 1.75 | 14.333 2.39 | 20.42 0.96
2.250 0.88 | 8.333 2.07 | 14.417 2.39 | 20.50 0.96
2.333 1.04 | 8.417 2.07 | 14.500 2.39 | 20.58 0.96
2.417 1.04 | 8.500 2.07 | 14.583 2.39 | 20.67 0.96
2.500 1.04 | 8.583 2.07 | 14.667 2.39 | 20.75 0.96
2.583 1.04 | 8.667 2.07 | 14.750 2.39 | 20.83 0.96
2.667 1.04 | 8.750 2.07 | 14.833 2.39 | 20.92 0.96
2.750 1.04 | 8.833 2.23 | 14.917 2.39 | 21.00 0.96
2.833 1.04 | 8.917 2.23 | 15.000 2.39 | 21.08 0.96
2.917 1.04 | 9.000 2.23 | 15.083 2.39 | 21.17 0.96
3.000 1.04 | 9.083 2.23 | 15.167 2.39 | 21.25 0.96
3.083 1.04 | 9.167 2.23 | 15.250 2.39 | 21.33 0.96
3.167 1.04 | 9.250 2.23 | 15.333 2.39 | 21.42 0.96
3.250 1.04 | 9.333 2.55 | 15.417 2.39 | 21.50 0.96
3.333 1.04 | 9.417 2.55 | 15.500 2.39 | 21.58 0.96
3.417 1.04 | 9.500 2.55 | 15.583 2.39 | 21.67 0.96
3.500 1.04 | 9.583 2.55 | 15.667 2.39 | 21.75 0.96
3.583 1.04 | 9.667 2.55 | 15.750 2.39 | 21.83 0.96
3.667 1.04 | 9.750 2.55 | 15.833 2.39 | 21.92 0.96
3.750 1.04 | 9.833 2.87 | 15.917 2.39 | 22.00 0.96
3.833 1.04 | 9.917 2.87 | 16.000 2.39 | 22.08 0.96
3.917 1.04 | 10.000 2.87 | 16.083 2.39 | 22.17 0.96
4.000 1.04 | 10.083 2.87 | 16.167 2.39 | 22.25 0.96
4.083 1.04 | 10.167 2.87 | 16.250 2.39 | 22.33 0.96
4.167 1.04 | 10.250 2.87 | 16.333 1.43 | 22.42 0.96
4.250 1.04 | 10.333 3.66 | 16.417 1.43 | 22.50 0.96
4.333 1.27 | 10.417 3.66 | 16.500 1.43 | 22.58 0.96
4.417 1.27 | 10.500 3.66 | 16.583 1.43 | 22.67 0.96

Table with 5 columns: Area, QPeak, TPeak, R.V., Inflow/Outflow. Values range from 4.500 to 6.083 and 1.27 to 12.167.

Max.Eff.Inten.(mm/hr)= 97.49 163.31
over (min) 5.00 10.00
Storage Coeff. (min) 1.95 (iii) 7.75 (ii)
Unit Hyd. Tpeak (min) 5.00 10.00
Unit Hyd. peak (cms) 0.31 0.13
PEAK FLOW (cms) 0.04 0.07 0.12 (iii)
TIME TO PEAK (hrs) 12.25 12.25 12.25
RUNOFF VOLUME (mm) 78.65 80.82 57.77
TOTAL RAINFALL (mm) 79.65 79.65 79.65
RUNOFF COEFFICIENT = 0.99 0.64 0.73

(1) CH PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 74.0 Ia = Dep. Storage (Above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: DUND ( 0331), Inlet Cap= 0.112, # of Inlets= 1, Total(cms)= 0.11. Values include 0.59, 0.11, 12.25, 57.77.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ ADD HYD ( 0061) ]
| 1 + 2 + 3
| AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 ( 0276): 1.28 0.270 12.25 65.25
| + ID2= 2 ( 0304): 5.48 1.011 12.25 57.72
| ID= 3 ( 0061): 6.76 1.282 12.25 59.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ ADD HYD ( 0061) ]
| 1 + 2 + 3
| AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 ( 0061): 6.76 1.282 12.25 59.14
| + ID2= 2 ( 0306): 14.21 2.945 12.25 62.67
| ID= 3 ( 0061): 20.97 4.227 12.25 61.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ ADD HYD ( 0061) ]
| 1 + 2 + 3
| AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 ( 0061): 20.97 4.227 12.25 61.53
| + ID2= 2 ( 0310): 0.59 0.11 12.42 40.60
| ID= 3 ( 0061): 21.56 4.228 12.25 60.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ RESERVOIR( 0067) ] OVERFLOW IS OFF
| I= 2 -> OUT= 1
| DT= 5.0 min | OUTFLOW STORAGE | OUTFLOW STORAGE

MAJOR SYS.(ID= 2): 0.00 0.00 0.00 0.00
MINOR SYS.(ID= 3): 0.59 0.11 12.25 57.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

DisRetention( 0329) | OUTFLOW: OFF UNDERDRAIN: ON
| I= 2 -> OUT= 3
| DT= 5.0 min

SURFACE PONDING LAYER:

Max. Ponding Storage(cu.m.)= 149.95

Table with columns: STAGE, AREA, STAGE, AREA. Values range from 0.000 to 1.000 and 0.000 to 100.000.

MULCH LAYER:
Depth (m)= 0.00 Porosity = 0.40
Maximum Mulch Layer Volume (cu.m.)= 0.00

ENGINEERED SOIL LAYER:
Soil moisture = 0.30 Depth (m)= 0.72
Width (m)= 3.00 Length (m)=240.00
Porosity = 0.47 Infiltration (m/hr)= 0.0060
Maximum Engineered Soil Layer Volume(cu.m.)= 242.09

DEPTH DISCHARGE | DEPTH DISCHARGE
(m) (cms) | (m) (cms)
0.000 0.000 | 0.559 0.030
0.279 0.010 | 0.762 0.040
0.406 0.020 | 1.300 0.050

STORAGE LAYER:
Depth (m)= 1.00 Porosity = 0.40
Seepage (m/hr)= 0.0010
Maximum Storage Layer Volume(cu.m.)= 288.00

TOTAL AVAILABLE STORAGE:

\*Storage Layer(cu.m.)= 690.04

NATIVE SOIL LAYER:
Infiltration (m/hr)= 0.0060

Table with columns: AREA, QPEAK, TPEAK, R.V., INFLOW: ID= 1 ( 0329), OUTFLOW: ID= 2 ( 0332).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: (cms) (ha.m.), (cms) (ha.m.). Values include 0.0000, 0.0000, 0.6610, 0.9850, 0.0400, 0.1140, 1.0960, 1.3320, 0.0760, 0.3050, 1.4260, 1.5900, 0.2810, 0.6450, 0.0000, 0.0000.

INFLOW: ID= 2 ( 0061) 21.558 4.228 12.25 60.96
OUTFLOW: ID= 1 ( 0067) 21.558 0.401 12.92 60.92

PEAK FLOW REDUCTION [out/QIn](%)= 9.47
TIME SHIFT OF PEAK FLOW (min)= 40.00
MAXIMUM STORAGE USED (ha.m.)= 0.7520

Table with columns: AREA, QPEAK, TPEAK, R.V., INFLOW: ID= 1 ( 0310), + ID2= 2 ( 0311).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: AREA, QPEAK, TPEAK, R.V., INFLOW: ID= 1 ( 0310), + ID2= 2 ( 0311).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: AREA, QPEAK, TPEAK, R.V., INFLOW: ID= 1 ( 0310), + ID2= 2 ( 0311).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

INFLOW: ID= 2 (ha) (cms) (hrs) (mm)
OUTFLOW: ID= 1 0.33 0.009 0.08 40.60
OVERFLOW: ID= 3 0.26 0.034 12.42 40.60

Volume Reduction Rate([RvIn-RVout]/RvIn) (%)= 29.72
Time to reach Max Ponding storage (hr)= 0.00
Volume of water for drawdown in LID (cu.m.)= 156.38
Volume of Max. Water Storage (cu.m.)= 0.00
Maximum Surface Ponding And Mulch Vol(cu.m.)= 149.95
Maximum Engineered Soil Volume (cu.m.)= 69.86
\*\*\*\* After simulation, water volume is not zero.

[ Junction Command(0310) ]

Table with columns: AREA, QPEAK, TPEAK, R.V., INFLOW: ID= 1 ( 0329), OUTFLOW: ID= 2 ( 0332).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ Junction Command(0322) ]

Table with columns: AREA, QPEAK, TPEAK, R.V., INFLOW: ID= 1 ( 0329), OUTFLOW: ID= 2 ( 0332).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ ADD HYD ( 0310) ]
| 1 + 2 + 3
| AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| \*\*\* M A R N I N G : HYDROGRAPH 0311 (ID= 2) IS DRY.
| \*\*\* M A R N I N G : HYDROGRAPH 0063 = HYDROGRAPH 0065
| ID1= 1 ( 0310): 0.26 0.034 12.42 40.60
| + ID2= 2 ( 0311): 0.00 0.000 0.00 0.00
| ID= 3 ( 0310): 0.26 0.034 12.42 40.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ ADD HYD ( 0310) ]

Table with columns: AREA, QPEAK, TPEAK, R.V., INFLOW: ID= 1 ( 0329), OUTFLOW: ID= 2 ( 0332).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.



NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: ADD HYD, Area, QPEAK, TPEAK, R.V., ID#1, ID#2, ID#3. Values for Area (ha) and QPEAK (cms) are provided for three different scenarios.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: ADD HYD, Area, QPEAK, TPEAK, R.V., ID#1, ID#2, ID#3. Values for Area (ha) and QPEAK (cms) are provided for three different scenarios.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: RESERVOIR, OVERFLOW IS OFF, DT=2.0 min, OUTFLOW STORAGE, INFLOW: ID#2, ID#1. Values for storage and flow are provided.

PEAK FLOW REDUCTION [Qout/Qin](%) = 86.03
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 0.0087

Table with columns: CALIB, STANHYD, Area, DT=5.0 min, Total Imp, Dir. Conn. Values for calibration and stormwater management parameters.

Table with columns: Surface Area, Dep. Storage, Average Slope, Length, Mannings n. Values for catchment characteristics.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Values for rainfall intensity over time.

Table with columns: Area, QPEAK, TPEAK, R.V., ID#1, ID#2, ID#3. Values for catchment characteristics.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Values for rainfall intensity over time.

Table with columns: Max.Eff.Inten, over, Storage Coeff, Unit Hyd, Tpeak, Unit Hyd, peak. Values for hydraulic performance metrics.

Table with columns: PEAK FLOW, TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, RUMOFF COEFFICIENT. Values for runoff characteristics.

\*\*\*WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD, Area, QPEAK, TPEAK, R.V., ID#1, ID#2, ID#3. Values for catchment characteristics.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANHYD, Area, DT=5.0 min, Total Imp, Dir. Conn. Values for calibration and stormwater management parameters.

IMPERVIOUS PERVIOUS (I)
Surface Area, Dep. Storage, Average Slope, Length, Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Values for rainfall intensity over time.

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD, Area, QPEAK, TPEAK, R.V., ID#1, ID#2, ID#3. Values for catchment characteristics.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANHYD, Area, DT=5.0 min, Total Imp, Dir. Conn. Values for calibration and stormwater management parameters.

IMPERVIOUS PERVIOUS (I)
Surface Area, Dep. Storage, Average Slope, Length, Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Values for rainfall intensity over time.

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD, Area, QPEAK, TPEAK, R.V., ID#1, ID#2, ID#3. Values for catchment characteristics.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANHYD, Area, DT=5.0 min, Total Imp, Dir. Conn. Values for calibration and stormwater management parameters.

IMPERVIOUS PERVIOUS (I)
Surface Area, Dep. Storage, Average Slope, Length, Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Values for rainfall intensity over time.

Table with columns: CALIB, STANHYD, Area, DT=5.0 min, Total Imp, Dir. Conn. Values for calibration and stormwater management parameters.

IMPERVIOUS PERVIOUS (I)
Surface Area, Dep. Storage, Average Slope, Length, Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Values for rainfall intensity over time.

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD, Area, QPEAK, TPEAK, R.V., ID#1, ID#2, ID#3. Values for catchment characteristics.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANHYD, Area, DT=5.0 min, Total Imp, Dir. Conn. Values for calibration and stormwater management parameters.

IMPERVIOUS PERVIOUS (I)
Surface Area, Dep. Storage, Average Slope, Length, Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Values for rainfall intensity over time.

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD, Area, QPEAK, TPEAK, R.V., ID#1, ID#2, ID#3. Values for catchment characteristics.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANHYD, Area, DT=5.0 min, Total Imp, Dir. Conn. Values for calibration and stormwater management parameters.

IMPERVIOUS PERVIOUS (I)
Surface Area, Dep. Storage, Average Slope, Length, Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Values for rainfall intensity over time.

Table with columns: Max.Eff.Inten, over, Storage Coeff, Unit Hyd, Tpeak, Unit Hyd, peak. Values for hydraulic performance metrics.

Table with columns: PEAK FLOW, TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, RUMOFF COEFFICIENT. Values for runoff characteristics.

Table with 5 columns: Runoff Volume, Peak Flow, Time to Peak, etc. for various scenarios. Includes a table for 'Unit Hyd. Peak (cms)' and a section for 'CALC | STORMHD (0019) |' with parameters like Area, Impervious, Pervious, etc.

Table with 5 columns: Runoff Volume, Peak Flow, Time to Peak, etc. for various scenarios. Includes a section for 'CALC | STORMHD (0019) |' and a section for 'CALC | STORMHD (0216) |' with parameters like Area, Impervious, Pervious, etc.

Table with 5 columns: Runoff Volume, Peak Flow, Time to Peak, etc. for various scenarios. Includes a section for 'CALC | STORMHD (0216) |' and a section for 'CALC | STORMHD (0273) |' with parameters like Area, Impervious, Pervious, etc.

Table with 5 columns: Runoff Volume, Peak Flow, Time to Peak, etc. for various scenarios. Includes a section for 'CALC | STORMHD (0273) |' and a section for 'CALC | STORMHD (0248) |' with parameters like Area, Impervious, Pervious, etc.

Table with 5 columns: Runoff Volume, Peak Flow, Time to Peak, etc. for various scenarios. Includes a section for 'CALC | STORMHD (0273) |' and a section for 'CALC | STORMHD (0273) |' with parameters like Area, Impervious, Pervious, etc.

Table with 5 columns: Runoff Volume, Peak Flow, Time to Peak, etc. for various scenarios. Includes a section for 'CALC | STORMHD (0273) |' and a section for 'CALC | STORMHD (0273) |' with parameters like Area, Impervious, Pervious, etc.

Table with 5 columns: Runoff Volume, Peak Flow, Time to Peak, etc. for various scenarios. Includes a section for 'CALC | STORMHD (0273) |' and a section for 'CALC | STORMHD (0273) |' with parameters like Area, Impervious, Pervious, etc.

Table with 5 columns: Runoff Volume, Peak Flow, Time to Peak, etc. for various scenarios. Includes a section for 'CALC | STORMHD (0273) |' and a section for 'CALC | STORMHD (0273) |' with parameters like Area, Impervious, Pervious, etc.

Table with 5 columns: Runoff Volume, Peak Flow, Time to Peak, etc. for various scenarios. Includes a section for 'CALC | STORMHD (0273) |' and a section for 'CALC | STORMHD (0273) |' with parameters like Area, Impervious, Pervious, etc.





|       |      |        |        |         |      |       |      |
|-------|------|--------|--------|---------|------|-------|------|
| 3.833 | 1.20 | 9.917  | 3.34   | 116.000 | 2.78 | 22.08 | 1.11 |
| 3.917 | 1.20 | 10.000 | 3.34   | 116.083 | 2.78 | 22.17 | 1.11 |
| 4.000 | 1.20 | 10.083 | 3.34   | 116.167 | 2.78 | 22.25 | 1.11 |
| 4.083 | 1.20 | 10.167 | 3.34   | 116.250 | 2.78 | 22.33 | 1.11 |
| 4.167 | 1.20 | 10.250 | 3.34   | 116.333 | 1.67 | 22.42 | 1.11 |
| 4.250 | 1.20 | 10.333 | 4.26   | 116.417 | 1.67 | 22.50 | 1.11 |
| 4.333 | 1.48 | 10.417 | 4.26   | 116.500 | 1.67 | 22.58 | 1.11 |
| 4.417 | 1.48 | 10.500 | 4.26   | 116.583 | 1.67 | 22.67 | 1.11 |
| 4.500 | 1.48 | 10.583 | 4.26   | 116.667 | 1.67 | 22.75 | 1.11 |
| 4.583 | 1.48 | 10.667 | 4.26   | 116.750 | 1.67 | 22.83 | 1.11 |
| 4.667 | 1.48 | 10.750 | 4.26   | 116.833 | 1.67 | 22.92 | 1.11 |
| 4.750 | 1.48 | 10.833 | 5.74   | 116.917 | 1.67 | 23.00 | 1.11 |
| 4.833 | 1.48 | 10.917 | 5.74   | 117.000 | 1.67 | 23.08 | 1.11 |
| 4.917 | 1.48 | 11.000 | 5.74   | 117.083 | 1.67 | 23.17 | 1.11 |
| 5.000 | 1.48 | 11.083 | 5.74   | 117.167 | 1.67 | 23.25 | 1.11 |
| 5.083 | 1.48 | 11.167 | 5.74   | 117.250 | 1.67 | 23.33 | 1.11 |
| 5.167 | 1.48 | 11.250 | 5.74   | 117.333 | 1.67 | 23.42 | 1.11 |
| 5.250 | 1.48 | 11.333 | 8.90   | 117.417 | 1.67 | 23.50 | 1.11 |
| 5.333 | 1.48 | 11.417 | 8.90   | 117.500 | 1.67 | 23.58 | 1.11 |
| 5.417 | 1.48 | 11.500 | 8.90   | 117.583 | 1.67 | 23.67 | 1.11 |
| 5.500 | 1.48 | 11.583 | 8.90   | 117.667 | 1.67 | 23.75 | 1.11 |
| 5.583 | 1.48 | 11.667 | 8.90   | 117.750 | 1.67 | 23.83 | 1.11 |
| 5.667 | 1.48 | 11.750 | 8.90   | 117.833 | 1.67 | 23.92 | 1.11 |
| 5.750 | 1.48 | 11.833 | 27.43  | 117.917 | 1.67 | 24.00 | 1.11 |
| 5.833 | 1.48 | 11.917 | 27.43  | 118.000 | 1.67 | 24.08 | 1.11 |
| 5.917 | 1.48 | 12.000 | 27.43  | 118.083 | 1.67 | 24.17 | 1.11 |
| 6.000 | 1.48 | 12.083 | 113.41 | 118.167 | 1.67 | 24.25 | 1.11 |
| 6.083 | 1.48 | 12.167 | 113.41 | 118.250 | 1.67 |       |      |

Max. Eff. Inten. (m/hr) = 113.42 199.26  
 over (min) = 5.00 10.00  
 Storage Coeff. (min) = 1.84 (11) 7.19 (11)  
 Unit Hyd. Tpeak (min) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.32 0.14

\*TOTALS\*  
 PEAK FLOW (cms) = 0.05 0.09 0.137 (111)  
 TIME TO PEAK (hrs) = 12.25 12.25 12.25  
 RUNOFF VOLUME (mm) = 91.66 62.44 69.74  
 TOTAL RAINFALL (mm) = 92.66 92.66 92.66  
 RUNOFF COEFFICIENT = 0.90 0.67 0.75

\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN = 74.0 Ia = Dep. Storage (Above)  
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
 DUHYD ( 0331)  
 Inlet Cap. = 0.132  
 # of Inlets = 1  
 Total (cms) = 0.1 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 TOTAL HYD. (ID= 1): 0.59 0.14 12.25 69.74  
 MAJOR SVS. (ID= 2): 0.01 0.02 12.25 69.74  
 MINOR SVS. (ID= 3): 0.58 0.11 12.17 69.74  
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 [Retention ( 0329)] OUTFLOW: OFF UNDERDRAIN: ON  
 [In 2--> OUT= 3 ]  
 DT= 5.0 min  
 SURFACE PONDING LAYER:  
 Max. Ponding Storage (cu.m.) = 149.95  
 STAGE AREA STAGE AREA  
 (m) (m2) (m) (m2)  
 0.000 100.000 1.500 100.000  
 0.500 100.000 1.000 0.000  
 1.000 100.000 0.000 0.000  
 MULCH LAYER:  
 Depth (m) = 0.00 Porosity = 0.40  
 Maximum Mulch Layer Volume (cu.m.) = 0.00  
 ENGINEERED SOIL LAYER:  
 Soil moisture = 0.30 Depth (m) = 0.72  
 Width (m) = 3.00 Length (m) = 240.00  
 Porosity = 0.47 Infiltration (m/hr) = 0.0060  
 Maximum Engineered Soil Layer Volume (cu.m.) = 242.09  
 DEPTH DISCHARGE DEPTH DISCHARGE  
 (m) (cms) (m) (cms)  
 0.000 0.000 1.500 0.030  
 0.279 0.010 1.762 0.040  
 0.406 0.020 1.900 0.050  
 STORAGE LAYER:  
 Depth (m) = 1.00 Porosity = 0.40  
 Seepage (m/hr) = 0.0010  
 Maximum Storage Layer Volume (cu.m.) = 288.00  
 TOTAL AVAILABLE STORAGE:

-----  
 TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil  
 +Storage Layer (cu.m.) = 680.04  
 NATIVE SOIL LAYER:  
 Infiltration (m/hr) = 0.0060  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 INFLOW: ID= 2 0.58 0.112 12.17 69.74  
 OUTFLOW: ID= 1 0.27 0.009 0.08 52.19  
 OVERFLOW: ID= 3 0.31 0.089 12.33 52.19  
 Volume Reduction Rate (RvIn-RvOut)/RvIn (%) = 25.17  
 Time to reach Max Ponding Storage (hr) = 0.00  
 Volume of water for drawdown in IID (cu.m.) = 156.45  
 Volume of Max. Water Storage (cu.m.) = 0.00  
 Maximum Surface Ponding and Mulch Vol. (cu.m.) = 149.95  
 Maximum Engineered Soil Volume (cu.m.) = 69.86  
 \*\*\*\* After simulation, water volume is not zero.

-----  
 [ Junction Command (0330) ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 INFLOW : ID= 3( 0329) 0.31 0.09 12.33 52.19  
 OUTFLOW: ID= 2( 0330) 0.31 0.09 12.33 52.19

-----  
 [ Junction Command (0332) ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 INFLOW : ID= 1( 0329) 0.27 0.01 0.08 52.19  
 OUTFLOW: ID= 2( 0332) 0.27 0.01 0.08 52.19

-----  
 [ ADD HYD ( 3106) ]  
 [ 1 + 2 + 3 ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0330): 0.31 0.089 12.33 52.19  
 + ID2= 2 ( 0331): 0.01 0.025 12.25 69.74

ID = 3 ( 3106): 0.33 0.089 12.33 52.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 [ ADD HYD ( 3106) ]  
 [ 3 + 2 + 1 ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 3 ( 3106): 0.33 0.089 12.33 52.86  
 + ID2= 2 ( 0332): 0.27 0.009 0.08 52.19  
 ID = 1 ( 3106): 0.59 0.090 12.33 52.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 [ ADD HYD ( 0332) ]  
 [ 1 + 2 + 3 ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0277): 1.23 0.270 12.08 63.33  
 + ID2= 2 ( 0310): 5.48 1.210 12.25 69.20  
 ID = 3 ( 0332): 6.71 1.480 12.25 68.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 [ ADD HYD ( 0332) ]  
 [ 3 + 2 + 1 ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 3 ( 0332): 6.71 1.480 12.25 68.13  
 + ID2= 2 ( 3106): 0.59 0.090 12.33 52.56  
 ID = 1 ( 0332): 7.30 1.507 12.25 66.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 [ ADD HYD ( 0332) ]  
 [ 1 + 2 + 3 ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0332): 7.30 1.507 12.25 66.87  
 + ID2= 2 ( 0316): 14.21 3.593 12.25 74.63  
 ID = 3 ( 0332): 21.51 5.099 12.25 72.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
 [ RESERVOIR ( 0319) ] OVERFLOW IS OFF  
 [ In 2--> OUT= 1 ]  
 DT= 5.0 min  
 OUTFLOW STORAGE OUTFLOW STORAGE  
 (cms) (ha.m.) (cms) (ha.m.)  
 0.0000 0.0000 0.0510 0.9050  
 0.0400 0.1140 1.0960 1.3320  
 0.0760 0.3690 1.4260 1.5860  
 0.2810 0.6450 0.0000 0.0000

AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 INFLOW : ID= 2 ( 0312) 21.510 5.099 12.25 72.00  
 OUTFLOW: ID= 1 ( 0319) 21.510 0.535 12.83 71.96

PEAK FLOW REDUCTION [Qout/Qin](%) = 10.49  
 TIME SHIFT OF PEAK FLOW (min) = 35.00  
 MAXIMUM STORAGE USED (ha.m.) = 0.8725



NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: ADD HYD (0243), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: ADD HYD (0243), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: RESERVOIR (0188), OVERFLOW IS OFF, DT=2-->OUT=1, Outflow (cms), Storage (ha.), Outflow (ha.), Storage (ha.).

Table with columns: IMPERVIOUS, PERVIOUS (1), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

PEAK FLOW REDUCTION [Qout/Qin](%)= 80.88, TIME SHIFT OF PEAK FLOW (min)= 5.00, MAXIMUM STORAGE USED (ha.)= 0.0118

Table with columns: CALIB, STANHYD (0252), Area (ha), Total Imp(%)= 69.00, Dir. Conn.(%)= 30.00, ID=1 DT=5.0 min.

IMPERVIOUS PERVIOUS (1)

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Large table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN. Contains multiple rows of rainfall data.

Table with columns: Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Large table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN. Contains multiple rows of rainfall data.

Max.Eff.Inten.(mm/hr)= 133.17, 251.52, over (min)= 5.00, 10.00, Storage Coeff. (min)= 1.43 (ii), 6.31 (ii), Unit Hyd. Tpeak (min)= 5.00, 10.00, Unit Hyd. peak (cms)= 0.33, 0.15

PEAK FLOW (cms)= 0.03, 0.06, TIME TO PEAK (hrs)= 12.25, 12.25, RUNOFF VOLUME (mm)= 107.80, 77.75, 86.76, TOTAL RAINFALL (mm)= 108.80, 108.80, 108.80, RUNOFF COEFFICIENT = 0.99, 0.71, 0.80

\*\*\*WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN = 74.0, Ia = Dep. Storage (Above), (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD (0179), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANHYD (0186), Area (ha), Total Imp(%)= 55.00, Dir. Conn.(%)= 1.00, ID=1 DT=5.0 min.

IMPERVIOUS PERVIOUS (1)

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: ADD HYD (0179), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANHYD (0186), Area (ha), Total Imp(%)= 55.00, Dir. Conn.(%)= 1.00, ID=1 DT=5.0 min.

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: ADD HYD (0179), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANHYD (0186), Area (ha), Total Imp(%)= 55.00, Dir. Conn.(%)= 1.00, ID=1 DT=5.0 min.

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: CALIB, STANHYD (0255), Area (ha), Total Imp(%)= 55.00, Dir. Conn.(%)= 1.00, ID=1 DT=5.0 min.

IMPERVIOUS PERVIOUS (1)

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: ADD HYD (0246), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANHYD (0246), Area (ha), Total Imp(%)= 55.00, Dir. Conn.(%)= 1.00, ID=1 DT=5.0 min.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: ADD HYD (0246), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANHYD (0246), Area (ha), Total Imp(%)= 55.00, Dir. Conn.(%)= 1.00, ID=1 DT=5.0 min.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Mannings n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID=1, ID=2, ID=3.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Hydrology simulation results table with columns for time, rain, and flow. Includes a summary table for peak flows and runoff coefficients.

Hydrology simulation results table with columns for time, rain, and flow. Includes a summary table for peak flows and runoff coefficients.

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Hydrology simulation results table with columns for time, rain, and flow. Includes a summary table for peak flows and runoff coefficients.

Table with columns: TIME, RAIN, INFL, INFIL, PEAK, TPEAK, R.V., (mm), (in), (ft), (m). Includes data for runoff volume, infiltration, and peak flows.

Table with columns: TIME, RAIN, INFL, INFIL, PEAK, TPEAK, R.V., (mm), (in), (ft), (m). Includes data for runoff volume, infiltration, and peak flows.

Table with columns: TIME, RAIN, INFL, INFIL, PEAK, TPEAK, R.V., (mm), (in), (ft), (m). Includes data for runoff volume, infiltration, and peak flows.

Table with columns: TIME, RAIN, INFL, INFIL, PEAK, TPEAK, R.V., (mm), (in), (ft), (m). Includes data for runoff volume, infiltration, and peak flows.

Table with columns: TIME, RAIN, INFL, INFIL, PEAK, TPEAK, R.V., (mm), (in), (ft), (m). Includes data for runoff volume, infiltration, and peak flows.

Table with columns: TIME, RAIN, INFL, INFIL, PEAK, TPEAK, R.V., (mm), (in), (ft), (m). Includes data for runoff volume, infiltration, and peak flows.

Table with columns: TIME, RAIN, INFL, INFIL, PEAK, TPEAK, R.V., (mm), (in), (ft), (m). Includes data for runoff volume, infiltration, and peak flows.

Table with columns: TIME, RAIN, INFL, INFIL, PEAK, TPEAK, R.V., (mm), (in), (ft), (m). Includes data for runoff volume, infiltration, and peak flows.

Table with columns: TIME, RAIN, INFL, INFIL, PEAK, TPEAK, R.V., (mm), (in), (ft), (m). Includes data for runoff volume, infiltration, and peak flows.

|       |      |        |        |         |      |       |      |
|-------|------|--------|--------|---------|------|-------|------|
| 3.833 | 1.41 | 9.917  | 3.92   | 116.000 | 3.26 | 22.08 | 1.31 |
| 3.917 | 1.41 | 10.000 | 3.92   | 116.083 | 3.26 | 22.17 | 1.31 |
| 4.000 | 1.41 | 10.083 | 3.92   | 116.167 | 3.26 | 22.25 | 1.31 |
| 4.083 | 1.41 | 10.167 | 3.92   | 116.250 | 3.26 | 22.33 | 1.31 |
| 4.167 | 1.41 | 10.250 | 3.92   | 116.333 | 1.96 | 22.42 | 1.31 |
| 4.250 | 1.41 | 10.333 | 5.00   | 116.417 | 1.96 | 22.50 | 1.31 |
| 4.333 | 1.74 | 10.417 | 5.00   | 116.500 | 1.96 | 22.58 | 1.31 |
| 4.417 | 1.74 | 10.500 | 5.00   | 116.583 | 1.96 | 22.67 | 1.31 |
| 4.500 | 1.74 | 10.583 | 5.00   | 116.667 | 1.96 | 22.75 | 1.31 |
| 4.583 | 1.74 | 10.667 | 5.00   | 116.750 | 1.96 | 22.83 | 1.31 |
| 4.667 | 1.74 | 10.750 | 5.00   | 116.833 | 1.96 | 22.92 | 1.31 |
| 4.750 | 1.74 | 10.833 | 6.75   | 116.917 | 1.96 | 23.00 | 1.31 |
| 4.833 | 1.74 | 10.917 | 6.75   | 117.000 | 1.96 | 23.08 | 1.31 |
| 4.917 | 1.74 | 11.000 | 6.75   | 117.083 | 1.96 | 23.17 | 1.31 |
| 5.000 | 1.74 | 11.083 | 6.75   | 117.167 | 1.96 | 23.25 | 1.31 |
| 5.083 | 1.74 | 11.167 | 6.75   | 117.250 | 1.96 | 23.33 | 1.31 |
| 5.167 | 1.74 | 11.250 | 6.75   | 117.333 | 1.96 | 23.42 | 1.31 |
| 5.250 | 1.74 | 11.333 | 10.44  | 117.417 | 1.96 | 23.50 | 1.31 |
| 5.333 | 1.74 | 11.417 | 10.44  | 117.500 | 1.96 | 23.58 | 1.31 |
| 5.417 | 1.74 | 11.500 | 10.44  | 117.583 | 1.96 | 23.67 | 1.31 |
| 5.500 | 1.74 | 11.583 | 10.44  | 117.667 | 1.96 | 23.75 | 1.31 |
| 5.583 | 1.74 | 11.667 | 10.44  | 117.750 | 1.96 | 23.83 | 1.31 |
| 5.667 | 1.74 | 11.750 | 10.44  | 117.833 | 1.96 | 23.92 | 1.31 |
| 5.750 | 1.74 | 11.833 | 12.20  | 117.917 | 1.96 | 24.00 | 1.31 |
| 5.833 | 1.74 | 11.917 | 12.20  | 118.000 | 1.96 | 24.08 | 1.31 |
| 5.917 | 1.74 | 12.000 | 12.20  | 118.083 | 1.96 | 24.17 | 1.31 |
| 6.000 | 1.74 | 12.083 | 133.16 | 118.167 | 1.96 | 24.25 | 1.31 |
| 6.083 | 1.74 | 12.167 | 133.17 | 118.250 | 1.96 |       |      |

Max. Eff. Inten. (m/hr) = 133.17 244.31  
 over (min) = 5.00 10.00  
 Storage Coeff. (min) = 1.72 (11) 6.66 (11)  
 Unit Hyd. Tpeak (min) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.32 0.14

\*TOTALS\*  
 PEAK FLOW (cms) = 0.05 0.11 0.169 (111)  
 TIME TO PEAK (hrs) = 12.25 12.25 12.25  
 RUNOFF VOLUME (mm) = 107.80 77.21 84.85  
 TOTAL RAINFALL (mm) = 108.80 108.80 108.80  
 RUNOFF COEFFICIENT = 0.90 0.71 0.78

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN = 74.0 Ia = Dep. Storage (Above)  
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

DUHWD ( 0331)  
 Inlet Cap. = 0.132  
 # of Inlets = 1  
 Total (cms) = 0.11 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 TOTAL HYD. (ID= 1): 0.59 0.17 12.25 84.85  
 MAJOR SVS. (ID= 2): 0.03 0.06 12.25 84.85  
 MINOR SVS. (ID= 3): 0.56 0.11 12.17 84.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Retention ( 0329) | OUTFLOW: OFF UNDERDRAIN: ON  
 In = 2 -> OUT = 3  
 DT = 5.0 min

SURFACE PONDING LAYER:  
 Max. Ponding Storage (cu.m.) = 149.95

| STAGE | AREA    | STAGE | AREA    |
|-------|---------|-------|---------|
| (m)   | (m2)    | (m)   | (m2)    |
| 0.000 | 100.000 | 1.500 | 100.000 |
| 0.500 | 100.000 | 0.000 | 0.000   |
| 1.000 | 100.000 | 0.000 | 0.000   |

MULCH LAYER:  
 Depth (m) = 0.00 Porosity = 0.40  
 Maximum Mulch Layer Volume (cu.m.) = 0.00

ENGINEERED SOIL LAYER:  
 Soil moisture = 0.30 Depth (m) = 0.72  
 Width (m) = 3.00 Length (m) = 240.00  
 Porosity = 0.47 Infiltration (m/hr) = 0.0060  
 Maximum Engineered Soil Layer Volume (cu.m.) = 242.00

| DEPTH | DISCHARGE | DEPTH | DISCHARGE |
|-------|-----------|-------|-----------|
| (m)   | (cms)     | (m)   | (cms)     |
| 0.000 | 0.000     | 0.559 | 0.030     |
| 0.279 | 0.010     | 0.762 | 0.040     |
| 0.406 | 0.020     | 1.300 | 0.050     |

STORAGE LAYER:  
 Depth (m) = 1.00 Porosity = 0.40  
 Seepage (m/hr) = 0.0010  
 Maximum Storage Layer Volume (cu.m.) = 288.00

TOTAL AVAILABLE STORAGE:

TOTAL STORAGE = Surface Ponding + Mulch Layer + Engineered soil  
 + Storage Layer (cu.m.) = 680.04

NATIVE SOIL LAYER:  
 Infiltration (m/hr) = 0.0060

| AREA            | QPEAK | TPEAK | R.V.  |       |
|-----------------|-------|-------|-------|-------|
| (ha)            | (cms) | (hrs) | (mm)  |       |
| INFLOW: ID= 2   | 0.56  | 0.112 | 12.17 | 84.85 |
| OUTFLOW: ID= 1  | 0.21  | 0.009 | 0.08  | 66.70 |
| OVERFLOW: ID= 3 | 0.35  | 0.110 | 12.33 | 66.70 |

Volume Reduction Rate (RvIn-RvOut)/RvIn (%) = 21.40  
 Time to reach Max Ponding Storage (hr) = 0.00  
 Volume of water for drawdown in IID (cu.m.) = 156.53  
 Volume of Max. Water Storage (cu.m.) = 0.00  
 Maximum Surface Ponding and Mulch Vol. (cu.m.) = 149.95  
 Maximum Engineered Soil Volume (cu.m.) = 60.86  
 \*\*\*\* After simulation, water volume is not zero.

Junction Command (0330)

| AREA                  | QPEAK | TPEAK | R.V.  |       |
|-----------------------|-------|-------|-------|-------|
| (ha)                  | (cms) | (hrs) | (mm)  |       |
| INFLOW : ID= 3( 0329) | 0.35  | 0.11  | 12.33 | 66.70 |
| OUTFLOW: ID= 2( 0330) | 0.35  | 0.11  | 12.33 | 66.70 |

Junction Command (0332)

| AREA                  | QPEAK | TPEAK | R.V. |       |
|-----------------------|-------|-------|------|-------|
| (ha)                  | (cms) | (hrs) | (mm) |       |
| INFLOW : ID= 1( 0329) | 0.21  | 0.01  | 0.08 | 66.70 |
| OUTFLOW: ID= 2( 0332) | 0.21  | 0.01  | 0.08 | 66.70 |

ADD HYD ( 3106)

| AREA            | QPEAK | TPEAK | R.V.  |       |
|-----------------|-------|-------|-------|-------|
| (ha)            | (cms) | (hrs) | (mm)  |       |
| ID= 1 ( 0330)   | 0.35  | 0.110 | 12.33 | 66.70 |
| + ID= 2 ( 0331) | 0.03  | 0.007 | 12.25 | 96.85 |

ID = 3 ( 3106): 0.38 0.147 12.25 68.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 3106)  
 3 + 2 = 1  
 ID= 3 ( 3106): 0.38 0.147 12.25 68.20  
 + ID= 2 ( 0332): 0.21 0.009 0.08 66.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0332)  
 1 + 2 = 3  
 ID= 1 ( 0277): 1.18 0.270 12.08 77.72  
 + ID= 2 ( 0310): 5.48 1.484 12.25 83.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0332)  
 3 + 2 = 1  
 ID= 3 ( 0332): 6.66 1.754 12.25 82.67  
 + ID= 2 ( 3106): 0.59 0.148 12.25 67.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0332)  
 1 + 2 = 3  
 ID= 1 ( 0332): 7.25 1.901 12.25 81.45  
 + ID= 2 ( 0316): 14.21 4.322 12.25 89.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR ( 0319) | OVERFLOW IS OFF  
 In = 2 -> OUT = 1  
 DT = 5.0 min

| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
|---------|---------|---------|---------|
| (cms)   | (ha.m.) | (cms)   | (ha.m.) |
| 0.0000  | 0.0000  | 0.0510  | 0.9050  |
| 0.0400  | 0.1140  | 1.0960  | 1.3320  |
| 0.0760  | 0.3690  | 1.4260  | 1.5860  |
| 0.2810  | 0.6450  | 0.0000  | 0.0000  |

| AREA                   | QPEAK  | TPEAK | R.V.  |       |
|------------------------|--------|-------|-------|-------|
| (ha)                   | (cms)  | (hrs) | (mm)  |       |
| INFLOW : ID= 2 ( 0312) | 21.458 | 6.223 | 12.25 | 86.90 |
| OUTFLOW: ID= 1 ( 0319) | 21.458 | 0.721 | 12.83 | 86.86 |

PEAK FLOW REDUCTION [Qout/Qin](%) = 11.59  
 TIME SHIFT OF PEAK FLOW (min) = 35.00  
 MAXIMUM STORAGE USED (ha.m.) = 1.0340





NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: ADD HYD (0243), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID# 1, 2, 3 and a summary row.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: ADD HYD (0243), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID# 1, 2, 3 and a summary row.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: RESERVOIR (0188), OVERFLOW IS OFF, DT=2-->OUT=1, Outflow (cms), Storage (ha.m.), Outflow (cms), Storage (ha.m.).

Table with columns: INFLOW: ID# 2 (0243), OUTFLOW: ID# 1 (0188), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), IMPERVIOUS PERVIOUS (I), PEAK FLOW REDUCTION, TIME SHIFT OF PEAK FLOW, MAXIMUM STORAGE USED.

Table with columns: CALIB, STANDHYD (0252), Area (ha), Total Imp (cms), Dir. Conn. (%), 30.00, IMPERVIOUS PERVIOUS (I).

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Manning's n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN.

Table with columns: Max. Eff. Inten. (mm/hr), over (min), Storage Coeff. (min), Unit Hyd. Tpeak (mm), Unit Hyd. peak (cms).

Table with columns: PEAK FLOW (cms), TIME TO PEAK (hrs), RUNOFF VOLUME (mm), TOTAL RAINFALL (mm), RUNOFF COEFFICIENT.

\*\*\*WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES: CN = 74.0 Ia = Dep. Storage (Above)

Table with columns: ADD HYD (0179), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID# 1, 2, 3 and a summary row.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANDHYD (0186), Area (ha), Total Imp (cms), Dir. Conn. (%), 1.00, IMPERVIOUS PERVIOUS (I).

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Manning's n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN.

Table with columns: Max. Eff. Inten. (mm/hr), over (min), Storage Coeff. (min), Unit Hyd. Tpeak (mm), Unit Hyd. peak (cms).

Table with columns: PEAK FLOW (cms), TIME TO PEAK (hrs), RUNOFF VOLUME (mm), TOTAL RAINFALL (mm), RUNOFF COEFFICIENT.

\*\*\*WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

\*\*\*WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES: CN = 74.0 Ia = Dep. Storage (Above)

Table with columns: ADD HYD (0179), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID# 1, 2, 3 and a summary row.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANDHYD (0186), Area (ha), Total Imp (cms), Dir. Conn. (%), 1.00, IMPERVIOUS PERVIOUS (I).

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Manning's n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN.

Table with columns: Max. Eff. Inten. (mm/hr), over (min), Storage Coeff. (min), Unit Hyd. Tpeak (mm), Unit Hyd. peak (cms).

Table with columns: PEAK FLOW (cms), TIME TO PEAK (hrs), RUNOFF VOLUME (mm), TOTAL RAINFALL (mm), RUNOFF COEFFICIENT.

\*\*\*WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

\*\*\*WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES: CN = 74.0 Ia = Dep. Storage (Above)

Table with columns: ADD HYD (0246), Area (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID# 1, 2, 3 and a summary row.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANDHYD (0246), Area (ha), Total Imp (cms), Dir. Conn. (%), 30.00, IMPERVIOUS PERVIOUS (I).

Table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Manning's n.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN.

Table with columns: TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN, TIME RAIN.

Table with columns: Max. Eff. Inten. (mm/hr), over (min), Storage Coeff. (min), Unit Hyd. Tpeak (mm), Unit Hyd. peak (cms).

Table with columns: PEAK FLOW (cms), TIME TO PEAK (hrs), RUNOFF VOLUME (mm), TOTAL RAINFALL (mm), RUNOFF COEFFICIENT.

\*\*\*WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

|       |      |        |      |         |      |       |      |
|-------|------|--------|------|---------|------|-------|------|
| 0.917 | 1.33 | 7.000  | 2.17 | 113.083 | 8.94 | 19.17 | 2.17 |
| 1.000 | 1.33 | 7.083  | 2.17 | 113.167 | 8.94 | 19.25 | 2.17 |
| 1.083 | 1.33 | 7.167  | 2.17 | 113.250 | 8.94 | 19.33 | 2.17 |
| 1.167 | 1.33 | 7.250  | 2.17 | 113.333 | 8.94 | 19.41 | 2.17 |
| 1.250 | 1.33 | 7.333  | 2.17 | 113.417 | 8.94 | 19.49 | 2.17 |
| 1.333 | 1.33 | 7.417  | 2.17 | 113.500 | 8.94 | 19.57 | 2.17 |
| 1.417 | 1.33 | 7.500  | 2.17 | 113.583 | 8.94 | 19.65 | 2.17 |
| 1.500 | 1.33 | 7.583  | 2.17 | 113.667 | 8.94 | 19.73 | 2.17 |
| 1.583 | 1.33 | 7.667  | 2.17 | 113.750 | 8.94 | 19.81 | 2.17 |
| 1.667 | 1.33 | 7.750  | 2.17 | 113.833 | 8.94 | 19.89 | 2.17 |
| 1.750 | 1.33 | 7.833  | 2.17 | 113.917 | 8.94 | 19.97 | 2.17 |
| 1.833 | 1.33 | 7.917  | 2.17 | 114.000 | 8.94 | 20.05 | 2.17 |
| 1.917 | 1.33 | 8.000  | 2.17 | 114.083 | 8.94 | 20.13 | 2.17 |
| 2.000 | 1.33 | 8.083  | 2.17 | 114.167 | 8.94 | 20.21 | 2.17 |
| 2.083 | 1.33 | 8.167  | 2.17 | 114.250 | 8.94 | 20.29 | 2.17 |
| 2.167 | 1.33 | 8.250  | 2.17 | 114.333 | 8.94 | 20.37 | 2.17 |
| 2.250 | 1.33 | 8.333  | 2.17 | 114.417 | 8.94 | 20.45 | 2.17 |
| 2.333 | 1.33 | 8.417  | 2.17 | 114.500 | 8.94 | 20.53 | 2.17 |
| 2.417 | 1.33 | 8.500  | 2.17 | 114.583 | 8.94 | 20.61 | 2.17 |
| 2.500 | 1.33 | 8.583  | 2.17 | 114.667 | 8.94 | 20.69 | 2.17 |
| 2.583 | 1.33 | 8.667  | 2.17 | 114.750 | 8.94 | 20.77 | 2.17 |
| 2.667 | 1.33 | 8.750  | 2.17 | 114.833 | 8.94 | 20.85 | 2.17 |
| 2.750 | 1.33 | 8.833  | 2.17 | 114.917 | 8.94 | 20.93 | 2.17 |
| 2.833 | 1.33 | 8.917  | 2.17 | 115.000 | 8.94 | 21.01 | 2.17 |
| 2.917 | 1.33 | 9.000  | 2.17 | 115.083 | 8.94 | 21.09 | 2.17 |
| 3.000 | 1.33 | 9.083  | 2.17 | 115.167 | 8.94 | 21.17 | 2.17 |
| 3.083 | 1.33 | 9.167  | 2.17 | 115.250 | 8.94 | 21.25 | 2.17 |
| 3.167 | 1.33 | 9.250  | 2.17 | 115.333 | 8.94 | 21.33 | 2.17 |
| 3.250 | 1.33 | 9.333  | 2.17 | 115.417 | 8.94 | 21.41 | 2.17 |
| 3.333 | 1.33 | 9.417  | 2.17 | 115.500 | 8.94 | 21.49 | 2.17 |
| 3.417 | 1.33 | 9.500  | 2.17 | 115.583 | 8.94 | 21.57 | 2.17 |
| 3.500 | 1.33 | 9.583  | 2.17 | 115.667 | 8.94 | 21.65 | 2.17 |
| 3.583 | 1.33 | 9.667  | 2.17 | 115.750 | 8.94 | 21.73 | 2.17 |
| 3.667 | 1.33 | 9.750  | 2.17 | 115.833 | 8.94 | 21.81 | 2.17 |
| 3.750 | 1.33 | 9.833  | 2.17 | 115.917 | 8.94 | 21.89 | 2.17 |
| 3.833 | 1.33 | 9.917  | 2.17 | 116.000 | 8.94 | 21.97 | 2.17 |
| 3.917 | 1.33 | 10.000 | 2.17 | 116.083 | 8.94 | 22.05 | 2.17 |
| 4.000 | 1.33 | 10.083 | 2.17 | 116.167 | 8.94 | 22.13 | 2.17 |
| 4.083 | 1.33 | 10.167 | 2.17 | 116.250 | 8.94 | 22.21 | 2.17 |
| 4.167 | 1.33 | 10.250 | 2.17 | 116.333 | 8.94 | 22.29 | 2.17 |
| 4.250 | 1.33 | 10.333 | 2.17 | 116.417 | 8.94 | 22.37 | 2.17 |
| 4.333 | 1.33 | 10.417 | 2.17 | 116.500 | 8.94 | 22.45 | 2.17 |
| 4.417 | 1.33 | 10.500 | 2.17 | 116.583 | 8.94 | 22.53 | 2.17 |
| 4.500 | 1.33 | 10.583 | 2.17 | 116.667 | 8.94 | 22.61 | 2.17 |
| 4.583 | 1.33 | 10.667 | 2.17 | 116.750 | 8.94 | 22.69 | 2.17 |
| 4.667 | 1.33 | 10.750 | 2.17 | 116.833 | 8.94 | 22.77 | 2.17 |
| 4.750 | 1.33 | 10.833 | 2.17 | 116.917 | 8.94 | 22.85 | 2.17 |
| 4.833 | 1.33 | 10.917 | 2.17 | 117.000 | 8.94 | 22.93 | 2.17 |
| 4.917 | 1.33 | 11.000 | 2.17 | 117.083 | 8.94 | 23.01 | 2.17 |
| 5.000 | 1.33 | 11.083 | 2.17 | 117.167 | 8.94 | 23.09 | 2.17 |

|       |      |        |      |         |      |       |      |
|-------|------|--------|------|---------|------|-------|------|
| 5.083 | 1.93 | 11.167 | 7.49 | 117.250 | 2.17 | 23.33 | 1.45 |
| 5.167 | 1.93 | 11.250 | 7.49 | 117.333 | 2.17 | 23.42 | 1.45 |
| 5.250 | 1.93 | 11.333 | 7.49 | 117.417 | 2.17 | 23.51 | 1.45 |
| 5.333 | 1.93 | 11.417 | 7.49 | 117.500 | 2.17 | 23.58 | 1.45 |
| 5.417 | 1.93 | 11.500 | 7.49 | 117.583 | 2.17 | 23.67 | 1.45 |
| 5.500 | 1.93 | 11.583 | 7.49 | 117.667 | 2.17 | 23.75 | 1.45 |
| 5.583 | 1.93 | 11.667 | 7.49 | 117.750 | 2.17 | 23.83 | 1.45 |
| 5.667 | 1.93 | 11.750 | 7.49 | 117.833 | 2.17 | 23.92 | 1.45 |
| 5.750 | 1.93 | 11.833 | 7.49 | 117.917 | 2.17 | 24.00 | 1.45 |
| 5.833 | 1.93 | 11.917 | 7.49 | 118.000 | 2.17 | 24.08 | 1.45 |
| 5.917 | 1.93 | 12.000 | 7.49 | 118.083 | 2.17 | 24.17 | 1.45 |
| 6.000 | 1.93 | 12.083 | 7.49 | 118.167 | 2.17 | 24.25 | 1.45 |
| 6.083 | 1.93 | 12.167 | 7.49 | 118.250 | 2.17 | 24.34 | 1.45 |

Unit Hyd. Peak (cms) = 0.149

PEAK FLOW (cms) = 0.148 (1)  
 TIME TO PEAK (hrs) = 12.33  
 RAINFALL VOLUME (mm) = 22.092  
 TOTAL RAINFALL (mm) = 120.770  
 RUNOFF COEFFICIENT = 0.602

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|       |      |        |      |         |      |       |      |
|-------|------|--------|------|---------|------|-------|------|
| 0.667 | 1.33 | 6.750  | 2.17 | 112.833 | 8.94 | 19.17 | 2.17 |
| 0.750 | 1.33 | 6.833  | 2.17 | 112.917 | 8.94 | 19.25 | 2.17 |
| 0.833 | 1.33 | 6.917  | 2.17 | 113.000 | 8.94 | 19.33 | 2.17 |
| 0.917 | 1.33 | 7.000  | 2.17 | 113.083 | 8.94 | 19.41 | 2.17 |
| 1.000 | 1.33 | 7.083  | 2.17 | 113.167 | 8.94 | 19.49 | 2.17 |
| 1.083 | 1.33 | 7.167  | 2.17 | 113.250 | 8.94 | 19.57 | 2.17 |
| 1.167 | 1.33 | 7.250  | 2.17 | 113.333 | 8.94 | 19.65 | 2.17 |
| 1.250 | 1.33 | 7.333  | 2.17 | 113.417 | 8.94 | 19.73 | 2.17 |
| 1.333 | 1.33 | 7.417  | 2.17 | 113.500 | 8.94 | 19.81 | 2.17 |
| 1.417 | 1.33 | 7.500  | 2.17 | 113.583 | 8.94 | 19.89 | 2.17 |
| 1.500 | 1.33 | 7.583  | 2.17 | 113.667 | 8.94 | 19.97 | 2.17 |
| 1.583 | 1.33 | 7.667  | 2.17 | 113.750 | 8.94 | 20.05 | 2.17 |
| 1.667 | 1.33 | 7.750  | 2.17 | 113.833 | 8.94 | 20.13 | 2.17 |
| 1.750 | 1.33 | 7.833  | 2.17 | 113.917 | 8.94 | 20.21 | 2.17 |
| 1.833 | 1.33 | 7.917  | 2.17 | 114.000 | 8.94 | 20.29 | 2.17 |
| 1.917 | 1.33 | 8.000  | 2.17 | 114.083 | 8.94 | 20.37 | 2.17 |
| 2.000 | 1.33 | 8.083  | 2.17 | 114.167 | 8.94 | 20.45 | 2.17 |
| 2.083 | 1.33 | 8.167  | 2.17 | 114.250 | 8.94 | 20.53 | 2.17 |
| 2.167 | 1.33 | 8.250  | 2.17 | 114.333 | 8.94 | 20.61 | 2.17 |
| 2.250 | 1.33 | 8.333  | 2.17 | 114.417 | 8.94 | 20.69 | 2.17 |
| 2.333 | 1.33 | 8.417  | 2.17 | 114.500 | 8.94 | 20.77 | 2.17 |
| 2.417 | 1.33 | 8.500  | 2.17 | 114.583 | 8.94 | 20.85 | 2.17 |
| 2.500 | 1.33 | 8.583  | 2.17 | 114.667 | 8.94 | 20.93 | 2.17 |
| 2.583 | 1.33 | 8.667  | 2.17 | 114.750 | 8.94 | 21.01 | 2.17 |
| 2.667 | 1.33 | 8.750  | 2.17 | 114.833 | 8.94 | 21.09 | 2.17 |
| 2.750 | 1.33 | 8.833  | 2.17 | 114.917 | 8.94 | 21.17 | 2.17 |
| 2.833 | 1.33 | 8.917  | 2.17 | 115.000 | 8.94 | 21.25 | 2.17 |
| 2.917 | 1.33 | 9.000  | 2.17 | 115.083 | 8.94 | 21.33 | 2.17 |
| 3.000 | 1.33 | 9.083  | 2.17 | 115.167 | 8.94 | 21.41 | 2.17 |
| 3.083 | 1.33 | 9.167  | 2.17 | 115.250 | 8.94 | 21.49 | 2.17 |
| 3.167 | 1.33 | 9.250  | 2.17 | 115.333 | 8.94 | 21.57 | 2.17 |
| 3.250 | 1.33 | 9.333  | 2.17 | 115.417 | 8.94 | 21.65 | 2.17 |
| 3.333 | 1.33 | 9.417  | 2.17 | 115.500 | 8.94 | 21.73 | 2.17 |
| 3.417 | 1.33 | 9.500  | 2.17 | 115.583 | 8.94 | 21.81 | 2.17 |
| 3.500 | 1.33 | 9.583  | 2.17 | 115.667 | 8.94 | 21.89 | 2.17 |
| 3.583 | 1.33 | 9.667  | 2.17 | 115.750 | 8.94 | 21.97 | 2.17 |
| 3.667 | 1.33 | 9.750  | 2.17 | 115.833 | 8.94 | 22.05 | 2.17 |
| 3.750 | 1.33 | 9.833  | 2.17 | 115.917 | 8.94 | 22.13 | 2.17 |
| 3.833 | 1.33 | 9.917  | 2.17 | 116.000 | 8.94 | 22.21 | 2.17 |
| 3.917 | 1.33 | 10.000 | 2.17 | 116.083 | 8.94 | 22.29 | 2.17 |
| 4.000 | 1.33 | 10.083 | 2.17 | 116.167 | 8.94 | 22.37 | 2.17 |
| 4.083 | 1.33 | 10.167 | 2.17 | 116.250 | 8.94 | 22.45 | 2.17 |
| 4.167 | 1.33 | 10.250 | 2.17 | 116.333 | 8.94 | 22.53 | 2.17 |
| 4.250 | 1.33 | 10.333 | 2.17 | 116.417 | 8.94 | 22.61 | 2.17 |
| 4.333 | 1.33 | 10.417 | 2.17 | 116.500 | 8.94 | 22.69 | 2.17 |
| 4.417 | 1.33 | 10.500 | 2.17 | 116.583 | 8.94 | 22.77 | 2.17 |
| 4.500 | 1.33 | 10.583 | 2.17 | 116.667 | 8.94 | 22.85 | 2.17 |
| 4.583 | 1.33 | 10.667 | 2.17 | 116.750 | 8.94 | 22.93 | 2.17 |
| 4.667 | 1.33 | 10.750 | 2.17 | 116.833 | 8.94 | 23.01 | 2.17 |
| 4.750 | 1.33 | 10.833 | 2.17 | 116.917 | 8.94 | 23.09 | 2.17 |

|       |      |         |      |         |      |       |      |
|-------|------|---------|------|---------|------|-------|------|
| 4.833 | 1.93 | 110.917 | 7.49 | 117.000 | 2.17 | 23.08 | 1.45 |
| 4.917 | 1.93 | 111.000 | 7.49 | 117.083 | 2.17 | 23.17 | 1.45 |
| 5.000 | 1.93 | 111.083 | 7.49 | 117.167 | 2.17 | 23.25 | 1.45 |
| 5.083 | 1.93 | 111.167 | 7.49 | 117.250 | 2.17 | 23.33 | 1.45 |
| 5.167 | 1.93 | 111.250 | 7.49 | 117.333 | 2.17 | 23.42 | 1.45 |
| 5.250 | 1.93 | 111.333 | 7.49 | 117.417 | 2.17 | 23.50 | 1.45 |
| 5.333 | 1.93 | 111.417 | 7.49 | 117.500 | 2.17 | 23.58 | 1.45 |
| 5.417 | 1.93 | 111.500 | 7.49 | 117.583 | 2.17 | 23.67 | 1.45 |
| 5.500 | 1.93 | 111.583 | 7.49 | 117.667 | 2.17 | 23.75 | 1.45 |
| 5.583 | 1.93 | 111.667 | 7.49 | 117.750 | 2.17 | 23.83 | 1.45 |
| 5.667 | 1.93 | 111.750 | 7.49 | 117.833 | 2.17 | 23.92 | 1.45 |
| 5.750 | 1.93 | 111.833 | 7.49 | 117.917 | 2.17 | 24.00 | 1.45 |
| 5.833 | 1.93 | 111.917 | 7.49 | 118.000 | 2.17 | 24.08 | 1.45 |
| 5.917 | 1.93 | 112.000 | 7.49 | 118.083 | 2.17 | 24.17 | 1.45 |
| 6.000 | 1.93 | 112.083 | 7.49 | 118.167 | 2.17 | 24.25 | 1.45 |
| 6.083 | 1.93 | 112.167 | 7.49 | 118.250 | 2.17 | 24.34 | 1.45 |

Max. Eff. Inten. (mm/hr) = 147.82 276.97  
 over (min) = 5.00 10.00  
 Storage Coeff. (min) = 1.57 (1) 6.27 (11)  
 Unit Hyd. Peak (cms) = 0.40 10.00  
 Unit Hyd. Peak (cms) = 0.33 0.15

PEAK FLOW (cms) = 0.00 0.15  
 TIME TO PEAK (hrs) = 12.25 12.25  
 RAINFALL VOLUME (mm) = 119.77 88.28  
 TOTAL RAINFALL (mm) = 120.77 120.77  
 RUNOFF COEFFICIENT = 0.99 0.73

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 \*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE LAYER.

(1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN = 70.0 in Dep. Storage (Above)  
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|       |      |         |        |         |      |       |      |
|-------|------|---------|--------|---------|------|-------|------|
| 4.833 | 1.93 | 110.917 | 7.49   | 117.000 | 2.17 | 23.08 | 1.45 |
| 4.917 | 1.93 | 111.000 | 7.49   | 117.083 | 2.17 | 23.17 | 1.45 |
| 5.000 | 1.93 | 111.083 | 7.49   | 117.167 | 2.17 | 23.25 | 1.45 |
| 5.083 | 1.93 | 111.167 | 7.49   | 117.250 | 2.17 | 23.33 | 1.45 |
| 5.167 | 1.93 | 111.250 | 7.49   | 117.333 | 2.17 | 23.42 | 1.45 |
| 5.250 | 1.93 | 111.333 | 7.49   | 117.417 | 2.17 | 23.50 | 1.45 |
| 5.333 | 1.93 | 111.417 | 7.49   | 117.500 | 2.17 | 23.58 | 1.45 |
| 5.417 | 1.93 | 111.500 | 7.49   | 117.583 | 2.17 | 23.67 | 1.45 |
| 5.500 | 1.93 | 111.583 | 7.49   | 117.667 | 2.17 | 23.75 | 1.45 |
| 5.583 | 1.93 | 111.667 | 7.49   | 117.750 | 2.17 | 23.83 | 1.45 |
| 5.667 | 1.93 | 111.750 | 7.49</ |         |      |       |      |

|       |      |        |      |         |      |       |      |
|-------|------|--------|------|---------|------|-------|------|
| 2.750 | 1.57 | 8.833  | 3.38 | 114.917 | 3.62 | 21.00 | 1.45 |
| 2.833 | 1.57 | 8.917  | 3.38 | 115.000 | 3.62 | 21.00 | 1.45 |
| 2.917 | 1.57 | 9.000  | 3.38 | 115.083 | 3.62 | 21.17 | 1.45 |
| 3.000 | 1.57 | 9.083  | 3.38 | 115.167 | 3.62 | 21.25 | 1.45 |
| 3.083 | 1.57 | 9.167  | 3.38 | 115.250 | 3.62 | 21.33 | 1.45 |
| 3.167 | 1.57 | 9.250  | 3.38 | 115.333 | 3.62 | 21.42 | 1.45 |
| 3.250 | 1.57 | 9.333  | 3.38 | 115.417 | 3.62 | 21.50 | 1.45 |
| 3.333 | 1.57 | 9.417  | 3.38 | 115.500 | 3.62 | 21.58 | 1.45 |
| 3.417 | 1.57 | 9.500  | 3.38 | 115.583 | 3.62 | 21.67 | 1.45 |
| 3.500 | 1.57 | 9.583  | 3.38 | 115.667 | 3.62 | 21.75 | 1.45 |
| 3.583 | 1.57 | 9.667  | 3.38 | 115.750 | 3.62 | 21.83 | 1.45 |
| 3.667 | 1.57 | 9.750  | 3.38 | 115.833 | 3.62 | 21.92 | 1.45 |
| 3.750 | 1.57 | 9.833  | 3.38 | 115.917 | 3.62 | 22.00 | 1.45 |
| 3.833 | 1.57 | 9.917  | 3.38 | 116.000 | 3.62 | 22.08 | 1.45 |
| 3.917 | 1.57 | 10.000 | 3.38 | 116.083 | 3.62 | 22.17 | 1.45 |
| 4.000 | 1.57 | 10.083 | 3.38 | 116.167 | 3.62 | 22.25 | 1.45 |
| 4.083 | 1.57 | 10.167 | 3.38 | 116.250 | 3.62 | 22.33 | 1.45 |
| 4.167 | 1.57 | 10.250 | 3.38 | 116.333 | 3.62 | 22.42 | 1.45 |
| 4.250 | 1.57 | 10.333 | 3.38 | 116.417 | 3.62 | 22.50 | 1.45 |
| 4.333 | 1.57 | 10.417 | 3.38 | 116.500 | 3.62 | 22.58 | 1.45 |
| 4.417 | 1.57 | 10.500 | 3.38 | 116.583 | 3.62 | 22.67 | 1.45 |
| 4.500 | 1.57 | 10.583 | 3.38 | 116.667 | 3.62 | 22.75 | 1.45 |
| 4.583 | 1.57 | 10.667 | 3.38 | 116.750 | 3.62 | 22.83 | 1.45 |
| 4.667 | 1.57 | 10.750 | 3.38 | 116.833 | 3.62 | 22.92 | 1.45 |
| 4.750 | 1.57 | 10.833 | 3.38 | 116.917 | 3.62 | 23.00 | 1.45 |
| 4.833 | 1.57 | 10.917 | 3.38 | 117.000 | 3.62 | 23.08 | 1.45 |
| 4.917 | 1.57 | 11.000 | 3.38 | 117.083 | 3.62 | 23.17 | 1.45 |
| 5.000 | 1.57 | 11.083 | 3.38 | 117.167 | 3.62 | 23.25 | 1.45 |
| 5.083 | 1.57 | 11.167 | 3.38 | 117.250 | 3.62 | 23.33 | 1.45 |
| 5.167 | 1.57 | 11.250 | 3.38 | 117.333 | 3.62 | 23.42 | 1.45 |
| 5.250 | 1.57 | 11.333 | 3.38 | 117.417 | 3.62 | 23.50 | 1.45 |
| 5.333 | 1.57 | 11.417 | 3.38 | 117.500 | 3.62 | 23.58 | 1.45 |
| 5.417 | 1.57 | 11.500 | 3.38 | 117.583 | 3.62 | 23.67 | 1.45 |
| 5.500 | 1.57 | 11.583 | 3.38 | 117.667 | 3.62 | 23.75 | 1.45 |
| 5.583 | 1.57 | 11.667 | 3.38 | 117.750 | 3.62 | 23.83 | 1.45 |
| 5.667 | 1.57 | 11.750 | 3.38 | 117.833 | 3.62 | 23.92 | 1.45 |
| 5.750 | 1.57 | 11.833 | 3.38 | 117.917 | 3.62 | 24.00 | 1.45 |
| 5.833 | 1.57 | 11.917 | 3.38 | 118.000 | 3.62 | 24.08 | 1.45 |
| 5.917 | 1.57 | 12.000 | 3.38 | 118.083 | 3.62 | 24.17 | 1.45 |
| 6.000 | 1.57 | 12.083 | 3.38 | 118.167 | 3.62 | 24.25 | 1.45 |
| 6.083 | 1.57 | 12.167 | 3.38 | 118.250 | 3.62 | 24.33 | 1.45 |

|  |  |               |        |                      |
|--|--|---------------|--------|----------------------|
| RUNOFF VOLUME (mm)   |  | 115.77        | 65.37  | 95.61                |
| TOTAL RAINFALL (mm)  |  | 120.77        | 120.77 | 120.77               |
| RUNOFF COEFFICIENT   |  | 0.96          | 0.54   | 0.79                 |
| **** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!                      |  |               |        |                      |
| (1) CN PROCEDURE SELECTED FOR Pervious LOSSES:                               |  |               |        |                      |
| CN* = 74.0 Ia = Dep. Storage (Above)   |  |               |        |                      |
| (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. |  |               |        |                      |
| (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.                            |  |               |        |                      |
| -----  |  |               |        |                      |
| CALIB  |  |               |        |                      |
| STANDHWD ( @ 0314 )  |  | Area (ha)=    | 1.43   |                      |
| ID= 1 Dt= 5.0 min  |  | Total Imp(S)= | 50.00  | Dir. Conn.(%)= 50.00 |
| -----  |  |               |        |                      |
| IMPERVIOUS PervIOUS (1)  |  |               |        |                      |
| Surface Area (ha)=   |  | 0.72          | 0.72   |                      |
| Dep. Storage (mm)=   |  | 2.00          | 5.00   |                      |
| Average Slope (%)=   |  | 9.50          | 2.00   |                      |
| Length (m)=  |  | 97.74         | 30.00  |                      |
| Mannings n =   |  | 0.013         | 0.250  |                      |
| NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.                        |  |               |        |                      |
| -----  |  |               |        |                      |
| TIME RAIN   TIME RAIN   TIME RAIN   TIME RAIN   TIME RAIN                    |  |               |        |                      |
| 0.083 0.00   6.167 1.93   12.250 147.82   18.33 2.17   24.42 2.17            |  |               |        |                      |
| 0.167 0.00   6.250 1.93   12.333 17.41   18.42 2.17   24.50 2.17             |  |               |        |                      |
| 0.250 0.00   6.333 2.17   12.417 17.39   18.50 2.17   24.58 2.17             |  |               |        |                      |
| 0.333 1.33   6.417 2.17   12.500 17.39   18.58 2.17   24.67 2.17             |  |               |        |                      |
| 0.417 1.33   6.500 2.17   12.583 17.39   18.67 2.17   24.75 2.17             |  |               |        |                      |
| 0.500 1.33   6.583 2.17   12.667 17.39   18.75 2.17   24.83 2.17             |  |               |        |                      |
| 0.583 1.33   6.667 2.17   12.750 17.39   18.83 2.17   24.92 2.17             |  |               |        |                      |
| 0.667 1.33   6.750 2.17   12.833 8.94   18.92 2.17   25.00 2.17              |  |               |        |                      |
| 0.750 1.33   6.833 2.17   12.917 8.94   19.00 2.17   25.08 2.17              |  |               |        |                      |
| 0.833 1.33   6.917 2.17   13.000 8.94   19.08 2.17   25.17 2.17              |  |               |        |                      |
| 0.917 1.33   7.000 2.17   13.083 8.94   19.17 2.17   25.25 2.17              |  |               |        |                      |
| 1.000 1.33   7.083 2.17   13.167 8.94   19.25 2.17   25.33 2.17              |  |               |        |                      |
| 1.083 1.33   7.167 2.17   13.250 8.94   19.33 2.17   25.42 2.17              |  |               |        |                      |
| 1.167 1.33   7.250 2.17   13.333 6.52   19.42 2.17   25.50 2.17              |  |               |        |                      |
| 1.250 1.33   7.333 2.17   13.417 6.52   19.50 2.17   25.58 2.17              |  |               |        |                      |
| 1.333 1.33   7.417 2.17   13.500 6.52   19.58 2.17   25.67 2.17              |  |               |        |                      |
| 1.417 1.33   7.500 2.17   13.583 6.52   19.67 2.17   25.75 2.17              |  |               |        |                      |
| 1.500 1.33   7.583 2.17   13.667 6.52   19.75 2.17   25.83 2.17              |  |               |        |                      |
| 1.583 1.33   7.667 2.17   13.750 6.52   19.83 2.17   25.92 2.17              |  |               |        |                      |

|       |      |        |      |         |      |       |      |
|-------|------|--------|------|---------|------|-------|------|
| 1.667 | 1.33 | 7.750  | 2.66 | 113.833 | 5.07 | 19.92 | 1.17 |
| 1.750 | 1.33 | 7.833  | 2.66 | 113.917 | 5.07 | 20.00 | 1.17 |
| 1.833 | 1.33 | 7.917  | 2.66 | 114.000 | 5.07 | 20.08 | 1.17 |
| 1.917 | 1.33 | 8.000  | 2.66 | 114.083 | 5.07 | 20.17 | 1.17 |
| 2.000 | 1.33 | 8.083  | 2.66 | 114.167 | 5.07 | 20.25 | 1.17 |
| 2.083 | 1.33 | 8.167  | 2.66 | 114.250 | 5.07 | 20.33 | 1.45 |
| 2.167 | 1.33 | 8.250  | 2.66 | 114.333 | 3.62 | 20.42 | 1.45 |
| 2.250 | 1.33 | 8.333  | 3.14 | 114.417 | 3.62 | 20.50 | 1.45 |
| 2.333 | 1.57 | 8.417  | 3.14 | 114.500 | 3.62 | 20.58 | 1.45 |
| 2.417 | 1.57 | 8.500  | 3.14 | 114.583 | 3.62 | 20.67 | 1.45 |
| 2.500 | 1.57 | 8.583  | 3.14 | 114.667 | 3.62 | 20.75 | 1.45 |
| 2.583 | 1.57 | 8.667  | 3.14 | 114.750 | 3.62 | 20.83 | 1.45 |
| 2.667 | 1.57 | 8.750  | 3.14 | 114.833 | 3.62 | 20.92 | 1.45 |
| 2.750 | 1.57 | 8.833  | 3.14 | 114.917 | 3.62 | 21.00 | 1.45 |
| 2.833 | 1.57 | 8.917  | 3.14 | 115.000 | 3.62 | 21.08 | 1.45 |
| 2.917 | 1.57 | 9.000  | 3.14 | 115.083 | 3.62 | 21.17 | 1.45 |
| 3.000 | 1.57 | 9.083  | 3.14 | 115.167 | 3.62 | 21.25 | 1.45 |
| 3.083 | 1.57 | 9.167  | 3.14 | 115.250 | 3.62 | 21.33 | 1.45 |
| 3.167 | 1.57 | 9.250  | 3.14 | 115.333 | 3.62 | 21.42 | 1.45 |
| 3.250 | 1.57 | 9.333  | 3.14 | 115.417 | 3.62 | 21.50 | 1.45 |
| 3.333 | 1.57 | 9.417  | 3.14 | 115.500 | 3.62 | 21.58 | 1.45 |
| 3.417 | 1.57 | 9.500  | 3.14 | 115.583 | 3.62 | 21.67 | 1.45 |
| 3.500 | 1.57 | 9.583  | 3.14 | 115.667 | 3.62 | 21.75 | 1.45 |
| 3.583 | 1.57 | 9.667  | 3.14 | 115.750 | 3.62 | 21.83 | 1.45 |
| 3.667 | 1.57 | 9.750  | 3.14 | 115.833 | 3.62 | 21.92 | 1.45 |
| 3.750 | 1.57 | 9.833  | 3.14 | 115.917 | 3.62 | 22.00 | 1.45 |
| 3.833 | 1.57 | 9.917  | 3.14 | 116.000 | 3.62 | 22.08 | 1.45 |
| 3.917 | 1.57 | 10.000 | 3.14 | 116.083 | 3.62 | 22.17 | 1.45 |
| 4.000 | 1.57 | 10.083 | 3.14 | 116.167 | 3.62 | 22.25 | 1.45 |
| 4.083 | 1.57 | 10.167 | 3.14 | 116.250 | 3.62 | 22.33 | 1.45 |
| 4.167 | 1.57 | 10.250 | 3.14 | 116.333 | 3.62 | 22.42 | 1.45 |
| 4.250 | 1.57 | 10.333 | 3.14 | 116.417 | 3.62 | 22.50 | 1.45 |
| 4.333 | 1.57 | 10.417 | 3.14 | 116.500 | 3.62 | 22.58 | 1.45 |
| 4.417 | 1.57 | 10.500 | 3.14 | 116.583 | 3.62 | 22.67 | 1.45 |
| 4.500 | 1.57 | 10.583 | 3.14 | 116.667 | 3.62 | 22.75 | 1.45 |
| 4.583 | 1.57 | 10.667 | 3.14 | 116.750 | 3.62 | 22.83 | 1.45 |
| 4.667 | 1.57 | 10.750 | 3.14 | 116.833 | 3.62 | 22.92 | 1.45 |
| 4.750 | 1.57 | 10.833 | 3.14 | 116.917 | 3.62 | 23.00 | 1.45 |
| 4.833 | 1.57 | 10.917 | 3.14 | 117.000 | 3.62 | 23.08 | 1.45 |
| 4.917 | 1.57 | 11.000 | 3.14 | 117.083 | 3.62 | 23.17 | 1.45 |
| 5.000 | 1.57 | 11.083 | 3.14 | 117.167 | 3.62 | 23.25 | 1.45 |
| 5.083 | 1.57 | 11.167 | 3.14 | 117.250 | 3.62 | 23.33 | 1.45 |
| 5.167 | 1.57 | 11.250 | 3.14 | 117.333 | 3.62 | 23.42 | 1.45 |
| 5.250 | 1.57 | 11.333 | 3.14 | 117.417 | 3.62 | 23.50 | 1.45 |
| 5.333 | 1.57 | 11.417 | 3.14 | 117.500 | 3.62 | 23.58 | 1.45 |
| 5.417 | 1.57 | 11.500 | 3.14 | 117.583 | 3.62 | 23.67 | 1.45 |
| 5.500 | 1.57 | 11.583 | 3.14 | 117.667 | 3.62 | 23.75 | 1.45 |
| 5.583 | 1.57 | 11.667 | 3.14 | 117.750 | 3.62 | 23.83 | 1.45 |
| 5.667 | 1.57 | 11.750 | 3.14 | 117.833 | 3.62 | 23.92 | 1.45 |
| 5.750 | 1.57 | 11.833 | 3.14 | 117.917 | 3.62 | 24.00 | 1.45 |
| 5.833 | 1.57 | 11.917 | 3.14 | 118.000 | 3.62 | 24.08 | 1.45 |
| 5.917 | 1.57 | 12.000 | 3.14 | 118.083 | 3.62 | 24.17 | 1.45 |
| 6.000 | 1.57 | 12.083 | 3.14 | 118.167 | 3.62 | 24.25 | 1.45 |
| 6.083 | 1.57 | 12.167 | 3.14 | 118.250 | 3.62 | 24.33 | 1.45 |

|       |      |        |      |         |      |       |      |
|-------|------|--------|------|---------|------|-------|------|
| 5.833 | 1.57 | 11.917 | 3.57 | 118.000 | 2.17 | 24.08 | 1.45 |
| 5.917 | 1.57 | 12.000 | 3.57 | 118.083 | 2.17 | 24.17 | 1.45 |
| 6.000 | 1.57 | 12.083 | 3.57 | 118.167 | 2.17 | 24.25 | 1.45 |
| 6.083 | 1.57 | 12.167 | 3.57 | 118.250 | 2.17 | 24.33 | 1.45 |

|                        |  |           |            |
|------------------------|--|-----------|------------|
| Max.Eff.Inten.(mm/hr)= |  | 147.82    | 96.52      |
| over (min)             |  | 5.00      | 10.00      |
| Storage Coeff. (min)=  |  | 2.94 (11) | 10.10 (11) |
| Unit Hyd. Tpeak (mm)   |  | 5.00      | 10.00      |
| Unit Hyd. peak (cms)   |  | 0.28      | 0.30       |

|                      |  |        |        |
|----------------------|--|--------|--------|
| PEAK FLOW (cms)=     |  | 0.99   | 0.29   |
| TIME TO PEAK (hrs)=  |  | 12.25  | 12.25  |
| TOTAL RAINFALL (mm)= |  | 120.77 | 120.77 |
| RUNOFF COEFFICIENT = |  | 0.98   | 0.54   |

\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR Pervious LOSSES:

CN\* = 74.0 Ia = Dep. Storage (Above)

(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

|   |  |  |  |  |
|---|--|--|--|--|
| NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.             |  |  |  |  |
| -----   |  |  |  |  |
| TIME RAIN   TIME RAIN   TIME RAIN   TIME RAIN   TIME RAIN         |  |  |  |  |
| 0.083 0.00   6.167 1.93   12.250 147.82   18.33 2.17   24.42 2.17 |  |  |  |  |
| 0.167 0.00   6.250 1.93   12.333 17.41   18.42 2.17   24.50 2.17  |  |  |  |  |
| 0.250 0.00   6.333 2.17   12.417 17.39   18.50 2.17   24.58 2.17  |  |  |  |  |
| 0.333 1.33   6.417 2.17   12.500 17.39   18.58 2.17   24.67 2.17  |  |  |  |  |
| 0.417 1.33   6.500 2.17   12.583 17.39   18.67 2.17   24.75 2.17  |  |  |  |  |
| 0.500 1.33   6.583 2.17   12.667 17.39   18.75 2.17   24.83 2.17  |  |  |  |  |
| 0.583 1.33   6.667 2.17   12.750 17.39   18.83 2.17   24.92 2.17  |  |  |  |  |
| 0.667 1.33   6.750 2.17   12.833 8.94   18.92 2.17   25.00 2.17   |  |  |  |  |
| 0.750 1.33   6.833 2.17   12.917 8.94   19.00 2.17   25.08 2.17   |  |  |  |  |
| 0.833 1.33   6.917 2.17   13.000 8.94   19.08 2.17   25.17 2.17   |  |  |  |  |
| 0.917 1.33   7.000 2.17   13.083 8.94   19.17 2.17   25.25 2.17   |  |  |  |  |
| 1.000 1.33   7.083 2.17   13.167 8.94   19.25 2.17   25.33 2.17   |  |  |  |  |
| 1.083 1.33   7.167 2.17   13.250 8.94   19.33 2.17   25.42 2.17   |  |  |  |  |
| 1.167 1.33   7.250 2.17   13.333 6.52   19.42 2.17   25.50 2.17   |  |  |  |  |
| 1.250 1.33   7.333 2.17   13.417 6.52   19.50 2.17   25.58 2.17   |  |  |  |  |
| 1.333 1.33   7.417 2.17   13.500 6.52   19.58 2.17   25.67 2.17   |  |  |  |  |
| 1.417 1.33   7.500 2.17   13.583 6.52   19.67 2.17   25.75 2.17   |  |  |  |  |
|   |  |  |  |  |

|       |      |        |        |         |      |       |      |
|-------|------|--------|--------|---------|------|-------|------|
| 3.833 | 1.57 | 9.917  | 4.35   | 116.000 | 3.62 | 22.08 | 1.45 |
| 3.917 | 1.57 | 10.000 | 4.35   | 116.083 | 3.62 | 22.17 | 1.45 |
| 4.000 | 1.57 | 10.083 | 4.35   | 116.167 | 3.62 | 22.25 | 1.45 |
| 4.083 | 1.57 | 10.167 | 4.35   | 116.250 | 3.62 | 22.33 | 1.45 |
| 4.167 | 1.57 | 10.250 | 4.35   | 116.333 | 3.62 | 22.42 | 1.45 |
| 4.250 | 1.57 | 10.333 | 5.56   | 116.417 | 2.17 | 22.50 | 1.45 |
| 4.333 | 1.59 | 10.417 | 5.56   | 116.500 | 2.17 | 22.58 | 1.45 |
| 4.417 | 1.59 | 10.500 | 5.56   | 116.583 | 2.17 | 22.67 | 1.45 |
| 4.500 | 1.59 | 10.583 | 5.56   | 116.667 | 2.17 | 22.75 | 1.45 |
| 4.583 | 1.59 | 10.667 | 5.56   | 116.750 | 2.17 | 22.83 | 1.45 |
| 4.667 | 1.59 | 10.750 | 5.56   | 116.833 | 2.17 | 22.92 | 1.45 |
| 4.750 | 1.59 | 10.833 | 7.49   | 116.917 | 2.17 | 23.00 | 1.45 |
| 4.833 | 1.59 | 10.917 | 7.49   | 117.000 | 2.17 | 23.08 | 1.45 |
| 4.917 | 1.59 | 11.000 | 7.49   | 117.083 | 2.17 | 23.17 | 1.45 |
| 5.000 | 1.59 | 11.083 | 7.49   | 117.167 | 2.17 | 23.25 | 1.45 |
| 5.083 | 1.59 | 11.167 | 7.49   | 117.250 | 2.17 | 23.33 | 1.45 |
| 5.167 | 1.59 | 11.250 | 7.49   | 117.333 | 2.17 | 23.42 | 1.45 |
| 5.250 | 1.59 | 11.333 | 11.59  | 117.417 | 2.17 | 23.50 | 1.45 |
| 5.333 | 1.59 | 11.417 | 11.59  | 117.500 | 2.17 | 23.58 | 1.45 |
| 5.417 | 1.59 | 11.500 | 11.59  | 117.583 | 2.17 | 23.67 | 1.45 |
| 5.500 | 1.59 | 11.583 | 11.59  | 117.667 | 2.17 | 23.75 | 1.45 |
| 5.583 | 1.59 | 11.667 | 11.59  | 117.750 | 2.17 | 23.83 | 1.45 |
| 5.667 | 1.59 | 11.750 | 11.59  | 117.833 | 2.17 | 23.92 | 1.45 |
| 5.750 | 1.59 | 11.833 | 35.75  | 117.917 | 2.17 | 24.00 | 1.45 |
| 5.833 | 1.59 | 11.917 | 35.75  | 118.000 | 2.17 | 24.08 | 1.45 |
| 5.917 | 1.59 | 12.000 | 35.75  | 118.083 | 2.17 | 24.17 | 1.45 |
| 6.000 | 1.59 | 12.083 | 147.81 | 118.167 | 2.17 | 24.25 | 1.45 |
| 6.083 | 1.59 | 12.167 | 147.81 | 118.250 | 2.17 |       |      |

Max. Eff. Infil. (m/hr) = 147.82 277.87  
 over (min) = 5.00 10.00  
 Storage Coeff. (min) = 1.65 (11) 6.34 (11)  
 Unit Hyd. Tpeak (min) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.32 0.15

\*TOTALS\*  
 PEAK FLOW (cms) = 0.06 0.13 0.192 (111)  
 TIME TO PEAK (hrs) = 12.25 12.25 12.25  
 RUNOFF VOLUME (mm) = 119.77 88.34 96.19  
 TOTAL RAINFALL (mm) = 120.77 120.77 120.77  
 RUNOFF COEFFICIENT = 0.99 0.73 0.80

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:  
 CN = 74.0 Ia = Dep. Storage (Above)  
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0331)  
 Inlet Cap. = 0.132  
 # of Inlets = 1  
 Total (cms) = 0.1 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 TOTAL HYD. (ID= 1): 0.59 0.19 12.25 96.19

MAJOR SVS. (ID= 2): 0.05 0.08 12.25 96.19  
 MINOR SVS. (ID= 3): 0.54 0.11 12.08 96.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Retention ( 0329) | OUTFLOW: OFF UNDERDRAIN: ON  
 In = 2 -> OUT = 3  
 DT = 5.0 min

SURFACE PONDING LAYER:  
 Max. Ponding Storage (cu.m.) = 149.95

| STAGE | AREA    | STAGE | AREA    |
|-------|---------|-------|---------|
| (m)   | (m2)    | (m)   | (m2)    |
| 0.000 | 100.000 | 1.500 | 100.000 |
| 0.500 | 100.000 | 0.000 | 0.000   |
| 1.000 | 100.000 | 0.000 | 0.000   |

MULCH LAYER:  
 Depth (m) = 0.00 Porosity = 0.40  
 Maximum Mulch Layer Volume (cu.m.) = 0.00

ENGINEERED SOIL LAYER:  
 Soil moisture = 0.30 Depth (m) = 0.72  
 Width (m) = 3.00 Length (m) = 240.00  
 Porosity = 0.47 Infiltration (m/hr) = 0.0060  
 Maximum Engineered Soil Layer Volume (cu.m.) = 242.09

| DEPTH | DISCHARGE | DEPTH | DISCHARGE |
|-------|-----------|-------|-----------|
| (m)   | (cms)     | (m)   | (cms)     |
| 0.000 | 0.000     | 0.559 | 0.030     |
| 0.279 | 0.010     | 0.762 | 0.040     |
| 0.406 | 0.020     | 1.300 | 0.050     |

STORAGE LAYER:  
 Depth (m) = 1.00 Porosity = 0.40  
 Seepage (m/hr) = 0.0010  
 Maximum Storage Layer Volume (cu.m.) = 288.00

TOTAL AVAILABLE STORAGE:

TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil  
 +Storage Layer (cu.m.) = 680.04

NATIVE SOIL LAYER:  
 Infiltration (m/hr) = 0.0060

| AREA            | QPEAK | TPEAK | R.V.  |
|-----------------|-------|-------|-------|
| (ha)            | (cms) | (hrs) | (mm)  |
| INFLOW: ID= 2   | 0.54  | 0.112 | 12.08 |
| OUTFLOW: ID= 1  | 0.19  | 0.009 | 0.08  |
| OVERFLOW: ID= 3 | 0.36  | 0.111 | 12.25 |

Volume Reduction Rate (RvIn-RvOut)/RvIn (%) = 19.40  
 Time to reach Max Ponding Storage (hr) = 0.00  
 Volume of water for drawdown in IID (cu.m.) = 156.54  
 Volume of Max. Water Storage (cu.m.) = 0.00  
 Maximum Surface Ponding and Mulch Vol (cu.m.) = 149.95  
 Maximum Engineered Soil Volume (cu.m.) = 60.86  
 \*\*\*\*\* After simulation, water volume is not zero.

Junction Command (0330)

| AREA                  | QPEAK | TPEAK | R.V.  |
|-----------------------|-------|-------|-------|
| (ha)                  | (cms) | (hrs) | (mm)  |
| INFLOW : ID= 3( 0329) | 0.36  | 0.11  | 12.25 |
| OUTFLOW: ID= 2( 0330) | 0.36  | 0.11  | 12.25 |

Junction Command (0332)

| AREA                  | QPEAK | TPEAK | R.V. |
|-----------------------|-------|-------|------|
| (ha)                  | (cms) | (hrs) | (mm) |
| INFLOW : ID= 1( 0329) | 0.19  | 0.01  | 0.08 |
| OUTFLOW: ID= 2( 0332) | 0.19  | 0.01  | 0.08 |

ADD HYD ( 3106)  
 1 + 2 + 3  
 ID1 = 1 ( 0330): 0.36 0.111 12.25 77.54  
 + ID2 = 2 ( 0331): 0.05 0.000 12.25 96.19

ID = 3 ( 3106): 0.41 0.191 12.25 79.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 3106)  
 3 + 2 + 1  
 ID1 = 3 ( 3106): 0.41 0.191 12.25 79.66  
 + ID2 = 2 ( 0332): 0.19 0.000 0.00 77.54  
 ID = 1 ( 3106): 0.59 0.192 12.25 79.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0332)  
 1 + 2 + 3  
 ID1 = 1 ( 0277): 1.15 0.270 12.08 88.59  
 + ID2 = 2 ( 0310): 5.48 1.080 12.25 94.68  
 ID = 3 ( 0332): 6.63 1.950 12.25 93.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0332)  
 3 + 2 + 1  
 ID1 = 3 ( 0332): 6.63 1.950 12.25 93.63  
 + ID2 = 2 ( 3106): 0.59 0.192 12.25 79.00  
 ID = 1 ( 0332): 7.22 2.142 12.25 92.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0332)  
 1 + 2 + 3  
 ID1 = 1 ( 0332): 7.22 2.142 12.25 92.43  
 + ID2 = 2 ( 0316): 14.21 4.870 12.25 100.94  
 ID = 3 ( 0332): 21.43 7.012 12.25 98.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR ( 0319) | OVERFLOW IS OFF  
 In = 2 -> OUT = 1  
 DT = 5.0 min

| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
|---------|---------|---------|---------|
| (cms)   | (ha.m)  | (cms)   | (ha.m)  |
| 0.0000  | 0.0000  | 0.0510  | 0.9050  |
| 0.0400  | 0.1140  | 1.0960  | 1.3320  |
| 0.0760  | 0.3690  | 1.4260  | 1.5860  |
| 0.2810  | 0.6450  | 0.0000  | 0.0000  |

| AREA                   | QPEAK  | TPEAK | R.V.  |
|------------------------|--------|-------|-------|
| (ha)                   | (cms)  | (hrs) | (mm)  |
| INFLOW : ID= 2 ( 0312) | 21.429 | 7.012 | 12.25 |
| OUTFLOW: ID= 1 ( 0319) | 21.429 | 0.868 | 12.83 |

PEAK FLOW REDUCTION [Qout/Qin](%) = 12.38  
 TIME SHIFT OF PEAK FLOW (min) = 35.00  
 MAXIMUM STORAGE USED (ha.m.) = 1.1524



NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: ADD HYD, AREA, QPEAK, TPEAK, R.V., ID#s, and values for various parameters.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: ADD HYD, AREA, QPEAK, TPEAK, R.V., ID#s, and values for various parameters.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: RESERVOIR, OVERFLOW IS OFF, DT, and values for various parameters.

Table with columns: INFLOW, OUTFLOW, PEAK FLOW REDUCTION, TIME SHIFT OF PEAK FLOW, and values for various parameters.

Table with columns: CALIB, STANDHYD, Area, and values for various parameters.

Table with columns: Surface Area, Dep. Storage, Average Slope, Length, Manning's n, and values for various parameters.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Large table with columns: TIME, RAIN, and values for various parameters over time.

Max. Eff. Inten. (mm/hr) over (min) Storage Coeff. (min) Unit Hyd. Tpeak (mm) Unit Hyd. peak (cms) PEAK FLOW (cms) TIME TO PEAK (hrs) RUNOFF VOLUME (mm) TOTAL RAINFALL (mm) RUMOFF COEFFICIENT

PEAK FLOW REDUCTION (Qout/Qin)(%) TIME SHIFT OF PEAK FLOW (min) MAXIMUM STORAGE USED (ha.m.)

CALIB, STANDHYD, Area, Total Imp, Dir. Conn, ImperVIOUS, PervIOUS

Table with columns: Area, QPEAK, TPEAK, R.V., ID#s, and values for various parameters.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

Large table with columns: TIME, RAIN, and values for various parameters over time.

Max. Eff. Inten. (mm/hr) over (min) Storage Coeff. (min) Unit Hyd. Tpeak (mm) Unit Hyd. peak (cms) PEAK FLOW (cms) TIME TO PEAK (hrs) RUNOFF VOLUME (mm) TOTAL RAINFALL (mm) RUMOFF COEFFICIENT

PEAK FLOW REDUCTION (Qout/Qin)(%) TIME SHIFT OF PEAK FLOW (min) MAXIMUM STORAGE USED (ha.m.)

CALIB, STANDHYD, Area, Total Imp, Dir. Conn, ImperVIOUS, PervIOUS

(1) CN PROCEDURE SELECTED FOR PervIOUS LOSSES: CN = 74.0 Ia = Dep. Storage (Above) (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD, AREA, QPEAK, TPEAK, R.V., ID#s, and values for various parameters.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANDHYD, Area, and values for various parameters.

Table with columns: Surface Area, Dep. Storage, Average Slope, Length, Manning's n, and values for various parameters.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TRANSFORMED HYETOGRAPH, TIME, RAIN, and values for various parameters over time.

Max. Eff. Inten. (mm/hr) over (min) Storage Coeff. (min) Unit Hyd. Tpeak (mm) Unit Hyd. peak (cms) PEAK FLOW (cms) TIME TO PEAK (hrs) RUNOFF VOLUME (mm) TOTAL RAINFALL (mm) RUMOFF COEFFICIENT

PEAK FLOW REDUCTION (Qout/Qin)(%) TIME SHIFT OF PEAK FLOW (min) MAXIMUM STORAGE USED (ha.m.)

CALIB, STANDHYD, Area, Total Imp, Dir. Conn, ImperVIOUS, PervIOUS

(1) CN PROCEDURE SELECTED FOR PervIOUS LOSSES: CN = 74.0 Ia = Dep. Storage (Above) (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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Max. Eff. Inten. (mm/hr) over (min) Storage Coeff. (min) Unit Hyd. Tpeak (mm) Unit Hyd. peak (cms) PEAK FLOW (cms) TIME TO PEAK (hrs) RUNOFF VOLUME (mm) TOTAL RAINFALL (mm) RUMOFF COEFFICIENT

PEAK FLOW REDUCTION (Qout/Qin)(%) TIME SHIFT OF PEAK FLOW (min) MAXIMUM STORAGE USED (ha.m.)

CALIB, STANDHYD, Area, Total Imp, Dir. Conn, ImperVIOUS, PervIOUS

(1) CN PROCEDURE SELECTED FOR PervIOUS LOSSES: CN = 74.0 Ia = Dep. Storage (Above) (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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Table with columns: TRANSFORMED HYETOGRAPH, TIME, RAIN, and values for various parameters over time.

Max. Eff. Inten. (mm/hr) over (min) Storage Coeff. (min) Unit Hyd. Tpeak (mm) Unit Hyd. peak (cms) PEAK FLOW (cms) TIME TO PEAK (hrs) RUNOFF VOLUME (mm) TOTAL RAINFALL (mm) RUMOFF COEFFICIENT

PEAK FLOW REDUCTION (Qout/Qin)(%) TIME SHIFT OF PEAK FLOW (min) MAXIMUM STORAGE USED (ha.m.)

CALIB, STANDHYD, Area, Total Imp, Dir. Conn, ImperVIOUS, PervIOUS

(1) CN PROCEDURE SELECTED FOR PervIOUS LOSSES: CN = 74.0 Ia = Dep. Storage (Above) (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD, AREA, QPEAK, TPEAK, R.V., ID#s, and values for various parameters.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANDHYD, Area, and values for various parameters.

Table with columns: Surface Area, Dep. Storage, Average Slope, Length, Manning's n, and values for various parameters.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TRANSFORMED HYETOGRAPH, TIME, RAIN, and values for various parameters over time.

Max. Eff. Inten. (mm/hr) over (min) Storage Coeff. (min) Unit Hyd. Tpeak (mm) Unit Hyd. peak (cms) PEAK FLOW (cms) TIME TO PEAK (hrs) RUNOFF VOLUME (mm) TOTAL RAINFALL (mm) RUMOFF COEFFICIENT

PEAK FLOW REDUCTION (Qout/Qin)(%) TIME SHIFT OF PEAK FLOW (min) MAXIMUM STORAGE USED (ha.m.)

CALIB, STANDHYD, Area, Total Imp, Dir. Conn, ImperVIOUS, PervIOUS

(1) CN PROCEDURE SELECTED FOR PervIOUS LOSSES: CN = 74.0 Ia = Dep. Storage (Above) (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD, AREA, QPEAK, TPEAK, R.V., ID#s, and values for various parameters.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, STANDHYD, Area, and values for various parameters.

Table with columns: Surface Area, Dep. Storage, Average Slope, Length, Manning's n, and values for various parameters.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TRANSFORMED HYETOGRAPH, TIME, RAIN, and values for various parameters over time.

Max. Eff. Inten. (mm/hr) over (min) Storage Coeff. (min) Unit Hyd. Tpeak (mm) Unit Hyd. peak (cms) PEAK FLOW (cms) TIME TO PEAK (hrs) RUNOFF VOLUME (mm) TOTAL RAINFALL (mm) RUMOFF COEFFICIENT

PEAK FLOW REDUCTION (Qout/Qin)(%) TIME SHIFT OF PEAK FLOW (min) MAXIMUM STORAGE USED (ha.m.)

CALIB, STANDHYD, Area, Total Imp, Dir. Conn, ImperVIOUS, PervIOUS

Hydrology data table with columns for time, rain, and flow. Includes a summary table at the bottom with values for Max. Eff. Inten., Storage Coeff., and Unit Hyd. Peak.

Hydrology data table with columns for time, rain, and flow. Includes a summary table at the bottom with values for Max. Eff. Inten., Storage Coeff., and Unit Hyd. Peak.

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Hydrology data table with columns for time, rain, and flow. Includes a summary table at the bottom with values for Max. Eff. Inten., Storage Coeff., and Unit Hyd. Peak.

Hydrology report showing runoff volume, total rainfall, and runoff coefficient. Includes a warning: STORAGE COEFF. IS SMALLER THAN TIME STEP! and a table of time series data for rain and runoff.

Hydrology report showing impervious and pervious areas, storage, and slope. Includes a table of time series data for rain and runoff.

Hydrology report showing impervious and pervious areas, storage, and slope. Includes a table of time series data for rain and runoff.

Hydrology report showing infiltration, storage, and peak flow. Includes a table of time series data for rain and runoff.

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|       |      |        |        |         |      |       |      |
|-------|------|--------|--------|---------|------|-------|------|
| 3.833 | 1.73 | 9.917  | 4.78   | 116.000 | 3.98 | 22.08 | 1.59 |
| 3.917 | 1.73 | 10.000 | 4.78   | 116.083 | 3.98 | 22.17 | 1.59 |
| 4.000 | 1.73 | 10.083 | 4.78   | 116.167 | 3.98 | 22.25 | 1.59 |
| 4.083 | 1.73 | 10.167 | 4.78   | 116.250 | 3.98 | 22.33 | 1.59 |
| 4.167 | 1.73 | 10.250 | 4.78   | 116.333 | 3.98 | 22.42 | 1.59 |
| 4.250 | 1.73 | 10.333 | 6.11   | 116.417 | 2.39 | 22.50 | 1.59 |
| 4.333 | 2.12 | 10.417 | 6.11   | 116.500 | 2.39 | 22.58 | 1.59 |
| 4.417 | 2.12 | 10.500 | 6.11   | 116.583 | 2.39 | 22.67 | 1.59 |
| 4.500 | 2.12 | 10.583 | 6.11   | 116.667 | 2.39 | 22.75 | 1.59 |
| 4.583 | 2.12 | 10.667 | 6.11   | 116.750 | 2.39 | 22.83 | 1.59 |
| 4.667 | 2.12 | 10.750 | 6.11   | 116.833 | 2.39 | 22.92 | 1.59 |
| 4.750 | 2.12 | 10.833 | 8.23   | 116.917 | 2.39 | 23.00 | 1.59 |
| 4.833 | 2.12 | 10.917 | 8.23   | 117.000 | 2.39 | 23.08 | 1.59 |
| 4.917 | 2.12 | 11.000 | 8.23   | 117.083 | 2.39 | 23.17 | 1.59 |
| 5.000 | 2.12 | 11.083 | 8.23   | 117.167 | 2.39 | 23.25 | 1.59 |
| 5.083 | 2.12 | 11.167 | 8.23   | 117.250 | 2.39 | 23.33 | 1.59 |
| 5.167 | 2.12 | 11.250 | 8.23   | 117.333 | 2.39 | 23.42 | 1.59 |
| 5.250 | 2.12 | 11.333 | 12.74  | 117.417 | 2.39 | 23.50 | 1.59 |
| 5.333 | 2.12 | 11.417 | 12.74  | 117.500 | 2.39 | 23.58 | 1.59 |
| 5.417 | 2.12 | 11.500 | 12.74  | 117.583 | 2.39 | 23.67 | 1.59 |
| 5.500 | 2.12 | 11.583 | 12.74  | 117.667 | 2.39 | 23.75 | 1.59 |
| 5.583 | 2.12 | 11.667 | 12.74  | 117.750 | 2.39 | 23.83 | 1.59 |
| 5.667 | 2.12 | 11.750 | 12.74  | 117.833 | 2.39 | 23.92 | 1.59 |
| 5.750 | 2.12 | 11.833 | 39.29  | 117.917 | 2.39 | 24.00 | 1.59 |
| 5.833 | 2.12 | 11.917 | 39.29  | 118.000 | 2.39 | 24.08 | 1.59 |
| 5.917 | 2.12 | 12.000 | 39.29  | 118.083 | 2.39 | 24.17 | 1.59 |
| 6.000 | 2.12 | 12.083 | 162.46 | 118.167 | 2.39 | 24.25 | 1.59 |
| 6.083 | 2.12 | 12.167 | 162.46 | 118.250 | 2.39 |       |      |

Max. Eff. Inten. (m/hr) = 162.47 311.50  
 over (min) = 5.00 10.00  
 Storage Coeff. (min) = 1.59 (11) 6.07 (11)  
 Unit Hyd. Tpeak (min) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.33 0.15

PEAK FLOW (cms) = 0.07 0.15 \*TOTALS\*  
 TIME TO PEAK (hrs) = 12.25 12.25 0.217 (111)  
 RUNOFF VOLUME (mm) = 131.74 99.59 107.62  
 TOTAL RAINFALL (mm) = 132.74 132.74 132.74  
 RUNOFF COEFFICIENT = 0.99 0.75 0.81

\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN THE STEP!  
 (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN = 74.0 Ia = Dep. Storage (Above)  
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
 DUHYD ( 0331)  
 Inlet Cap. = 0.132  
 # of Inlets = 1  
 Total (cms) = 0.11 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 TOTAL HYD. (ID= 1): 0.59 0.22 12.25 107.62  
 MAJOR SVS. (ID= 2): 0.06 0.10 12.25 107.62  
 MINOR SVS. (ID= 3): 0.53 0.11 12.08 107.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
 -----  
 [Retention ( 0329) ] OUTFLOW: OFF UNDERDRAIN: ON  
 [In 2--> OUT= 3 ]  
 [DT= 5.0 min ]

-----  
 SURFACE PONDING LAYER:  
 Max. Ponding Storage (cu.m.) = 149.95  
 STAGE AREA STAGE AREA  
 (m) (m2) (m) (m2)  
 0.000 100.000 1.500 100.000  
 0.500 100.000 1.000 0.000  
 1.000 100.000 0.000 0.000

MULCH LAYER:  
 Depth (m) = 0.00 Porosity = 0.40  
 Maximum Mulch Layer Volume (cu.m.) = 0.00  
 ENGINEERED SOIL LAYER:  
 Soil moisture = 0.30 Depth (m) = 0.72  
 Width (m) = 3.00 Length (m) = 240.00  
 Porosity = 0.47 Infiltration (m/hr) = 0.0060  
 Maximum Engineered Soil Layer Volume (cu.m.) = 242.09

DEPTH DISCHARGE DEPTH DISCHARGE  
 (m) (cms) (m) (cms)  
 0.000 0.000 1.500 0.030  
 0.279 0.010 1.762 0.040  
 0.406 0.020 1.900 0.050

STORAGE LAYER:  
 Depth (m) = 1.00 Porosity = 0.40  
 Seepage (m/hr) = 0.0010  
 Maximum Storage Layer Volume (cu.m.) = 288.00  
 TOTAL AVAILABLE STORAGE:

TOTAL STORAGE=Surface Ponding + Mulch Layer + Engineered soil  
 +Storage Layer (cu.m.) = 680.04  
 NATIVE SOIL LAYER:  
 Infiltration (m/hr) = 0.0060  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 INFLOW: ID= 2 0.53 0.112 12.08 107.62  
 OUTFLOW: ID= 1 0.16 0.009 0.08 88.40  
 OVERFLOW: ID= 3 0.37 0.111 12.25 88.40

Volume Reduction Rate (RvIn-RvOut)/RvIn (%) = 17.86  
 Time to reach Max Ponding Storage (hr) = 0.00  
 Volume of water for drawdown in IID (cu.m.) = 156.56  
 Volume of Max. Water Storage (cu.m.) = 0.00  
 Maximum Surface Ponding and Mulch Vol. (cu.m.) = 149.95  
 Maximum Engineered Soil Volume (cu.m.) = 60.86  
 \*\*\*\* After simulation, water volume is not zero.

[ Junction Command (0330) ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 INFLOW : ID= 3( 0329) 0.37 0.11 12.25 88.40  
 OUTFLOW: ID= 2( 0330) 0.37 0.11 12.25 88.40

[ Junction Command (0332) ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 INFLOW : ID= 1( 0329) 0.16 0.01 0.08 88.40  
 OUTFLOW: ID= 2( 0332) 0.16 0.01 0.08 88.40

[ ADD HYD ( 3106) ]  
 [ 1 + 2 + 3 ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1 = 1 ( 0330): 0.37 0.111 12.25 88.40  
 + ID2 = 2 ( 0331): 0.06 0.105 12.25 107.62

ID = 3 ( 3106): 0.43 0.215 12.25 91.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ ADD HYD ( 3106) ]  
 [ 3 + 2 + 1 ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1 = 3 ( 3106): 0.43 0.215 12.25 91.20  
 + ID2 = 2 ( 0332): 0.16 0.009 0.08 88.40  
 ID = 1 ( 3106): 0.59 0.216 12.25 90.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ ADD HYD ( 0332) ]  
 [ 1 + 2 + 3 ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1 = 1 ( 0277): 1.13 0.270 12.08 99.59  
 + ID2 = 2 ( 0310): 5.48 1.974 12.25 105.74  
 ID = 3 ( 0332): 6.61 2.244 12.25 104.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ ADD HYD ( 0332) ]  
 [ 3 + 2 + 1 ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1 = 3 ( 0332): 6.61 2.244 12.25 104.69  
 + ID2 = 2 ( 3106): 0.59 0.216 12.25 90.42  
 ID = 1 ( 0332): 7.20 2.461 12.25 103.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ ADD HYD ( 0332) ]  
 [ 1 + 2 + 3 ]  
 AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1 = 1 ( 0332): 7.20 2.461 12.25 103.52  
 + ID2 = 2 ( 0316): 14.21 5.424 12.25 112.29  
 ID = 3 ( 0332): 21.40 7.884 12.25 109.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

[ RESERVOIR ( 0319) ] OVERFLOW IS OFF  
 [ In 2--> OUT= 1 ]  
 [DT= 5.0 min ]  
 OUTFLOW STORAGE OUTFLOW STORAGE  
 (cms) (ha.m) (cms) (ha.m.)  
 0.0000 0.0000 0.0510 0.9050  
 0.0400 0.1140 1.0960 1.3320  
 0.0760 0.3690 1.4260 1.5860  
 0.2810 0.6450 0.0000 0.0000

AREA QPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 INFLOW : ID= 2 ( 0312) 21.405 7.884 12.25 109.34  
 OUTFLOW: ID= 1 ( 0319) 21.405 1.019 12.75 109.31

PEAK FLOW REDUCTION [Out/In](%) = 12.93  
 TIME SHIFT OF PEAK FLOW (min) = 30.00  
 MAXIMUM STORAGE USED (ha.m.) = 1.2720

# APPENDIX E

## SWM Facility Calculations



**Water Quality Requirements**

Project #: 1060-6220  
 Project: Glenelg Expansion Lands  
 Date: 2022.08.22  
 By: KS

**Water Quality Requirements for Wet Pond**

| Areas Contributing                        | Area (ha)    | % Imp       | 25mm RV (mm) | 25mm RV (m <sup>3</sup> ) |
|---|--------------|-------------|--------------|---------------------------|
| Drainage to Tree Pits                     | 14.21        | 71.0        |              | 0                         |
| Drainage to SWM Pond                      | 4.05         | 60.0        |              | 0                         |
| SWM Pond Block                            | 1.43         | 50.0        |              | 0                         |
| Drainage to Perm Pavers Walkway           | 0.59         | 66.0        |              | 0                         |
| Drainage to Tree Pits                     | 1.28         | 70.0        |              | 0                         |
| <b>WEIGHTED IMP</b>                       | <b>21.55</b> | <b>67.3</b> | 15.54        | <b>3350</b>               |
| MOE Total WQ Volume (m <sup>3</sup> /ha)  |              |             |              | 219                       |
| MOE ED Volume (m <sup>3</sup> /ha)        |              |             |              | 40                        |
| MOE ED Volume (m <sup>3</sup> )           |              |             |              | 862                       |
| MOE PP Volume (m <sup>3</sup> /ha)        |              |             |              | 179                       |
| MOE PP Volume (m <sup>3</sup> )           |              |             |              | 3852                      |
| Pond Required ED Volume (m <sup>3</sup> ) |              |             |              | 3350                      |
| Pond Required PP Volume (m <sup>3</sup> ) |              |             |              | 3852                      |
| Available ED Volume (m <sup>3</sup> )     |              |             |              | 3368                      |
| Provided PP Volume (m <sup>3</sup> )      |              |             |              | 3990                      |

**EXTENDED DETENTION SPECIFICATIONS - SWM FACILITY (PER MECP)**

|   |                |                   |
|---|----------------|-------------------|
| Extended Detention Volume (Area x runoff from 25 mm storm event)    |                | 3350              |
| † (drawdown time - seconds, <i>hours in italics</i> )               | 24.0           | 86400             |
| Ao (cross section area of orifice - sqm)                            |                | 0.0314            |
| h (maximum water elevation above orifice for extended detention- m) |                | 0.83              |
| C (discharge coefficient)   |                | 0.64              |
| Ap (average surface area for extended detention - sqm)              |                | 4415              |
| <br>  |                |                   |
| $† = 2 * A_p * (h^{0.5}) / (C * A_o * (g * 2)^{0.5})$               |                |                   |
| Ao =  | 0.03284676 sqm | d = 205 mm        |
| Extended Detention Orifice Diameter (as designed)                   |                | d = <b>200</b> mm |

### SWMF STAGE STORAGE DISCHARGE

| Outlet Structure               |        |         |
|--------------------------------|--------|---------|
| E.D. Orifice Diameter:         | 0.200  | m       |
| E.D. Orifice Invert Elevation: | 518.20 | m       |
| V-notch angle                  | 0      | degrees |
| V-notch constant               | 0.00   | const   |
| V-notch invert                 | 0.00   | m       |
| Rect weir length               | 0.31   | m       |
| Rect weir invert               | 519.03 | m       |
| Extended Detention Depth:      | 0.83   | m       |

|            | Pond Dimensions |                          |               |                             | Outlet Structure                    |                                  |                                     | Cell Spillway                    |                                      | Total Discharge<br>(cu.m/s) | Storage<br>(ha-m) |
|------------|-----------------|--------------------------|---------------|-----------------------------|-------------------------------------|----------------------------------|-------------------------------------|----------------------------------|--------------------------------------|-----------------------------|-------------------|
|            | Elev.<br>(m)    | Depth<br>Above PP<br>(m) | Area<br>(sqm) | Storage<br>Volume<br>(cu.m) | ED Orifice<br>Discharge<br>(cu.m/s) | V-notch<br>Discharge<br>(cu.m/s) | Rect. Weir<br>Discharge<br>(cu.m/s) | Emerg. Weir<br>Ave. Width<br>(m) | Emerg. Weir<br>Discharge<br>(cu.m/s) |                             |                   |
| PP         | 518.20          | 0.00                     | 3460          | 0                           | 0.000                               | 0.000                            | 0.000                               | 0.00                             | 0.000                                | 0.000                       | 0.000             |
|            | 518.30          | 0.10                     | 3697          | 358                         | 0.000                               | 0.000                            | 0.000                               | 0.00                             | 0.000                                | 0.000                       | 0.036             |
|            | 518.40          | 0.20                     | 3934          | 739                         | 0.028                               | 0.000                            | 0.000                               | 0.00                             | 0.000                                | 0.028                       | 0.074             |
|            | 518.50          | 0.30                     | 4171          | 1145                        | 0.040                               | 0.000                            | 0.000                               | 0.00                             | 0.000                                | 0.040                       | 0.114             |
|            | 518.60          | 0.40                     | 4407          | 1573                        | 0.049                               | 0.000                            | 0.000                               | 0.00                             | 0.000                                | 0.049                       | 0.157             |
|            | 518.70          | 0.50                     | 4644          | 2026                        | 0.056                               | 0.000                            | 0.000                               | 0.00                             | 0.000                                | 0.056                       | 0.203             |
|            | 518.80          | 0.60                     | 4881          | 2502                        | 0.063                               | 0.000                            | 0.000                               | 0.00                             | 0.000                                | 0.063                       | 0.250             |
|            | 518.90          | 0.70                     | 5118          | 3002                        | 0.069                               | 0.000                            | 0.000                               | 0.00                             | 0.000                                | 0.069                       | 0.300             |
| ED         | 519.03          | 0.83                     | 5426          | 3688                        | 0.076                               | 0.000                            | 0.000                               | 0.00                             | 0.000                                | 0.076                       | 0.369             |
|            | 519.10          | 0.90                     | 5560          | 4072                        | 0.080                               | 0.000                            | 0.011                               | 0.00                             | 0.000                                | 0.090                       | 0.407             |
|            | 519.20          | 1.00                     | 5751          | 4638                        | 0.084                               | 0.000                            | 0.040                               | 0.00                             | 0.000                                | 0.124                       | 0.464             |
|            | 519.30          | 1.10                     | 5942          | 5222                        | 0.089                               | 0.000                            | 0.080                               | 0.00                             | 0.000                                | 0.169                       | 0.522             |
|            | 519.40          | 1.20                     | 6134          | 5826                        | 0.093                               | 0.000                            | 0.128                               | 0.00                             | 0.000                                | 0.222                       | 0.583             |
|            | 519.50          | 1.30                     | 6325          | 6449                        | 0.098                               | 0.000                            | 0.184                               | 0.00                             | 0.000                                | 0.281                       | 0.645             |
|            | 519.60          | 1.40                     | 6516          | 7091                        | 0.102                               | 0.000                            | 0.245                               | 0.00                             | 0.000                                | 0.347                       | 0.709             |
|            | 519.70          | 1.50                     | 6707          | 7752                        | 0.105                               | 0.000                            | 0.313                               | 0.00                             | 0.000                                | 0.418                       | 0.775             |
|            | 519.80          | 1.60                     | 6899          | 8433                        | 0.109                               | 0.000                            | 0.385                               | 0.00                             | 0.000                                | 0.494                       | 0.843             |
|            | 519.90          | 1.70                     | 7090          | 9132                        | 0.113                               | 0.000                            | 0.463                               | 0.00                             | 0.000                                | 0.576                       | 0.913             |
|            | 520.00          | 1.80                     | 7281          | 9851                        | 0.116                               | 0.000                            | 0.545                               | 0.00                             | 0.000                                | 0.661                       | 0.985             |
|            | 520.10          | 1.90                     | 7473          | 10588                       | 0.119                               | 0.000                            | 0.631                               | 0.00                             | 0.000                                | 0.751                       | 1.059             |
|            | 520.20          | 2.00                     | 7664          | 11345                       | 0.123                               | 0.000                            | 0.722                               | 0.00                             | 0.000                                | 0.845                       | 1.135             |
|            | 520.30          | 2.10                     | 7855          | 12121                       | 0.126                               | 0.000                            | 0.816                               | 0.00                             | 0.000                                | 0.942                       | 1.212             |
|            | 520.40          | 2.20                     | 8046          | 12916                       | 0.129                               | 0.000                            | 0.915                               | 0.00                             | 0.000                                | 1.044                       | 1.292             |
| 100-YR HWL | 520.45          | 2.25                     | 8142          | 13321                       | 0.131                               | 0.000                            | 0.965                               | 0.00                             | 0.000                                | 1.096                       | 1.332             |
|            | 520.55          | 2.35                     | 8353          | 14146                       | 0.134                               | 0.000                            | 1.069                               | 0.00                             | 0.000                                | 1.203                       | 1.415             |
|            | 520.65          | 2.45                     | 8563          | 14992                       | 0.137                               | 0.000                            | 1.176                               | 0.00                             | 0.000                                | 1.313                       | 1.499             |
| TOP        | 520.75          | 2.55                     | 8774          | 15858                       | 0.139                               | 0.000                            | 1.287                               | 0.00                             | 0.000                                | 1.426                       | 1.586             |



Project No: 1060-6220  
 Project: Glenelg Expansion Lands  
 File: Forebay Design  
 Design by: K. Swain  
 Date: 2022-08-26

### Forebay Design Calculations

|                                  | Variable   | Value       |           |
|----------------------------------|--|-------------|-----------|
| Forebay Settling Length          | Length of forebay (m)  | 50.0        |           |
|                                  | Average Width of forebay (m)                                     | 6.5         |           |
|                                  | Length-to-width ratio of forebay                                 | 7.7         |           |
|                                  | Peak flow rate from forebay in quality event (m <sup>3</sup> /s) | 0.076       |           |
|                                  | Settling velocity (m/s)  | 0.0003      |           |
|                                  |  |             |           |
|                                  | <b>Required Forebay Length (m)</b>                               |             | <b>44</b> |
| Dispersion Length                | Inlet flowrate in 5 year event (m <sup>3</sup> /s)               | 4.228       |           |
|                                  | Depth of of the permanent pool in the forebay (m)                | 1.58        |           |
|                                  | Desired velocity in the forebay (m/s)                            | 0.5         |           |
|                                  |  |             |           |
|                                  | <b>Length of Dispersion (m)</b>                                  | <b>43</b>   |           |
| Velocity in Forebay Check        | Depth of forebay in 10 year event (m) **                         | 1.70        |           |
|                                  | Cross sectional area (m <sup>2</sup> )                           | 16.2        |           |
|                                  | 10 Year Event Flowrate (m <sup>3</sup> /s)                       | 5.10        |           |
|                                  |  |             |           |
|                                  | <b>Velocity in Forebay (m/s)*</b>                                | <b>0.32</b> |           |
| Forebay Bottom Width             | Length of forebay (m)  | 50.0        |           |
|                                  |  |             |           |
|                                  | <b>Minimum Forebay Bottom Width (m)</b>                          | <b>6.25</b> |           |
| <b>DESIGN FOREBAY LENGTH (m)</b> |  | <b>50.0</b> |           |
| <b>DESIGN BOTTOM WIDTH (m)</b>   |  | <b>6.5</b>  |           |

\* Desired maximum average velocity in the forebay is 0.15 m/s, per MOE 2003, Page 4-56

# APPENDIX F

## Water Balance Calculations

**Water Budget - Pre-Development**  
**Project Name: Glenelg Expansion Lands**  
**Water Balance/Water Budget Assessment**

Pre-development area available for infiltration (landscaped/lawn area considered to infiltrate)  
 Pre-development area not available for infiltration (total site area less landscaped area noted above)  
 Remaining Impervious area (e.g. parking asphalt area)

Note: site land use areas consistent with Pre-Development SWM hydrologic modeling & calculations

| Catchment Designation                                     | Site                      |                                   |                                    |                                       |                                  | Glenelg Phase 2 Lands                               | Totals        |
|---|---------------------------|-----------------------------------|------------------------------------|---------------------------------------|----------------------------------|---|---------------|
|   | Pervious Area to CP Trail | Pervious Area to North Tile Drain | Pervious Area to South Residential | Pervious Area to Southeast Tile Drain | Pervious Area to East Tile Drain | Pervious Area from Glenelg Phase 2 Lands - External |               |
| Area (m <sup>2</sup> )                                    | 43200                     | 133300                            | 30000                              | 22900                                 | 30500                            | 7800  | 267700        |
| Pervious Area (m <sup>2</sup> )                           | 43200                     | 133300                            | 30000                              | 22900                                 | 30500                            | 7800  | 267700        |
| Impervious Area (m <sup>2</sup> )                         | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| <b>Infiltration Factors</b>                               |                           |                                   |                                    |                                       |                                  |   |               |
| Topography Infiltration Factor                            | 0.25                      | 0.25                              | 0.25                               | 0.25                                  | 0.25                             | 0.25  |               |
| Soil Infiltration Factor                                  | 0.2                       | 0.2                               | 0.2                                | 0.2                                   | 0.2                              | 0.2   |               |
| Land Cover Infiltration Factor                            | 0.1                       | 0.1                               | 0.1                                | 0.1                                   | 0.1                              | 0.1   |               |
| MOE Infiltration Factor                                   | 0.55                      | 0.55                              | 0.55                               | 0.55                                  | 0.55                             | 0.55  |               |
| Actual Infiltration Factor                                | 0.55                      | 0.55                              | 0.55                               | 0.55                                  | 0.55                             | 0.55  |               |
| Run-off Coefficient                                       | 0.25                      | 0.25                              | 0.25                               | 0.25                                  | 0.25                             | 0.25  |               |
| Runoff from Impervious Surfaces *                         | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   |               |
| <b>Inputs (per Unit Area)</b>                             |                           |                                   |                                    |                                       |                                  |   |               |
| Precipitation (mm/yr)                                     | 1106                      | 1106                              | 1106                               | 1106                                  | 1106                             | 1106  | 1106          |
| Run-On (mm/yr)  | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Other Inputs (mm/yr)                                      | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| <b>Total Inputs (mm/yr)</b>                               | <b>1106</b>               | <b>1106</b>                       | <b>1106</b>                        | <b>1106</b>                           | <b>1106</b>                      | <b>1106</b>   | <b>1106</b>   |
| <b>Outputs (per Unit Area)</b>                            |                           |                                   |                                    |                                       |                                  |   |               |
| Precipitation Surplus (mm/yr)                             | 573                       | 573                               | 573                                | 573                                   | 573                              | 573   | 573           |
| Net Surplus (mm/yr)                                       | 573                       | 573                               | 573                                | 573                                   | 573                              | 573   | 573           |
| Evapotranspiration (mm/yr) *                              | 533                       | 533                               | 533                                | 533                                   | 533                              | 533   | 533           |
| Infiltration (mm/yr)                                      | 315                       | 315                               | 315                                | 315                                   | 315                              | 315   | 315           |
| Topsoil Amendment Infiltration (mm/yr)                    | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Bioretention Infiltration (mm/yr)                         | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Tree Pit Infiltration (mm/yr)                             | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Permeable Paver Infiltration - Park (mm/yr)               | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Permeable Paver Infiltration - North (mm/yr)              | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Total Infiltration (mm/yr)                                | 315                       | 315                               | 315                                | 315                                   | 315                              | 315   | 315           |
| Runoff Pervious Areas (mm/yr)                             | 258                       | 258                               | 258                                | 258                                   | 258                              | 258   | 258           |
| Runoff Impervious Areas (mm/yr)                           | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Total Runoff (mm/yr)                                      | 258                       | 258                               | 258                                | 258                                   | 258                              | 258   | 258           |
| <b>Total Outputs (mm/yr)</b>                              | <b>1106</b>               | <b>1106</b>                       | <b>1106</b>                        | <b>1106</b>                           | <b>1106</b>                      | <b>1106</b>   | <b>1106</b>   |
| <b>Difference (Inputs - Outputs)</b>                      | <b>0</b>                  | <b>0</b>                          | <b>0</b>                           | <b>0</b>                              | <b>0</b>                         | <b>0</b>  | <b>0</b>      |
| <b>Inputs (Volumes)</b>                                   |                           |                                   |                                    |                                       |                                  |   |               |
| Precipitation (m <sup>3</sup> /yr)                        | 47788                     | 147456                            | 33186                              | 25332                                 | 33739                            | 8628  | 296130        |
| Run-On (m <sup>3</sup> /yr)                               | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Other Inputs (m <sup>3</sup> /yr)                         | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| <b>Total Inputs (m<sup>3</sup>/yr)</b>                    | <b>47788</b>              | <b>147456</b>                     | <b>33186</b>                       | <b>25332</b>                          | <b>33739</b>                     | <b>8628</b>   | <b>296130</b> |
| <b>Outputs (Volumes)</b>                                  |                           |                                   |                                    |                                       |                                  |   |               |
| Precipitation Surplus (m <sup>3</sup> /yr)                | 24766                     | 76420                             | 17199                              | 13128                                 | 17485                            | 4472  | 153470        |
| Net Surplus (m <sup>3</sup> /yr)                          | 24766                     | 76420                             | 17199                              | 13128                                 | 17485                            | 4472  | 153470        |
| Evapotranspiration (m <sup>3</sup> /yr) *                 | 23022                     | 71037                             | 15987                              | 12204                                 | 16254                            | 4157  | 142660        |
| Infiltration (m <sup>3</sup> /yr)                         | 13621                     | 42031                             | 9459                               | 7221                                  | 9617                             | 2459  | 84408         |
| Topsoil Amendment Infiltration (m <sup>3</sup> /yr)       | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Bioretention Infiltration (m <sup>3</sup> /yr)            | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Tree Pit Infiltration (m <sup>3</sup> /yr)                | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Permeable Paver Infiltration - Park (m <sup>3</sup> /yr)  | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Permeable Paver Infiltration - North (m <sup>3</sup> /yr) | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Total Infiltration (m <sup>3</sup> /yr)                   | 13621                     | 42031                             | 9459                               | 7221                                  | 9617                             | 2459  | 84408         |
| Runoff Pervious Areas (m <sup>3</sup> /yr)                | 11145                     | 34389                             | 7739                               | 5908                                  | 7868                             | 2012  | 69061         |
| Runoff Impervious Areas (m <sup>3</sup> /yr)              | 0                         | 0                                 | 0                                  | 0                                     | 0                                | 0   | 0             |
| Total Runoff (m <sup>3</sup> /yr)                         | 11145                     | 34389                             | 7739                               | 5908                                  | 7868                             | 2012  | 69061         |
| <b>Total Outputs (m<sup>3</sup>/yr)</b>                   | <b>47788</b>              | <b>147456</b>                     | <b>33186</b>                       | <b>25332</b>                          | <b>33739</b>                     | <b>8628</b>   | <b>296130</b> |
| <b>Difference (Inputs - Outputs)</b>                      | <b>0</b>                  | <b>0</b>                          | <b>0</b>                           | <b>0</b>                              | <b>0</b>                         | <b>0</b>  | <b>0</b>      |

**NOTES:**

\* Evaporation from impervious areas was assumed to be 20% of precipitation.



**Water Budget - Post-Development Without Mitigation**  
**Project Name: Glenelg Expansion Lands**  
**Water Balance/Water Budget Assessment**

Post-development area available for infiltration (landscaped/lawn area considered to infiltrate)  
 Post-development area not available for infiltration (total site area less landscaped area noted above)  
 Remaining Impervious area (e.g. parking asphalt area, building/rooftop area)

**Note:** site land use areas consistent with Post-Development SWM hydrologic modeling & calculations

| Catchment Designation                                     | Site          |                       |                      | Glenelg Phase 2 Lands |                 | Totals        |
|---|---------------|-----------------------|----------------------|-----------------------|-----------------|---------------|
|   | Pervious Area | Impervious Block Area | Impervious Road Area | Pervious Area         | Impervious Area |               |
| Area (m <sup>2</sup> )                                    | 99378         | 100744                | 59778                | 6318                  | 1482            | 267700        |
| Pervious Area (m <sup>2</sup> )                           | 99378         | 0                     | 0                    | 6318                  | 0               | 105696        |
| Impervious Area (m <sup>2</sup> )                         | 0             | 100744                | 59778                | 0                     | 1482            | 162004        |
| <b>Infiltration Factors</b>                               |               |                       |                      |                       |                 |               |
| Topography Infiltration Factor                            | 0.25          | 0                     | 0                    | 0.25                  | 0               |               |
| Soil Infiltration Factor                                  | 0.2           | 0                     | 0                    | 0.2                   | 0               |               |
| Land Cover Infiltration Factor                            | 0.1           | 0                     | 0                    | 0.1                   | 0               |               |
| MOE Infiltration Factor                                   | 0.55          | 0                     | 0                    | 0.55                  | 0               |               |
| Actual Infiltration Factor                                | 0.55          | 0                     | 0                    | 0.55                  | 0               |               |
| Run-off Coefficient                                       | 0.25          | 1                     | 1                    | 0.25                  | 1               |               |
| Runoff from Impervious Surfaces *                         | 0             | 0.8                   | 0.8                  | 0                     | 0.8             |               |
| <b>Inputs (per Unit Area)</b>                             |               |                       |                      |                       |                 |               |
| Precipitation (mm/yr)                                     | 1106          | 1106                  | 1106                 | 1106                  | 1106            | 1106          |
| Run-On (mm/yr)  | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| Other Inputs (mm/yr)                                      | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| <b>Total Inputs (mm/yr)</b>                               | <b>1106</b>   | <b>1106</b>           | <b>1106</b>          | <b>1106</b>           | <b>1106</b>     | <b>1106</b>   |
| <b>Outputs (per Unit Area)</b>                            |               |                       |                      |                       |                 |               |
| Precipitation Surplus (mm/yr)                             | 573           | 885                   | 885                  | 573                   | 885             | 760           |
| Net Surplus (mm/yr)                                       | 573           | 885                   | 885                  | 573                   | 885             | 760           |
| Evapotranspiration (mm/yr) *                              | 533           | 221                   | 221                  | 533                   | 221             | 346           |
| Infiltration (mm/yr)                                      | 315           | 0                     | 0                    | 315                   | 0               | 126           |
| Topsoil Amendment Infiltration (mm/yr)                    | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| Bioretention Infiltration (mm/yr)                         | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| Tree Pit Infiltration (mm/yr)                             | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| Permeable Paver Infiltration - Park (mm/yr)               | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| Permeable Paver Infiltration - North (mm/yr)              | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| Total Infiltration (mm/yr)                                | 315           | 0                     | 0                    | 315                   | 0               | 126           |
| Runoff Pervious Areas (mm/yr)                             | 258           | 0                     | 0                    | 258                   | 0               | 103           |
| Runoff Impervious Areas (mm/yr)                           | 0             | 885                   | 885                  | 0                     | 885             | 531           |
| Total Runoff (mm/yr)                                      | 258           | 885                   | 885                  | 258                   | 885             | 634           |
| <b>Total Outputs (mm/yr)</b>                              | <b>1106</b>   | <b>1106</b>           | <b>1106</b>          | <b>1106</b>           | <b>1106</b>     | <b>1106</b>   |
| <b>Difference (Inputs- Outputs)</b>                       | <b>0</b>      | <b>0</b>              | <b>0</b>             | <b>0</b>              | <b>0</b>        | <b>0</b>      |
| <b>Inputs (Volumes)</b>                                   |               |                       |                      |                       |                 |               |
| Precipitation (m <sup>3</sup> /yr)                        | 109932        | 111443                | 66126                | 6989                  | 1639            | 296130        |
| Run-On (m <sup>3</sup> /yr)                               | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| Other Inputs (m <sup>3</sup> /yr)                         | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| <b>Total Inputs (m<sup>3</sup>/yr)</b>                    | <b>109932</b> | <b>111443</b>         | <b>66126</b>         | <b>6989</b>           | <b>1639</b>     | <b>296130</b> |
| <b>Outputs (Volumes)</b>                                  |               |                       |                      |                       |                 |               |
| Precipitation Surplus (m <sup>3</sup> /yr)                | 56972         | 89154                 | 52901                | 3422                  | 1312            | 203961        |
| Net Surplus (m <sup>3</sup> /yr)                          | 56972         | 89154                 | 52901                | 3422                  | 1312            | 203961        |
| Evapotranspiration (m <sup>3</sup> /yr) *                 | 52960         | 22289                 | 13225                | 3367                  | 328             | 92168         |
| Infiltration (m <sup>3</sup> /yr)                         | 31335         | 0                     | 0                    | 1992                  | 0               | 33327         |
| Topsoil Amendment Infiltration (m <sup>3</sup> /yr)       | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| Bioretention Infiltration (m <sup>3</sup> /yr)            | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| Tree Pit Infiltration (m <sup>3</sup> /yr)                | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| Permeable Paver Infiltration - Park (m <sup>3</sup> /yr)  | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| Permeable Paver Infiltration - North (m <sup>3</sup> /yr) | 0             | 0                     | 0                    | 0                     | 0               | 0             |
| Total Infiltration (m <sup>3</sup> /yr)                   | 31335         | 0                     | 0                    | 1992                  | 0               | 33327         |
| Runoff Pervious Areas (m <sup>3</sup> /yr)                | 25638         | 0                     | 0                    | 1630                  | 0               | 27268         |
| Runoff Impervious Areas (m <sup>3</sup> /yr)              | 0             | 89154                 | 52901                | 0                     | 1312            | 143367        |
| Total Runoff (m <sup>3</sup> /yr)                         | 25638         | 89154                 | 52901                | 1630                  | 1312            | 170634        |
| <b>Total Outputs (m<sup>3</sup>/yr)</b>                   | <b>109932</b> | <b>111443</b>         | <b>66126</b>         | <b>6989</b>           | <b>1639</b>     | <b>296130</b> |
| <b>Difference (Inputs- Outputs)</b>                       | <b>0</b>      | <b>0</b>              | <b>0</b>             | <b>0</b>              | <b>0</b>        | <b>0</b>      |

Pre-Development Total Infiltration:

84408 m<sup>3</sup>/yr

**NOTES:**

\* Evaporation from impervious areas was assumed to be 20% of precipitation.





Project: Glenelg Expansion Lands  
 Project No: 1060-6220  
 Modelled By: KS  
 Checked By: AW  
 Date: 4-Aug-2022

**Water Budget Summary**  
**Project Name: Glenelg Expansion Lands**  
**Water Balance/Water Budget Assessment**

| Characteristic  | Site            |                  |  |                      |  |
|---|-----------------|------------------|--|----------------------|--|
|   | Pre-Development | Post-Development | Post-Development<br><i>with Mitigation</i> | Change (Pre to Post) | Change (Pre to Post)<br><i>with Mitigation</i> |
| <b>Inputs (Volumes)</b>                                   |                 |                  |  |                      |  |
| Precipitation (m <sup>3</sup> /yr)                        | 296130          | 296130           | 296130                                     | 0%                   | 0%   |
| Run-On (m <sup>3</sup> /yr)                               | 0               | 0                | 0  | 0%                   | 0%   |
| Other inputs (m <sup>3</sup> /yr)                         | 0               | 0                | 0  | 0%                   | 0%   |
| <b>Total Inputs (m<sup>3</sup>/yr)</b>                    | <b>296130</b>   | <b>296130</b>    | <b>296130</b>                              | <b>0</b>             | <b>0</b>                                       |
| <b>Outputs (Volumes)</b>                                  |                 |                  |  |                      |  |
| Precipitation Surplus (m <sup>3</sup> /yr)                | 153470          | 203961           | 198598                                     | 33%                  | 29%  |
| Net Surplus (m <sup>3</sup> /yr)                          | 153470          | 203961           | 198598                                     | 33%                  | 29%  |
| Evapotranspiration (m <sup>3</sup> /yr)                   | 142660          | 92168            | 90827                                      | -35%                 | -36%   |
| Infiltration (m <sup>3</sup> /yr)                         | 84408           | 33327            | 33327                                      | -61%                 | -61%   |
| Topsoil Amendment Infiltration (m <sup>3</sup> /yr)       | 0               | 0                | 6409                                       | -                    | <b>6409 m3/yr</b>                              |
| Bioretention Infiltration (m <sup>3</sup> /yr)            | 0               | 0                | 2295                                       | -                    | <b>2295 m3/yr</b>                              |
| Tree Pit Infiltration (m <sup>3</sup> /yr)                | 0               | 0                | 14508                                      | -                    | <b>14508 m3/yr</b>                             |
| Permeable Paver Infiltration - Park (m <sup>3</sup> /yr)  | 0               | 0                | 2489                                       | -                    | <b>2489 m3/yr</b>                              |
| Permeable Paver Infiltration - North (m <sup>3</sup> /yr) | 0               | 0                | 2316                                       | -                    | <b>2316 m3/yr</b>                              |
| <b>Total Infiltration (m<sup>3</sup>/yr)</b>              | <b>84408</b>    | <b>33327</b>     | <b>61345</b>                               | -61%                 | -27%   |
| Runoff Pervious Areas (m <sup>3</sup> /yr)                | 69061           | 27268            | 13525                                      | -61%                 | -80%   |
| Runoff Impervious Areas (m <sup>3</sup> /yr)              | 0               | 143367           | 123728                                     | -                    | -  |
| <b>Total Runoff (m<sup>3</sup>/yr)</b>                    | <b>69061</b>    | <b>170634</b>    | <b>137253</b>                              | <b>147%</b>          | <b>99%</b>                                     |
| <b>Total Outputs (m<sup>3</sup>/yr)</b>                   | <b>296130</b>   | <b>296130</b>    | <b>289426</b>                              | <b>0%</b>            | <b>0%</b>                                      |

**NOTES:**

\* Total Infiltration into groundwater system (25702m<sup>3</sup>/yr) is to be maintained via the proposed LIDs.

Months contributing to Water Balance (winter months not considered due to freezing effects) - April, May, June, July, August, September, October = 7 months



**Project:** Glenelg Expansion Lands  
**Project No:** 1060-6220  
**Modelled By:** KS  
**Date:** 4-Aug-2022

**Design Storm Determination - Permeable Pavers in Park**  
**Project Name: Glenelg Expansion Lands**  
**Water Balance/Water Budget Assessment**

Days with Precipitation (From Climate Data)

|           | Apr  | May  | Jun  | Jul  | Aug  | Sep  | Oct  | Total |
|-----------|------|------|------|------|------|------|------|-------|
| >= 0.2 mm | 14.6 | 13   | 12.8 | 11.9 | 13.1 | 15.3 | 16.9 | 98    |
| >= 5 mm   | 4.7  | 5.6  | 5.2  | 4.5  | 5.2  | 6.2  | 6.1  | 38    |
| >= 10 mm  | 2.1  | 2.9  | 3.2  | 2.2  | 3    | 3.5  | 2.9  | 20    |
| >= 25 mm  | 0.32 | 0.64 | 0.86 | 0.81 | 0.81 | 0.86 | 0.33 | 5     |

Available Precipitation

| Storm Event (mm) | Total Days Per Year | Incremental Precipitation (mm/yr) | Cummulative Precipitation (mm/yr) |
|------------------|---------------------|-----------------------------------|-----------------------------------|
| 0.2              | 98                  | 19.5                              | 19.5                              |
| 5                | 38                  | 187.5                             | 207.0                             |
| 10               | 20                  | 198.0                             | 405.0                             |
| 25               | 5                   | 115.8                             | 520.8                             |
| <b>Total</b>     | <b>160</b>          | <b>520.8</b>                      |                                   |

Permeable Paver Infiltration Target: 150 mm/yr  
 Runoff Coefficient: 0.35  
 Design Precipitation: 432 mm/yr (Design Infiltration / Contributing RC)  
 Therefore Min. Design Storm: 13.44 mm  
 Volume of Storage Required: 223.68 m<sup>3</sup>



Project: Glenelg Expansion Lands  
 Project No: 1060-6220  
 Modelled By: KS  
 Date: 8-4-2022

**Mitigation Sizing - Permeable Pavers in Park**  
**Project Name: Glenelg Expansion Lands**  
**Water Balance/Water Budget Assessment**

Contributing Drainage Area: 16639 m<sup>2</sup>  
 Runoff Coefficient: 0.35  
 Design Storm: 13 mm  
 Design Runoff/Infiltration Volume: 223.7 m<sup>3</sup>

**Maximum Depth of Cell**

$$d_{c\max} = i \cdot (t_s - d_p) / V_f$$

Where:

$d_{c\max}$  = Maximum cell depth (mm)

$i$  = Infiltration rate for native soils (mm/hr)

$V_f$  = Void space ratio for filter bed and gravel storage layer (assume 0.4)

$t_s$  = Time to drain

$d_p$  = Maximum surface ponding depth (mm)

**Assumptions**

|               |         |
|---------------|---------|
| $i^1$ =       | 6 mm/hr |
| $V_f$ =       | 0.4     |
| $t_s$ =       | 48 hr   |
| $d_p$ =       | 0 mm    |
| $d_{c\max}$ = | 0.7 m   |

Area of Permeable Pavers: 800 m<sup>2</sup>  
 Storage Depth: 0.720 m  
 Drawdown Time: 48 hr  
 Void Ratio: 0.4  
 Total Volume Retained: 230.4 m<sup>3</sup>

**Based on Borehole MW22-313 S**

Depth to Groundwater: 1.985 m  
 Storage Depth: 0.720 m  
 Clearance from Groundwater: 1.265 m

**Therefore, the proposed system will drain within 48 hours and will provide a retention volume that exceeds the volume for mitigation.**



**Project:** Glenelg Expansion Lands  
**Project No:** 1060-6220  
**Modelled By:** KS  
**Date:** 4-Aug-2022

**Design Storm Determination - Permeable Pavers to North Walkway**  
**Project Name: Glenelg Expansion Lands**  
**Water Balance/Water Budget Assessment**

Days with Precipitation (From Climate Data)

|           | Apr  | May  | Jun  | Jul  | Aug  | Sep  | Oct  | Total |
|-----------|------|------|------|------|------|------|------|-------|
| >= 0.2 mm | 14.6 | 13   | 12.8 | 11.9 | 13.1 | 15.3 | 16.9 | 98    |
| >= 5 mm   | 4.7  | 5.6  | 5.2  | 4.5  | 5.2  | 6.2  | 6.1  | 38    |
| >= 10 mm  | 2.1  | 2.9  | 3.2  | 2.2  | 3    | 3.5  | 2.9  | 20    |
| >= 25 mm  | 0.32 | 0.64 | 0.86 | 0.81 | 0.81 | 0.86 | 0.33 | 5     |

Available Precipitation

| Storm Event (mm) | Total Days Per Year | Incremental Precipitation (mm/yr) | Cummulative Precipitation (mm/yr) |
|------------------|---------------------|-----------------------------------|-----------------------------------|
| 0.2              | 98                  | 19.5                              | 19.5                              |
| 5                | 38                  | 187.5                             | 207.0                             |
| 10               | 20                  | 198.0                             | 405.0                             |
| 25               | 5                   | 115.8                             | 520.8                             |
| <b>Total</b>     | <b>160</b>          | <b>520.8</b>                      |                                   |

Permeable Paver Infiltration Target: 392 mm/yr  
 Runoff Coefficient: 0.66  
 Design Precipitation: 591 mm/yr (Design Infiltration / Contributing RC)  
 Therefore Min. Design Storm: 34.06 mm  
 Volume of Storage Required: 201.38 m<sup>3</sup>



Project: Glenelg Expansion Lands  
Project No: 1060-6220  
Modelled By: KS  
Date: 8-4-2022

**Mitigation Sizing - Permeable Pavers to North Walkway**  
**Project Name: Glenelg Expansion Lands**  
**Water Balance/Water Budget Assessment**

Contributing Drainage Area: 5912 m<sup>2</sup>  
Runoff Coefficient: 0.66  
Design Storm: 34 mm  
Design Runoff/Infiltration Volume: 201.4 m<sup>3</sup>

**Maximum Depth of Cell**

$$d_{c\max} = i \cdot (t_s - d_p) / i / V_r$$

Where:

$d_{c\max}$  = Maximum cell depth (mm)

$i$  = Infiltration rate for native soils (mm/hr)

$V_r$  = Void space ratio for filter bed and gravel storage layer (assume 0.4)

$t_s$  = Time to drain

$d_p$  = Maximum surface ponding depth (mm)

**Assumptions**

|               |         |
|---------------|---------|
| $i^1$ =       | 6 mm/hr |
| $V_r$ =       | 0.4     |
| $t_s$ =       | 48 hr   |
| $d_p$ =       | 0 mm    |
| $d_{c\max}$ = | 0.7 m   |

Area of Permeable Pavers: 700 m<sup>2</sup>  
Storage Depth: 0.720 m  
Drawdown Time: 48 hr  
Void Ratio: 0.4  
Total Volume Retained: 201.6 m<sup>3</sup>

**Based on Borehole MW22-313 S**

Depth to Groundwater: 1.985 m  
Storage Depth: 0.720 m  
Clearance from Groundwater: 1.265 m

**Therefore, the proposed system will drain within 48 hours and will provide a retention volume that exceeds the volume for mitigation.**



**Project:** Glenelg Expansion Lands  
**Project No:** 1060-6220  
**Modelled By:** KS  
**Date:** 4-Aug-2022

**Design Storm Determination - Bioretention**  
**Project Name: Glenelg Expansion Lands**  
**Water Balance/Water Budget Assessment**

Days with Precipitation (From Climate Data)

|           | Apr  | May  | Jun  | Jul  | Aug  | Sep  | Oct  | Total |
|-----------|------|------|------|------|------|------|------|-------|
| >= 0.2 mm | 14.6 | 13   | 12.8 | 11.9 | 13.1 | 15.3 | 16.9 | 98    |
| >= 5 mm   | 4.7  | 5.6  | 5.2  | 4.5  | 5.2  | 6.2  | 6.1  | 38    |
| >= 10 mm  | 2.1  | 2.9  | 3.2  | 2.2  | 3    | 3.5  | 2.9  | 20    |
| >= 25 mm  | 0.32 | 0.64 | 0.86 | 0.81 | 0.81 | 0.86 | 0.33 | 5     |

Available Precipitation

| Storm Event (mm) | Total Days Per Year | Incremental Precipitation (mm/yr) | Cummulative Precipitation (mm/yr) |
|------------------|---------------------|-----------------------------------|-----------------------------------|
| 0.2              | 98                  | 19.5                              | 19.5                              |
| 5                | 38                  | 187.5                             | 207.0                             |
| 10               | 20                  | 198.0                             | 405.0                             |
| 25               | 5                   | 115.8                             | 520.8                             |
| <b>Total</b>     | <b>160</b>          | <b>520.8</b>                      |                                   |

Bioretention Infiltration Target: 138 mm/yr  
 Runoff Coefficient: 0.35  
 Design Precipitation: 398 mm/yr (Design Infiltration / Contributing RC)  
 Therefore Min. Design Storm: 9.08 mm  
 Volume of Storage Required: 151.13 m<sup>3</sup>





**Project:** Glenelg Expansion Lands  
**Project No:** 1060-6220  
**Modelled By:** KS  
**Date:** 8-4-2022

**Mitigation Sizing - Bioretention**  
**Project Name: Glenelg Expansion Lands**  
**Water Balance/Water Budget Assessment**

|                                    |                      |
|------------------------------------|----------------------|
| Contributing Drainage Area:        | 16639 m <sup>2</sup> |
| Runoff Coefficient:                | 0.35                 |
| Design Storm:                      | 9 mm                 |
| Design Runoff/Infiltration Volume: | 151.1 m <sup>3</sup> |

**Maximum Depth of Cell**

$$d_{c\max} = i*(t_s - d_p/i)/V_r$$

Where:

$d_{c\max}$  = Maximum cell depth (mm)

$i$  = Infiltration rate for native soils (mm/hr)

$V_r$  = Void space ratio for filter bed and gravel storage layer (assume 0.4)

$t_s$  = Time to drain

$d_p$  = Maximum surface ponding depth (mm)

**Assumptions**

|               |         |
|---------------|---------|
| $i^1$ =       | 6 mm/hr |
| $V_r$ =       | 0.4     |
| $t_s$ =       | 48 hr   |
| $d_p$ =       | 0 mm    |
| $d_{c\max}$ = | 0.7 m   |

|                         |                       |
|-------------------------|-----------------------|
| Length of Bioretention: | 60 m                  |
| Width of Bioretention:  | 3 m                   |
| Number of LIDs:         | 3                     |
| Storage Depth:          | 0.720 m               |
| Drawdown Time:          | 48 hr                 |
| Total Volume Retained:  | 155.52 m <sup>3</sup> |

**Based on Borehole MW22-313 S**

|                             |         |
|-----------------------------|---------|
| Depth to Groundwater:       | 1.985 m |
| Storage Depth:              | 0.720 m |
| Clearance from Groundwater: | 1.265 m |

**Therefore, the proposed system will drain within 48 hours and will provide a retention volume that exceeds the volume for mitigation.**



**Project:** Glenelg Expansion Lands  
**Project No:** 1060-6220  
**Modelled By:** KS  
**Date:** 27-Jul-2022

**Design Storm Determination - Tree Pits**  
**Project Name: Glenelg Expansion Lands**  
**Water Balance/Water Budget Assessment**

Days with Precipitation (From Climate Data)

|           | Apr  | May  | Jun  | Jul  | Aug  | Sep  | Oct  | Total |
|-----------|------|------|------|------|------|------|------|-------|
| >= 0.2 mm | 14.6 | 13   | 12.8 | 11.9 | 13.1 | 15.3 | 16.9 | 98    |
| >= 5 mm   | 4.7  | 5.6  | 5.2  | 4.5  | 5.2  | 6.2  | 6.1  | 38    |
| >= 10 mm  | 2.1  | 2.9  | 3.2  | 2.2  | 3    | 3.5  | 2.9  | 20    |
| >= 25 mm  | 0.32 | 0.64 | 0.86 | 0.81 | 0.81 | 0.86 | 0.33 | 5     |

Available Precipitation

| Storm Event (mm) | Total Days Per Year | Incremental Precipitation (mm/yr) | Cummulative Precipitation (mm/yr) |
|------------------|---------------------|-----------------------------------|-----------------------------------|
| 0.2              | 98                  | 19.5                              | 19.5                              |
| 5                | 38                  | 187.5                             | 207.0                             |
| 10               | 20                  | 198.0                             | 405.0                             |
| 25               | 5                   | 115.8                             | 520.8                             |
| <b>Total</b>     | <b>160</b>          | <b>520.8</b>                      |                                   |

Tree Pit Infiltration Target: 96 mm/yr  
 Runoff Coefficient: 0.67  
 Design Precipitation: 143 mm/yr (Design Infiltration / Contributing RC)  
 Therefore Min. Design Storm: 3.35 mm  
 Volume of Storage Required: 505.4 m<sup>3</sup>



Project: Glenelg Expansion Lands  
 Project No: 1060-6220  
 Modelled By: KS  
 Date: 7-27-2022

**Stormwater Tree Pit**  
**Project Name: Glenelg Expansion Lands**

**Water Balance/Water Budget Assessment**

| Location                     | Block Area (Ha) | TIMP | RC   | A*RC | A*RC Proration | Required Storage Volume (m <sup>3</sup> ) | Native Soil Infiltration Rate* (mm/hr) | Safety Factor | Native Soil Infiltration Rate with Safety Factor (mm/hr) | Void Space of Sand Layer | Void Space Ratio of Gravel Layer | Depth of Sand Layer (m) | Depth of Gravel Layer (m) | Actual Length of Bioretention Cell (m) | Actual Width of the Bioretention Cell (m) | Provided Surface Area (m <sup>2</sup> ) | Drawdown Time (hrs) | Total Retention Storage Provided per LID (m <sup>3</sup> ) | # of LIDs | Total Retention Storage Provided (m <sup>3</sup> ) |
|------------------------------|-----------------|------|------|------|----------------|---|--|---------------|--|--------------------------|----------------------------------|-------------------------|---------------------------|--|---|---|---------------------|--|-----------|--|
| Tree Pits - CP Rail          | 1.280           | 69%  | 0.68 | 0.87 | 8%             | 40  | 12                                     | 2.0           | 6.00   | 0.20                     | 0.40                             | 0.10                    | 0.67                      | 2                                      | 2   | 4                                       | 48.00               | 1,152  | 29        | 33.41  |
| Tree Pits - East Tile Drain  | 0.863           | 65%  | 0.66 | 0.57 | 5%             | 26  | 12                                     | 2.0           | 6.00   | 0.20                     | 0.40                             | 0.10                    | 0.67                      | 2                                      | 2   | 4                                       | 48.00               | 1,152  | 20        | 23.04  |
| Tree Pits - North Tile Drain | 14.208          | 68%  | 0.68 | 9.60 | 87%            | 505                                       | 12                                     | 2.0           | 6.00   | 0.20                     | 0.40                             | 0.10                    | 0.67                      | 2                                      | 2   | 4                                       | 48.00               | 1,152  | 390       | 449.28   |

Notes:  
 \*Soil infiltration rate estimated as minimum 12mm/hr for infiltration LID's as per MOE 2003 Design Guidelines.  
 \*\*Tree Species used shall be salt resistant species (i.e. Honey Locust, American Elm Cultivars, Kentucky Coffee-tree, Hackberry, & Bur Oak).  
 \*\*\*Use of a Type 4 Bio Retention Mix is recommended for this feature, as per TS 5.10.

**Soil Volumes**

| Area ID     | Soil Depth (m) | Provided LID Area (m <sup>2</sup> ) | Provided Volume (m <sup>3</sup> ) | Allow. Vol per Tree (m <sup>3</sup> )* | Allowable Number of Trees in LID |
|-------------|----------------|-------------------------------------|-----------------------------------|--|----------------------------------|
| Tree Pit#0A | 1.50           | 3.00                                | 4.5                               | 30                                     | 1.0                              |

# APPENDIX G

## Glenelg Phase 2 Lands Design Excerpts



Project Name: GlenElg Phase 2  
 Project Number: 1060-5545  
 Date: 2022-08-28  
 By: AM

D.A. NAME TR-1  
 D.A. AREA (ha) 0.78

**Hydrologic Parameters: CALIB STANDHYD Command**  
**Post Development Drainage Area: Catchment TR-1**  
**Uncontrolled Area to CP Rail Trail**

**Curve Number Calculation**

| Soil Types Present: |    |                  |        |      |
|---------------------|----|------------------|--------|------|
| Type                | ID | Hydrologic Group | % Area | Area |
| Listowel Silt Loam  | Ls | BC               | 100    | 0.78 |
| Total Area Check    |    |                  |        | 0.78 |

| Impervious Landuses Present: |           |    |           |    |           |    |           |    |           |    |           |       |
|------------------------------|-----------|----|-----------|----|-----------|----|-----------|----|-----------|----|-----------|-------|
| Soils                        | Roadway   |    | Sidewalk  |    | Driveway  |    | Building  |    | SWMF      |    | Subtotals |       |
|                              | Area (ha) | CN | Area (ha) | CN | Area (ha) | CN | Area (ha) | CN | Area (ha) | CN | Area      | A*CN  |
| Ls                           | 0.00      | 98 | 0.00      | 98 | 0.00      | 98 | 0.15      | 98 | 0         | 50 | 0.15      | 14.36 |
|                              |           | 98 |           | 98 |           | 98 |           | 98 |           | 50 | 0         | 0     |
|                              |           | 98 |           | 98 |           | 98 |           | 98 |           | 50 | 0         | 0     |
|                              |           | 98 |           | 98 |           | 98 |           | 98 |           | 50 | 0         | 0     |
| Subtotal Area                | 0.00      |    | 0.00      |    | 0.00      |    | 0.15      |    | 0         |    |           |       |

| Pervious Landuses Present: |           |    |           |    |           |    |           |    |            |    |           |       |
|----------------------------|-----------|----|-----------|----|-----------|----|-----------|----|------------|----|-----------|-------|
| Soils                      | Woodland  |    | Meadow    |    | Wetland   |    | Lawn      |    | Cultivated |    | Subtotals |       |
|                            | Area (ha) | CN | Area (ha) | CN | Area (ha) | CN | Area (ha) | CN | Area (ha)  | CN | Area      | A*CN  |
| Ls                         | 0         | 67 | 0         | 71 | 0         | 50 | 0.63      | 74 | 0          | 78 | 0.63      | 46.88 |
|                            | 0         |    | 0         |    | 0         |    | 0         |    | 0          |    | 0         | 0     |
|                            | 0         |    | 0         |    | 0         |    | 0         |    | 0          |    | 0         | 0     |
|                            | 0         |    | 0         |    | 0         |    | 0         |    | 0          |    | 0         | 0     |
| Subtotal Area              | 0         |    | 0         |    | 0         |    | 0.63      |    | 0          |    |           |       |

|                              |                                 |      |
|------------------------------|---------------------------------|------|
| Pervious Area Calculations   | Total Pervious Area             | 0.63 |
|                              | Composite Pervious Curve Number | 74   |
| Impervious Area Calculations | Total Directly Connected Area   | 0.0  |
|                              | Total Indirectly Connected Area | 0.15 |
|                              | Total Impervious Area           | 0.15 |
|                              | % X imp                         | 0    |
|                              | % T imp                         | 19   |
| Total Area Check             |                                 | 0.78 |

**Initial Abstraction and Tp Calculations**

| Landuse    | IA (mm) | Area (ha) | A * IA |
|------------|---------|-----------|--------|
| Woodland   | 10      | 0         | 0      |
| Meadow     | 8       | 0         | 0      |
| Wetland    | 16      | 0         | 0      |
| Lawn       | 5       | 0.63      | 3.17   |
| Cultivated | 7       | 0         | 0      |


| Land Use   | IA (mm) | Slope (%) | Travel Length (m) | Manning's n |
|------------|---------|-----------|-------------------|-------------|
| Pervious   | 5.0     | 2         | 30                | 0.25        |
| Impervious | 2.0     | 0.5       | 72                | 0.013       |

# LIST OF FIGURES


|                   |  |
|-------------------|--|
| <b>Figure 1:</b>  | Site Location Plan                                   |
| <b>Figure 2:</b>  | Draft Plan of Subdivision                            |
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| <b>Figure 9:</b>  | Preliminary SWMF #1                                  |
| <b>Figure 10:</b> | Proposed LID Plan #1                                 |



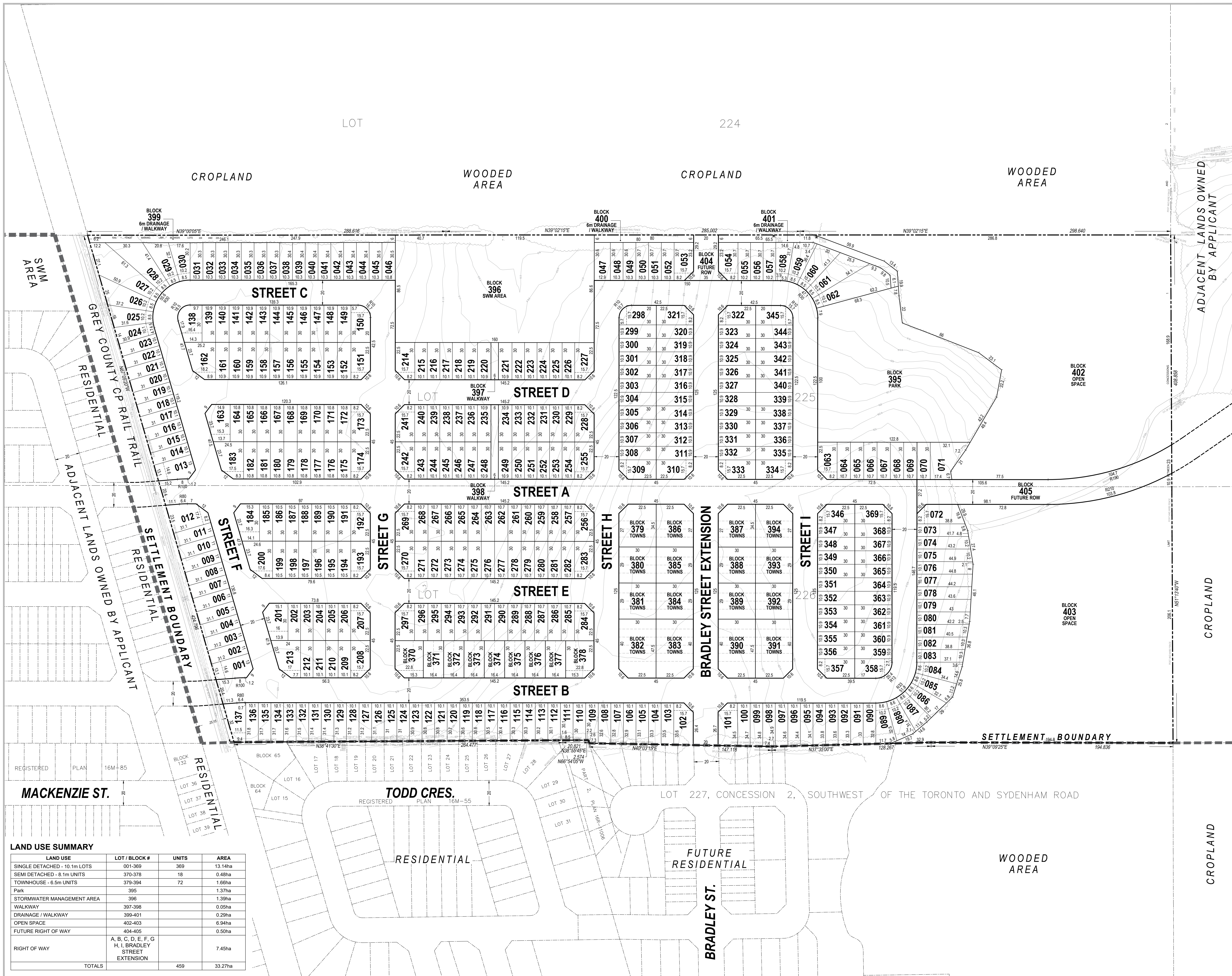


|   |                 |
|---|-----------------|
| Legend  |                 |
|  | = SUBJECT LANDS |

|         |                         |  |  |
|---------|-------------------------|--|--|
| Project | TOWN OF DUNDALK         |  |  |
| Drawing | GLENELG EXPANSION LANDS |  |  |

|   |        |  |                   |
|---|--------|--|-------------------|
|  |        | ADMIRAL BUILDING<br>1 FIRST STREET, SUITE 200<br>COLLINGWOOD, ON, L9Y 1A1<br>705-446-3510 T<br>705-446-3520 F<br>WWW.CROZIER.CA<br>INFO@CROZIER.CA |                   |
| Drawn By  | D.K.   | Design By  | R.M.              |
| Scale   | N.T.S. | Date   | 08/25/2022        |
| Check By  |        | R.M.   | Project 1060-6220 |
|   |        |  | Drawing FIG. 1    |





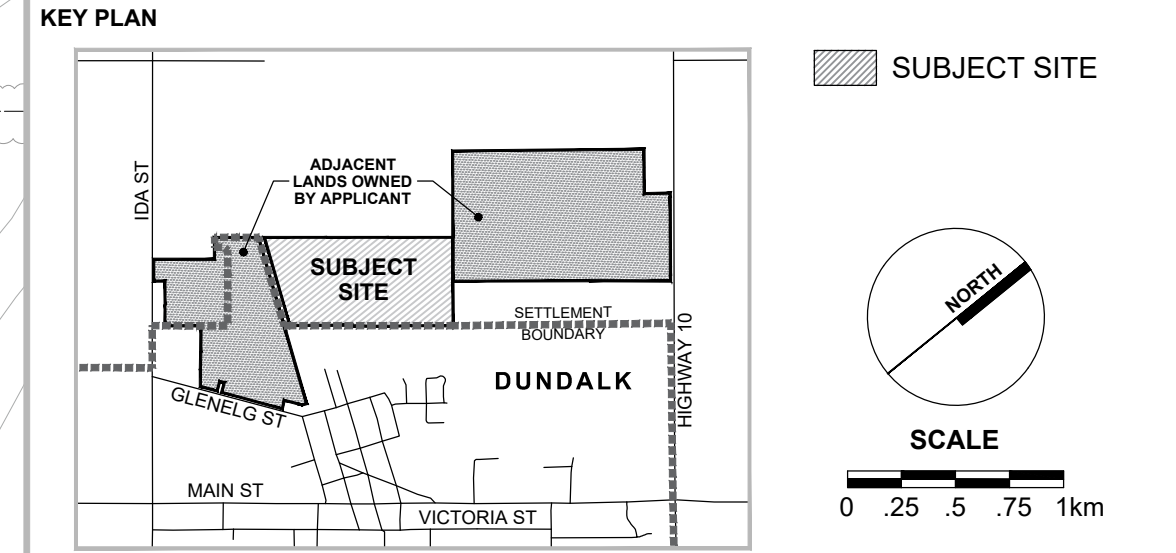
**LEGAL DESCRIPTION**  
 PART OF LOTS 225 AND 226  
 CONCESSION 2, SOUTHWEST OF THE TORONTO AND SYDENHAM ROAD  
 GEOGRAPHIC TOWNSHIP OF PROTON  
 TOWNSHIP OF SOUTHGATE  
 COUNTY OF GREY

**OWNER'S CERTIFICATE**  
 I HEREBY AUTHORIZE MACNAUGHTON HERMSEN BRITTON CLARKSON PLANNING LIMITED  
 TO SUBMIT THIS PLAN FOR APPROVAL.

DATE: AUGUST 18, 2022  
 SHAKIR REHMATULLAH - PRESIDENT  
 DUNDALK VILLAGE TWO INC.

**SURVEYOR'S CERTIFICATE**  
 I HEREBY CERTIFY THAT THE BOUNDARIES OF THE LAND TO BE SUBDIVIDED ON THIS PLAN  
 AND THEIR RELATIONSHIP TO THE ADJACENT LANDS ARE ACCURATELY AND CORRECTLY  
 SHOWN.

DATE: AUGUST 18, 2022  
 DAN DZALDOV - O.L.S.  
 SCHAEFFER DZALDOV BENNETT LTD.



| REVISION No.  | DATE   | ISSUED / REVISION                       | BY |
|---|--|---|----|
| ADDITIONAL INFORMATION REQUIRED UNDER SECTION 51(17)<br>OF THE PLANNING ACT R.S.O. 1990 C.P.13 AS AMENDED |  |   |    |
| A. AS SHOWN   | E. AS SHOWN  | J. AS SHOWN                             |    |
| B. AS SHOWN   | F. AS SHOWN  | K. ALL SERVICES AS REQUIRED             |    |
| C. AS SHOWN   | G. AS SHOWN  | (WATER, SANITARY,<br>STORMWATER, HYDRO) |    |
| D. 369 SINGLES, 18 SEMIS,<br>& 72 TOWNHOUSES.   | H. MUNICIPAL WATER SUPPLY<br>& L.O.A.M.S.I.T. LOAM | L. AS SHOWN                             |    |

**PLANNING URBAN DESIGN & LANDSCAPE ARCHITECTURE**  
**MHBC PLANNING**  
 113 COLLIER STREET  
 MARKHAM, ON L3R 0G6  
 P: 705 728 0045 F: 705 728 2010  
 WWW.MHBCPLAN.COM

**LAND USE SUMMARY**

| LAND USE                     | LOT / BLOCK #  | UNITS      | AREA           |
|------------------------------|--|------------|----------------|
| SINGLE DETACHED - 10.1m LOTS | 001-369  | 369        | 13.14ha        |
| SEMI DETACHED - 8.1m UNITS   | 370-378  | 18         | 0.48ha         |
| TOWNHOUSE - 6.5m UNITS       | 379-394  | 72         | 1.66ha         |
| Park                         | 395  |            | 1.37ha         |
| STORMWATER MANAGEMENT AREA   | 396  |            | 1.39ha         |
| WALKWAY                      | 397-398  |            | 0.05ha         |
| DRAINAGE / WALKWAY           | 399-401  |            | 0.29ha         |
| OPEN SPACE                   | 402-403  |            | 6.94ha         |
| FUTURE RIGHT OF WAY          | 404-405  |            | 0.50ha         |
| RIGHT OF WAY                 | A, B, C, D, E, F, G<br>H, I, BRADLEY STREET<br>EXTENSION |            | 7.45ha         |
| <b>TOTALS</b>                |  | <b>459</b> | <b>33.27ha</b> |

**STAMP**

|            |                  |
|------------|------------------|
| DATE       | AUG. 18, 2022    |
| FILE No.   | 15184AT          |
| SCALE      | 1:1,400 (ARCH D) |
| DRAWN BY   | M.M.             |
| CHECKED BY | K.C.             |
| OTHER      |                  |

**PROJECT**  
**GLENELG PHASE 3**  
 DUNDALK VILLAGE TWO INC.  
 3621 HIGHWAY 7 EAST, SUITE 503  
 MARKHAM, ON L3R 0G6  
 P:(905) 479-9292 F:(905) 429-9165  
 WWW.FLATOGROUP.COM

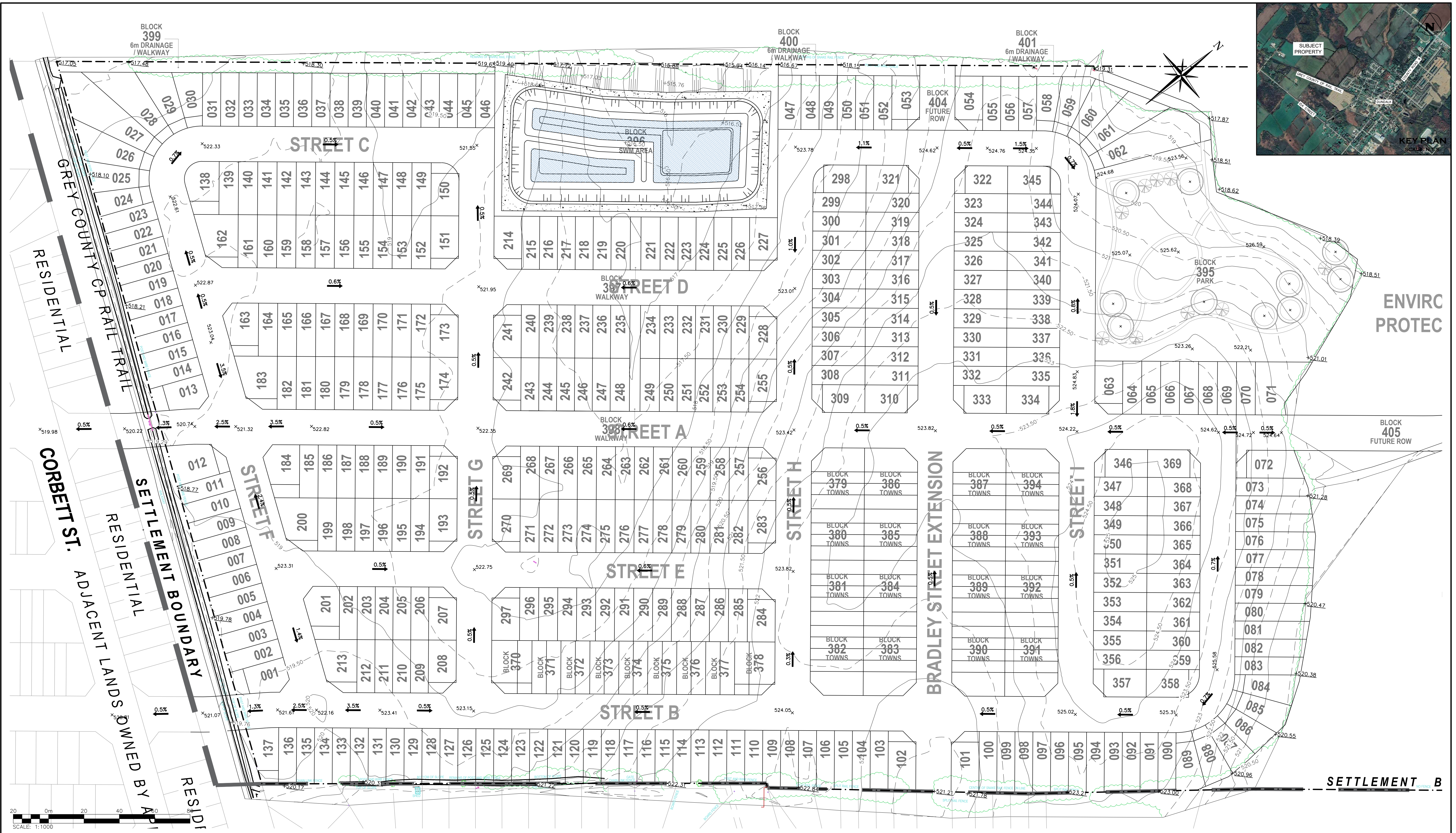
**FILE NAME**  
 DRAFT  
 PLAN OF SUBDIVISION

**DWG No.**  
 1 of 1

**SCALE BAR**  
 0 7 14 21 28 35 52.5 70 105 140m  
 MEASUREMENTS SHOWN ON THIS PLAN ARE IN METRES AND CAN BE  
 CONVERTED TO FEET BY DIVIDING BY 0.3048

N:\Southgate\15184AT\Drawings\Draft\_Plan\CAD\





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| LEGEND    |                                     |
|-----------|-------------------------------------|
| ---XXX.X  | EX CONTOUR                          |
| -XXX.XX   | PR. ELEVATION                       |
| +XXX.XX   | PR. ELEVATION (MATCH EX. ELEVATION) |
| - - - - - | PROPERTY LINE                       |

| No. | ISSUE          | DATE: MMM/DD/YYYY | Engineer |
|-----|----------------|-------------------|----------|
| 0   | 1ST SUBMISSION | AUG/26/2022       |          |

| Project  |  |
|--|--|
| GLENELG EXPANSION LANDS<br>TOWNSHIP OF SOUTHGATE |  |
| Drawing  |  |
| PRELIMINARY GRADING PLAN                         |  |

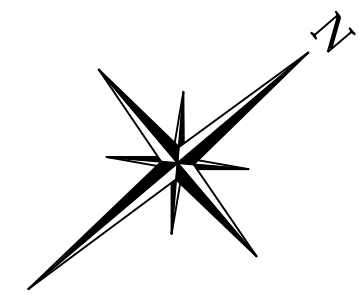
|          |        |           |           |         |           |
|----------|--------|-----------|-----------|---------|-----------|
| Drawn By | R.D.M. | Design By | K.S./A.W. | Project | 1060-6220 |
| Check By | K.S.   | Check By  | A.W.      | Scale   | 1:1000    |
|          |        |           |           | Drawing | FIG 3     |

**ADMIRAL BUILDING**  
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COLLINGWOOD, ON, L9Y 1A1  
705-446-3510 T  
705-446-3520 F  
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DRAINAGE TO GREY COUNTY CP TRAIL (OUTLET #1)  
4.32 ha.

WOODED AREA

CROPLAND

WOODED AREA

PROPERTY LINE

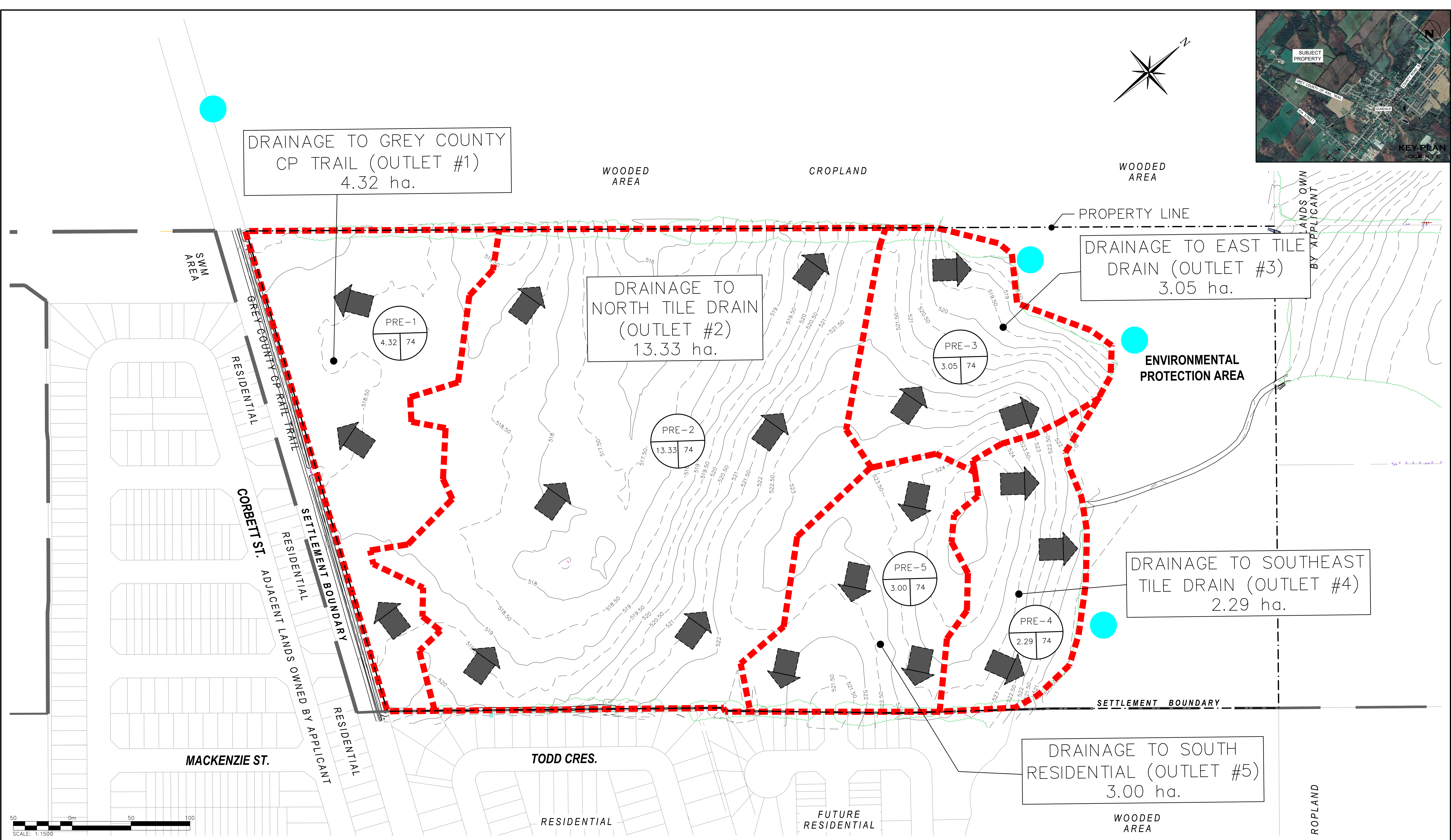
DRAINAGE TO EAST TILE DRAIN (OUTLET #3)  
3.05 ha.

DRAINAGE TO NORTH TILE DRAIN (OUTLET #2)  
13.33 ha.

ENVIRONMENTAL PROTECTION AREA

DRAINAGE TO SOUTHEAST TILE DRAIN (OUTLET #4)  
2.29 ha.

DRAINAGE TO SOUTH RESIDENTIAL (OUTLET #5)  
3.00 ha.



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| LEGEND |                                 |
|--------|---------------------------------|
|        | OVERLAND FLOW DIRECTION         |
|        | CATCHMENT AREAS                 |
|        | PROPERTY LINE                   |
|        | DRAINAGE ID                     |
|        | AREA (ha)   CN COEFFICIENT      |
|        | APPROXIMATE TILE DRAIN LOCATION |

| No. | ISSUE          | DATE: MMM/DD/YYYY | Engineer |
|-----|----------------|-------------------|----------|
| 0   | 1ST SUBMISSION | AUG/26/2022       |          |
|     |                |                   |          |
|     |                |                   |          |
|     |                |                   |          |

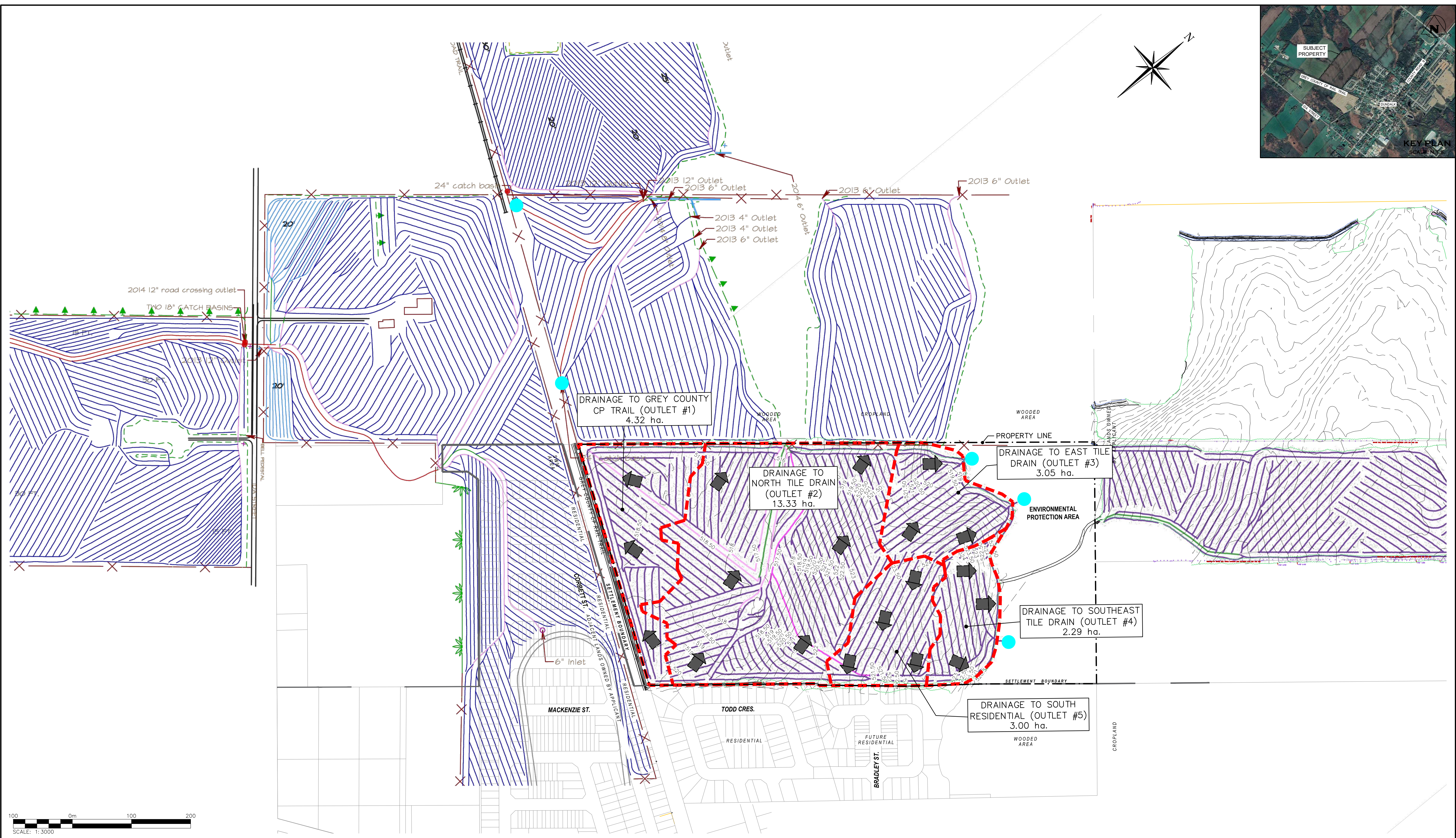
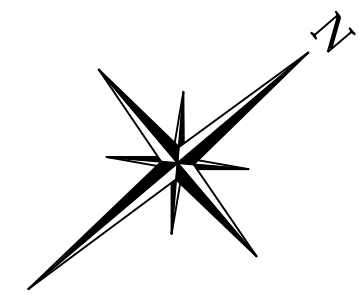
| Project  |  |
|--|--|
| GLENELG EXPANSION LANDS<br>TOWNSHIP OF SOUTHGATE |  |
| Drawing  |  |
| PREDEVELOPMENT DRAINAGE PLAN                     |  |

|          |        |           |           |         |           |
|----------|--------|-----------|-----------|---------|-----------|
| Drawn By | R.D.M. | Design By | K.S./A.W. | Project | 1060-6220 |
| Check By | K.S.   | Check By  | A.W.      | Scale   | 1:1500    |
|          |        |           |           | Drawing | FIG 5     |

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ADMIRAL BUILDING  
1 FIRST STREET, SUITE 200  
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705-446-3520 F  
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| LEGEND |                                 |
|--------|---------------------------------|
|        | OVERLAND FLOW DIRECTION         |
|        | CATCHMENT AREAS                 |
|        | PROPERTY LINE                   |
|        | APPROXIMATE TILE DRAIN LOCATION |

| No. | ISSUE          | DATE: MMM/DD/YYYY |
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| Engineer | Engineer | Project |
|----------|----------|---------|
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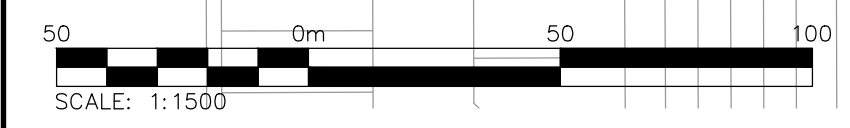
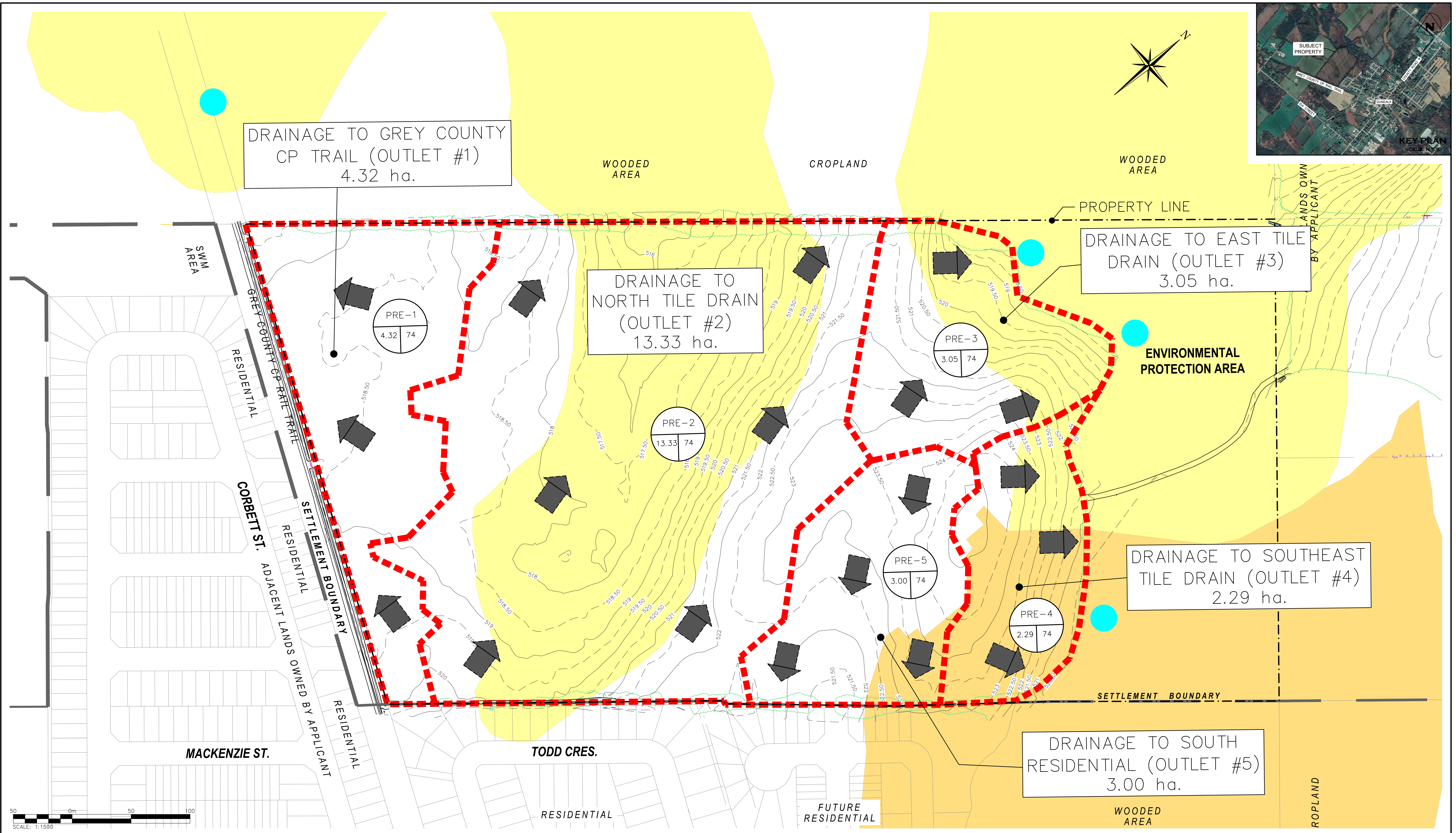
**GLENELG EXPANSION LANDS  
TOWNSHIP OF SOUTHGATE**

**EXISTING TILE DRAINS AND  
PREDEVELOPMENT DRAINAGE**

ADMIRAL BUILDING  
1 FIRST STREET, SUITE 200  
COLLINGWOOD, ON, L9Y 1A1  
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|----------|--------|-----------|-----------|---------|--------------|
| Drawn By | R.D.M. | Design By | K.S./A.W. | Project | 1060-6220    |
| Check By | K.S.   | Check By  | A.W.      | Scale   | 1:3000       |
|          |        |           |           |         | <b>FIG 6</b> |





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| LEGEND |                                 |
|--------|---------------------------------|
|        | OVERLAND FLOW DIRECTION         |
|        | CATCHMENT AREAS                 |
|        | PROPERTY LINE                   |
|        | DRAINAGE ID                     |
|        | AREA (ha)   CN COEFFICIENT      |
|        | APPROXIMATE TILE DRAIN LOCATION |
|        | GRCA REGULATION LIMIT           |
|        | SVCA SCREENING AREA (2021)      |

| No. | ISSUE          | DATE: MMM/DD/YYYY | Engineer |
|-----|----------------|-------------------|----------|
| 0   | 1ST SUBMISSION | AUG/26/2022       |          |
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|         |  |  |  |
|---------|--|--|--|
| Project |  | GLENELG EXPANSION LANDS<br>TOWNSHIP OF SOUTHGATE     |  |
| Drawing |  | CONSERVATION AREAS & PREDEVELOPMENT<br>DRAINAGE PLAN |  |

|          |        |           |           |         |           |
|----------|--------|-----------|-----------|---------|-----------|
| Drawn By | R.D.M. | Design By | K.S./A.W. | Project | 1060-6220 |
| Check By | K.S.   | Check By  | A.W.      | Scale   | 1:1500    |
|          |        |           |           | Drawing | FIG 7     |

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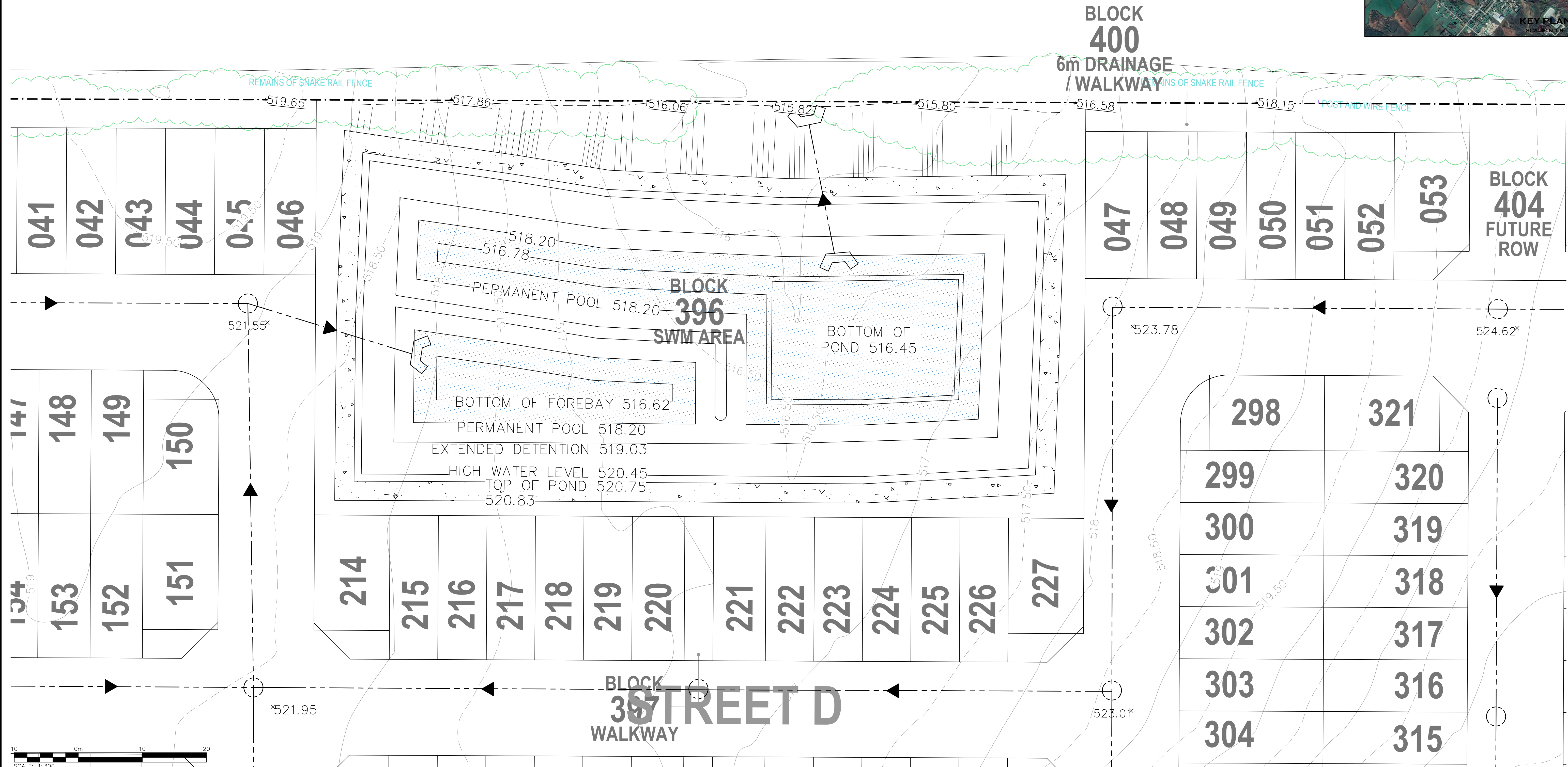
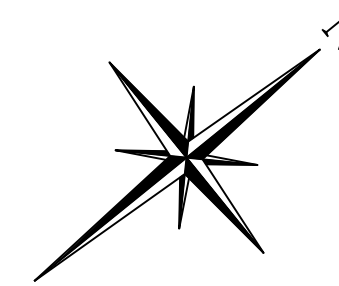
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# AREA



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|      |  |
|------|--|
| Town |  |
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| 0   | 1ST SUBMISSION |
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| DATE: MMM/DD/YYYY | Engineer |
| AUG/26/2022       |          |
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| Engineer |  |
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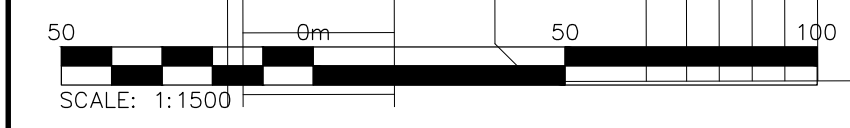
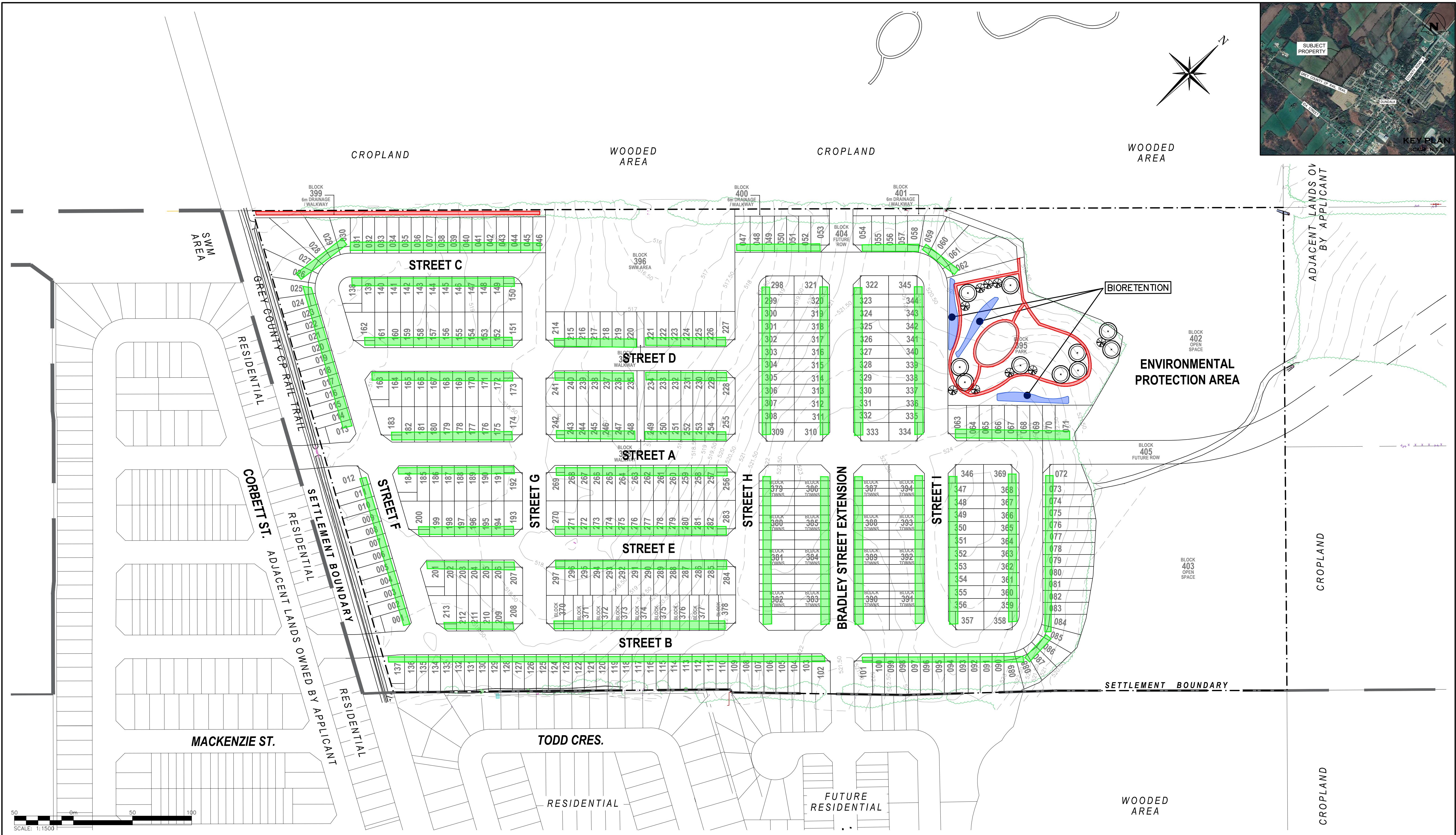
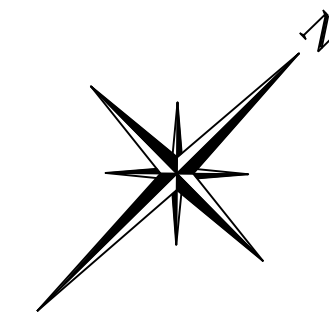
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|---------|--|
| Project | GLENELG EXPANSION LANDS<br>TOWNSHIP OF SOUTHGATE |
| Drawing | PRELIMINARY SWMF#1                               |

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ADMIRAL BUILDING  
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705-446-3520 F  
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|          |        |           |           |         |           |
|----------|--------|-----------|-----------|---------|-----------|
| Drawn By | R.D.M. | Design By | K.S./A.W. | Project | 1060-6220 |
| Check By | K.S.   | Check By  | A.W.      | Scale   | 1:300     |
|          |        |           |           |         | Drawing   |
|          |        |           |           |         | FIG 9     |





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| LEGEND |  |
|--------|--|
|        | APPROXIMATE LOCATION OF PERMEABLE PAVEMENT |
|        | PROPERTY LINE                              |
|        | APPROXIMATE LOCATION OF TREE PITS          |
|        | APPROXIMATE LOCATION OF BIORETENTION       |

| No. | ISSUE          | DATE: MMM/DD/YYYY |
|-----|----------------|-------------------|
| 0   | 1ST SUBMISSION | AUG/26/2022       |

| Engineer | Project |
|----------|---------|
|          |         |

**GLENELG EXPANSION LANDS  
TOWNSHIP OF SOUTHGATE**

PROPOSED LID PLAN #1

**CROZIER**  
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|                  |                      |                    |
|------------------|----------------------|--------------------|
| Drawn By: R.D.M. | Design By: K.S./A.W. | Project: 1060-6220 |
| Check By: K.S.   | Scale: 1:1500        | Drawing: FIG 10    |